



INTERIOR REMODEL - MEZZANINE ADDITION

5995 W. AMELIA EARHART DRIVE,
SALT LAKE CITY UTAH, 84116

Architect: Geroux Architects, PLLC
Contact: Joe Geroux, NCARB
917.686.6517



APRIL 18, 2023

CONSTRUCTION SET

ZONING: Salt Lake City Zoning Ordinance (Title 21A)

Zone: Salt Lake City M-1
Use: Warehouse
Lot Size: 23.49 Acres
Allowable Building Height: Maximum Structure Height 85'-0" (Not Applicable)
Actual Building Height: 31'-10" (Interior Remodel Only)
Minimum Yard Requirements: Front & Corner Side 15'; Interior Side & Rear None

PROJECT DESCRIPTION:

The project includes the addition of a steel frame mezzanine within the existing warehouse facility. The mezzanine includes (2) exit stairs and (1) accessible elevator connecting the main warehouse floor area.

CODE REVIEW:

Applicable Codes: Salt Lake City Building Department & Fire Department
2018 IEBC - Compliance Method = Prescriptive Compliance Method (Ch 3 & 5).
2018 International Building Code (IBC) w/ Utah State Amendments which Include:
Building, Plumbing, Fuel/Gas, Mechanical, and Fire Codes.
2020 National Electrical Code (NEC) State of Utah Title 15A requirements have been compiled into these CD's
2018 International Energy Conservation Code (IECC)
ANSI A117.1-2009

Occupancy	Existing Building = F-1
Construction Type	Existing Building = IIB (Ch 6)
Allowable Area	Unlimited
Actual Area	10,690 sqft (Mezzanine Platform)
Allowable Height	N/A (See 503.1.3) (Also Note 504.4 = 12 Stories w/ Sprinkler).
Actual Height	1 Story - 32'-10" at greatest height (Interior Remodel Only - No Exterior Changes)
Number of Exits	2 exits provided from mezzanine to main warehouse level.
Seismic Design	Category = DII Design Wind Speed = N/A
Sprinklers:	Provided with Existing Building. All new modifications will be NFPA 13. Sprinkler Design is by Deferred Submittal. GC to Submit layout for Approval by Salt Lake City Fire Department and Authority Having Jurisdiction.
Fire Separations	508.3. Non - Required
Fire Resistance Ratings	See IBC Table 601 for Type II B - Building Elements (Hours) Primary Structure = 0 Bearing Walls = 0 (0 Exterior; 0 Interior) Non Bearing Walls & Partitions Ext. / Int.= 0 Floor Construction = 0 (6" Concrete Slab on Grade) Roof Construction = 0
Plumbing Fixtures	All minimum required plumbing fixtures are provided in existing building per IBC table 2902.1. Additional fixtures being provided for convience and proximity to office area.

SUBCONSULTANTS:

Structural Engineer:
Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Mechanical Engineering:
Van Boerum & Frank Associates
181 E. 5600 S.
Murray, Utah 84107

Electrical Engineering:
Hunt Electric Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

LEGAL DESCRIPTION:

Lot 4, Bonneville Center Plat A Amended Lots 3 & 4.
7596-2684 7834-0864 7859-1093 9076-7702

DEFERRED SUBMITTALS:

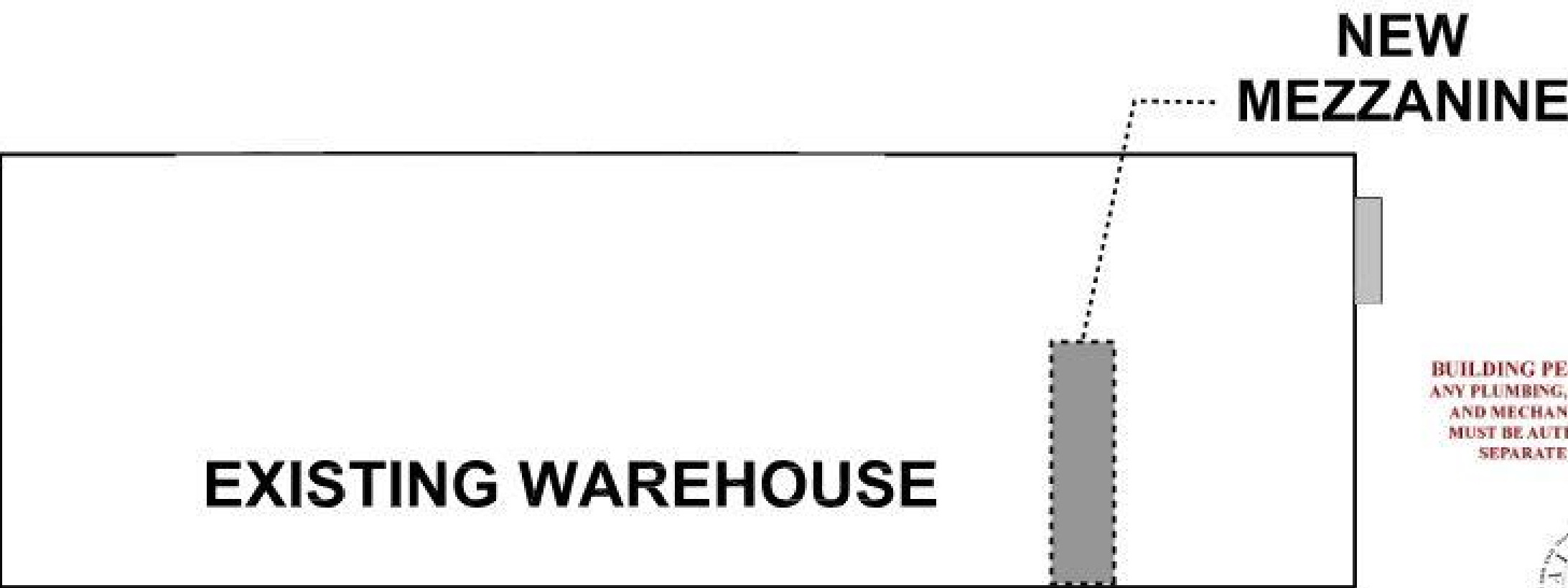
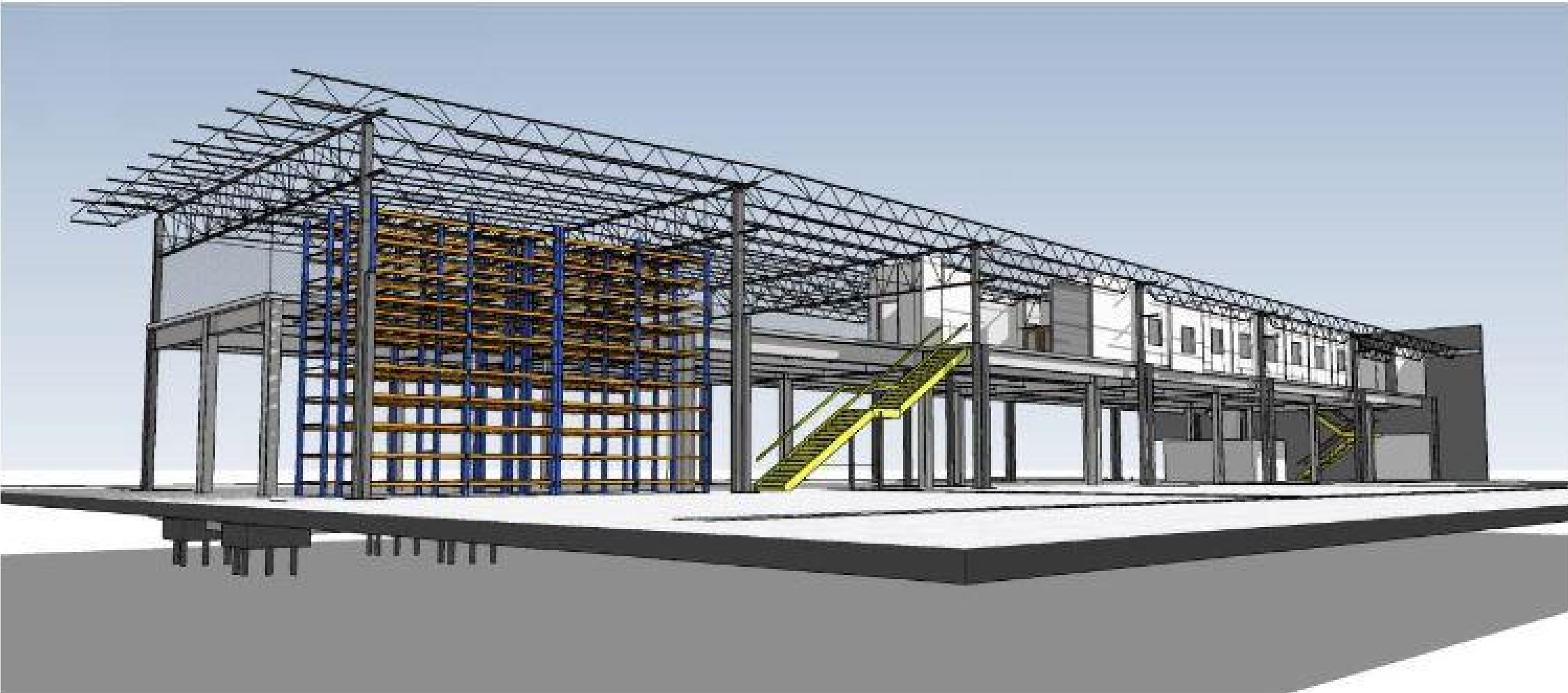
1. Fire Alarm Systems / Controls
2. Fire Suppression Calculations / Shop Drawing Submittal
3. Metal Stud Framing & Siesmic Connections
4. Seismic Bracing for Mechanical, Electrical, Plumbing Components per ASCE 7.

PROJECT DATA : SQFT AREA

Added Mezzanine = 10,690 sqft
Total Area = 10,690 sqft

EXISTING MINIMUM R-VALUES:

GLAZING (FENESTRATION U-FACTOR) = N/A
ROOF R-VALUE = R-30
STEEL FRAMED WALL R-VALUE = R-21
SLAB R-VALUE (within 4' of interior foundation) = R-10



BUILDING PERMIT ONLY
ANY PLUMBING, ELECTRICAL,
AND MECHANICAL WORK
MUST BE AUTHORIZED BY
SEPARATE PERMIT



SALT LAKE CITY BUILDING SERVICES
SUBMIT CONSTRUCTION DOCUMENTS HAVING BEEN:
FOR REVIEW FOR CODE COMPLIANCE
FOR SCHEMATIC REVIEW
CHECKED BY: CHAD TUCKER
DATE APPROVED: 02/08/2023
PERMIT NUMBER: 1840002-01000

GENERAL NOTES

1. IF THERE ARE ANY CONFLICTS BETWEEN ITEMS ON DRAWINGS AND GENERAL NOTES, THE MOST STRINGENT REQUIREMENT AND HIGHEST PRICE PRODUCT GOVERNS
2. ACTUAL SITE DIMENSIONS MAY VARY, CONTRACTOR TO VERIFY ALL DIMENSIONS BEFORE STARTING WORK. CONTRACTOR TO NOTIFY ARCHITECT UPON DISCOVERY OF ANY DISCREPENCIES.

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KEY PLAN

No.	Description	Date
1	PERMIT SUBMISSION	04/18/23
2	PERMIT RE-SUBMISSION	05/18/23
3		
4		
5		
6		

NOTES
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Written dimensions shall have precedence over scaled dimensions. Contractors shall verify and be responsible for all dimensions and conditions on the job and the Architect shall be informed of any variations from the dimensions and conditions shown on the drawing. Shop drawings shall be submitted to the Architect for approval before proceeding with fabrication.

Project Name
ALBANY MEZZANINE

Sheet Title

**FIRE LIFE SAFETY
EGRESS PLANS**

Scale
1/16"=1'-0"

Date
04.18.2023

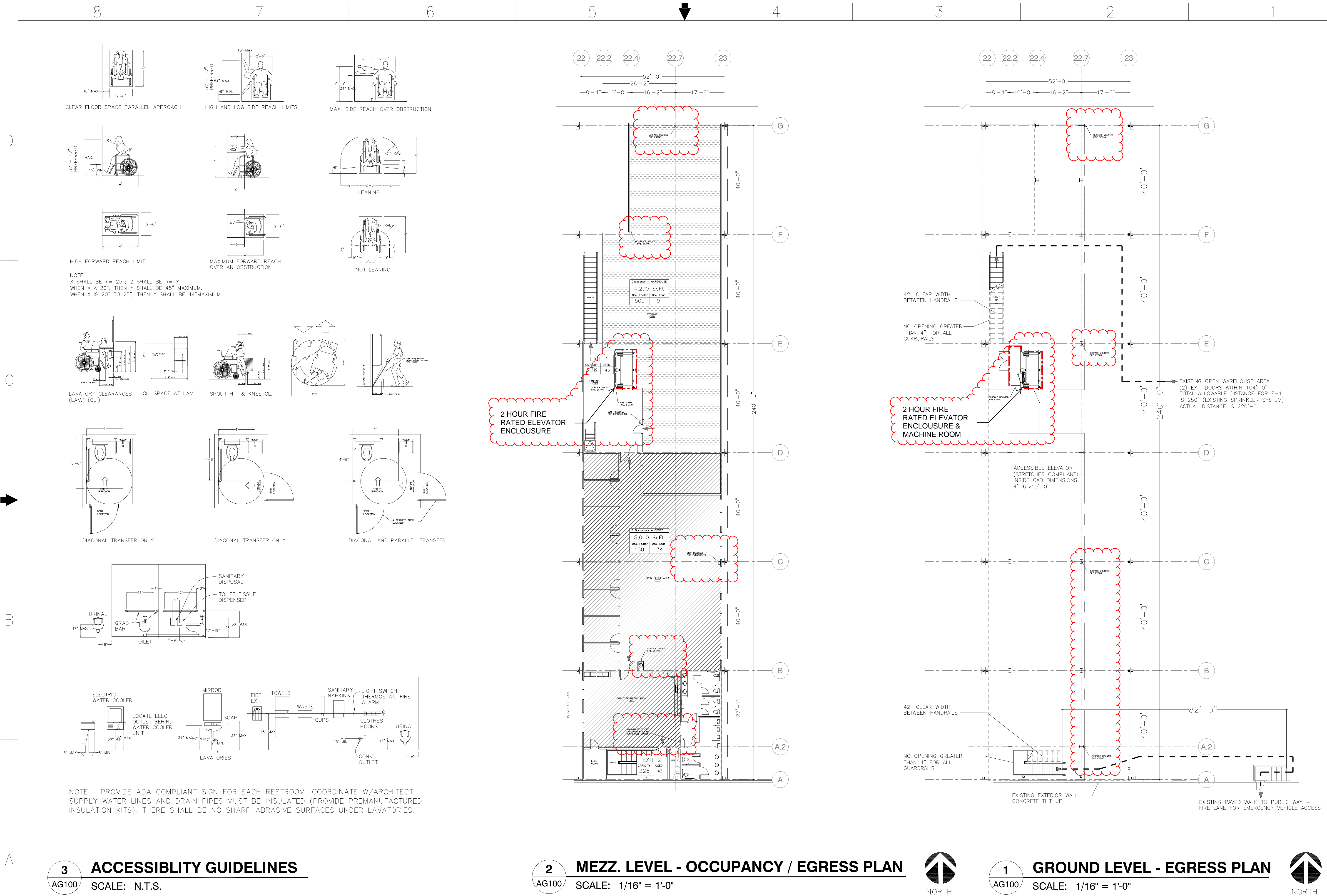
Drawn
GxA

Project No.
001-23

Sheet No.

AG001

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116



2 MEZZ. LEVEL - OCCUPANCY / EGRESS PLAN
SCALE: 1/16" = 1'-0"

1 GROUND LEVEL - EGRESS PLAN
SCALE: 1/16" = 1'-0"

OCCUPANCY - GRAPHIC LEGEND

- BUSINESS OCCUPANCY
- WAREHOUSE OCCUPANCY

GENERAL NOTES

- DO NOT SCALE DRAWINGS.
- NOT FOR CONSTRUCTION. FOR PRICING ONLY.
- CONTRACTOR TO VERIFY ALL QUANTITIES FOR TAKE OFF AND COST ESTIMATING.

3 ACCESSIBILITY GUIDELINES
SCALE: N.T.S.

ACCESSIBILITY GENERAL NOTES

- CONTRACTOR TO ALSO REFER TO ALL CURRENT FEDERAL AND STATE OF UTAH ADOPTED ACCESSIBILITY REQUIREMENTS.

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DIVISION 1 - GENERAL

1.1 GENERAL NOTES

- A. The intent of these drawings and specifications is to include all labour, materials and services necessary for the completion of all work shown, prescribed or reasonably implied, but not limited to that explicitly indicated in the contract documents.
- B. See Cover for all applicable codes.
- C. Contractor to submit shop drawings to Architect for review for all products being installed. The approval of shop drawings will not relieve the contractor from responsibility for deviations from the drawings or specifications unless he or she has (in writing) brought to attention such deviations, at the time of submission, nor will it relieve the contract of responsibility for errors of any sort in the shop drawings.
- D. Dimensions shown in the documents take precedence over all others. Under no circumstances will dimensions be scaled directly from drawings. Large scale drawings take precedence over small scale drawings. The contractor will notify the Architect of any discrepancies or conflicts.
- E. Floor elevations are as noted with a drop between existing slab and new slab.
- F. The general contractor and all subcontractors will verify all dimensions and conditions on the job site prior to beginning of construction and report any discrepancies to the architect.
- G. The contractor shall have 1 hardcopy of the approved construction drawings and building permit on site for use by all parties.
- H. The general contractor / subcontractors will furnish adequate shoring, bracing, barricades and protective measures, etc., required to safely protect the entire construction site and periphery, and will be fully responsible for on site construction safety and protocols per local and state requirements.
- I. All symbols and abbreviations used on the drawings are considered to be construction standards. If the contractor has questions regarding abbreviations or their exact meaning, the contractor will notify the Architect for clarifications.
- J. Details marked "typical" will apply in all cases unless specifically indicated otherwise.
- K. No substitutions to the documents will be allowed without prior written approval by the Architect.
- L. The contractor will provide protection for all pedestrians, site visitors, employees, consultants and all others within the project area.
- M. Contractor shall assume responsibility for securing all chattel and real property on the job site.
- N. Contractor shall assume responsibility for fire, theft, destruction, or vandalism on job site during construction.
- O. All walls, floors, doors, & door frames shall be constructed plumb, level and true, and rigidly connected as required by seismic requirements per local and state building codes.
- P. Contractor will verify with equipment manufacturers required pad size and base, as well as verify locations for all mechanical and electrical equipment and power, water and drain installations prior to proceeding with work.
- Q. The contractor shall verify all field measurements as necessary to insure proper fit of walls, doors, fixtures, and equipment.
- R. The contractor will provide to the Owner an operations manual for all equipment installed including all instructions, warranties and documents pertaining to the aforementioned equipment.
- S. Contact between dissimilar metals must be protected to prevent any deterioration due to galvanic action or corrosion.
- T. All installed materials and equipment shall bear a U.L., ASTM, I.C.B.O. or similarly accepted listing. All manufactured materials used must be installed bearing the appropriate labels identifying this fact.
- U. The Owner or Architect shall in no way be responsible for the means and methods of work performed, the safety in or about the site, performance of the work, or the timeliness in which the work is performed.
- V. The contractor shall not allow unauthorized personnel on the job site.
- W. All omissions and conflicts between the various elements of the drawings and/or specifications shall be brought to the attention of the Architect immediately before proceeding with any so work related to the possible omission or conflict. No changes are to be made unless the Architect and Owner is notified in writing.
- X. The contractor shall be responsible for the enforcement of Federal and State of Utah Occupational Safety and Health Administration requirements and regulations.

DIVISION 2 - SITE WORK

2.1 SOILS ENGINEERING

- A. All Load and Calculations are based on Existing Soils report. Contractor to verify with Architect if any diviation is discovered during construction.

2.2 GENERAL

- A. The contractor shall feild verify the location of all existing utility services in the area to be excavated prior to the beginning of excavation. Bluestakes call numbers include 811, 801-208-2100. Contractor shall cal 811 before digging. I
- B. Soil compaction and site preparation shall be in accordance with the certification of the letter from the soils / geotechnical engineer.
- C. Finish excavation for foundation shall be neat and true to line with loose material removed from excavation.
- D. The footing excavations shall be kept free from loose material and standing water.
- E. The contractor shall provide for dewatering of excavations for either surface water, seepage or ground water.
- F. The contractor shall provide and install all cribbing, sheathing and shoring required to safely retain any earth banks.
- G. The contractor shall brace or protect from lateral loads the foundation and retaining walls until attached floors or slabs are completely in place and have attained full strength.
- H. The contractor shall protect all utility and service lines encountered during excavation and backfilling. Damage to the lines caused during the course of construction to be repaired by contractor.
- I. Footing backfill and utility trench backfill within building area shall be mechanically compacted in layers to the approval of the Architect, Soils or Structural Engineer. Flooding is not be permitted.
- J. Subsurface drainage shall be provided behind retaining walls if retaining is installed.
- K. Contractor to protect all existing landscaping and retore any damaged landscaping in kind.
- L. The contractor shall coordinate all site work with the work of other trades to insure orderly progress of the total work and shall take precautionary measures to protect all underground work shown on the contract documents and other underground work not indicated.
- M. The site shall be cleared of all stumps, roots and other deleterious materials to a depth of not less than twelve inches below ground surfaces in the area to be occupied by the building or proposed structure.
- N. All horizontal walking surfaces to be continuous and without abrupt vertical changes exceeding ½" maximum. All horizontal walking surfaces will be maintained slip resistant.
- O. Any obstruction which overhangs a pedestrian's way will be a minimum of eighty inches above the walking surface as measured from the bottom of the obstruction.

DIVISION 3 - CONCRETE - SEE STRUCTURAL

DIVISION 4 - MASONRY - N/A

DIVISION 5 - STEEL - SEE STRUCTURAL.

DIVISION 6 - WOOD - N/A

DIVISION 7 - THERMAL AND MOISTURE PROTECTION

7.1 FLASHING

- A. All flashing, counterflashing, and coping when of metal shall be of not less than No. 26 U.S. gauge corrosion-resistant metal or as shown on plans or refer to SMACHA specifications.
- B. Flash and counterflash at all roof to wall conditions. G.I. flash and caulk wood beams and outlookers projecting through exterior walls or roof surfaces.
- C. Flash all exterior openings with approved waterproofing, which conforms to the standards of local and state codes. Flash at horizontal projections from vent, or battered exterior surface.
- D. Provide cap flashing on all exposed walls and at roof penetrations.
- E. Provide a butyl based flexible flashing or a liquid applied flashing system at all wall penetrations.

7.2 SEALANTS

- A. Caulk all flashings at doors, louvers and exterior openings. Sound caulk all top and bottom of walls on both sides at termination of gypsum board.

7.3 WATERPROOFING

- A. Provide "HENRY" or equal liquid applied waterproofing on exterior face of elevator foundation walls.

7.4 INSULATION / SOUND BATT

- A. All new walls run from new mezzanine concrete floor to underside of existing deck. Provide unfaced sound attenuation batt in all walls. See Wall Types for wall width.

7.5 ROOFING

- A. GC To submitt all roofing penetration locations to Architect for Review prior to cutting any roof items.
- B. GC to repair all roof openings with UL Listed matching materials in kind.

DIVISION 8 - DOORS, WINDOWS, AND GLAZINGS

- 8.1 - GLASS - All interior windows to include insulated glass per approved submittal.

8.2 WEATHERSTRIPPING AND THRESHOLDS

- A. All doors shall be weatherstripped for sound attenuation and protection.
- B. Provide weatherstripping completely around doors.

8.3 HOLLOW METAL DOORS

GENERAL

American National Standards Institute/Steel Door Institute (ANSI/A250.8) "Recommended Specifications for Standard Steel Doors and Frames." National Fire Protection Association (N.F.P.A.) #80 Standard for Fire Doors and Fire Windows, Steel Door Institute (S.D.I.) all publication standards. Door Hardware Institute (D.H.I.) all publication standards. U.L. 10C, fire listed products. American Society for Testing Materia(A.S.T.M.)A1008/A1008MStandard Specification for Steel, Sheet, Cold-Rolled, Carbon, Structural, High-Strength Low-Alloy and High-Strength Low-Alloy with Improved FormabilityA568/A568MStandard Specification for Steel, Sheet, Carbon, and High-Strength, Low-Alloy, Hot-Rolled and Cold-Rolled, General Requirements forA924/A924MStandard Specification for General Requirements for Steel Sheet, Metallic-Coated by the Hot-Dip ProcessA653/A653MStandard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process.

Shop Drawings: Submit for fabrication and installation of steel doors and frames. Include details of each frame type, elevations of door design types, conditions at openings, details of construction, location and installation requirements of finish hardware and reinforcements, and details of joints and connections. Show anchorage and accessory items required for installation. Submit manufacturer's technical product data substantiating that products comply with requirements.

Provide Fire Rated Doors as Listed on the Door Schedule. Delivery Storage and Handling: For welded frames, provide channel steel shipping spreader welded to the bottom of each jamb at door opening to prevent damage in transit and jobsite handling. Shipping spreader shall be removed and replaced with a "setting" spreader at bottom of frame at time of frame installation (see "Installation"). General contractor shall inspect hollow metal work upon delivery for damage. Minor damage may be repaired provided refinished items are equal in all respects to new work and acceptable to architect; otherwise, remove and replace damaged items as required.

PRODUCTS

Manufacturers: CURRIES Company or approved equal. Steel Requirements: All doors and frames to be manufactured of commercial quality, stretcher leveled flatness, cold rolled steel per ASTM A1008 and A568 general requirements or galvanized steel sheet shall be as per ASTM A924 or A653 hot dip galvanized to A60 minimum coating weight standard. Internal reinforcing may be manufactured of hot rolled pickled and oiled steel per ASTM A1011.

Steel Doors: Comply with ANSI/A250.8/SDI 100. Provide metal doors in 20, 18, 16, or 14 gauge steel as (cont.) specified on plans in accordance with performance levels defined in ANSI/A250.8/SDI 100 and ANSI A250.4

Test Procedure and Acceptance Criteria for Physical Endurance. Door size cycle tested to be 4070 to

EXECUTION

Erection Installation: Install hollow metal units in accordance with manufacturer's instructions and final shop drawings. Fit doors to frames and floors with proper clearances and to achieve the maximum operational effectiveness and appearance of each unit. S.D.I. 122 "Installation and Troubleshooting Guide for Standard Steel Doors and Frames".

ROLL UP SERVICE DOORS.

- A. ANSI/DASMA 108 - American National Standards Institute Standard Method For Testing Sectional Garage Doors And Rolling Doors: Determination Of Structural Performance Under Uniform Static Air Pressure Difference.
- B. NFRC 102 - Test Procedure for Measuring the Steady-State Thermal Transmittance of Fenestration Systems.
- C. ASTM E 90 - Standard Test Method for Laboratory Measurement of Airborne Sound Transmission Loss of Building Partitions and Element. ASTM E 330 - Standard Test Method for Structural Performance of Exterior Windows, Doors, Skylights and Curtain Walls by Uniform Static Air Pressure Difference. ASTM A 653 - Standard Specification for Steel Sheet, Zinc-Coated (Galvanized) or Zinc-Iron Alloy-Coated (Galvannealed) by the Hot-Dip Process. ASTM A 924 - Standard Specification for General Requirements for Steel Sheet, Metallic-Coated by the Hot-Dip Process.
- D. NEMA 250 - Enclosures for Electrical Equipment (1000 Volts Maximum).
- E. NEMA MG 1 - Motors and Generators.

DESIGN / PERFORMANCE REQUIREMENTS

- A. Single-Source Responsibility: Provide doors, tracks, motors, and accessories from one manufacturer for each type of door. Provide secondary components from source acceptable to manufacturer of primary components.
- B. Products Requiring Electrical Connection: Listed and classified by Underwriters Laboratories, Inc. acceptable to authority having jurisdiction as suitable for purpose specified.

SUBMITTALS

- C. Product Data: Manufacturer's data sheets on each product to be used, including:
1. Preparation instructions and recommendations.
 2. Storage and handling requirements and recommendations.
 3. Details of construction and fabrication.
 4. Installation instructions.
- C. Shop Drawings: Include detailed plans, elevations, details of framing members, anchoring methods, required clearances, hardware, and accessories. Include relationship with adjacent construction.

MANUFACTURER

Overhead Door Company or approved equal.

DIVISION 9 - FINISHES - See below and See A200 & A201.

9.1 EXTERIOR METALS & MATERIALS - N/A

9.2 INTERIOR FINISH MATERIALS

- A. Gypsum wallboard:
1. All gypsum wallboard shall be TYPE X or Equal and installed in accordance with the provisions of the 2018 IBC,applicable edition, State and local codes.
 2. All walls to be 5/8" UL Listed gypsum wallboard unless otherwise noted. Refer to floor plans and sections for fire rated walls and assemblies.
 4. The size & spacing of fasteners shall comply w/ 2018 IBC, State and local building codes.
 3. Provide metal trim at all exposed edges and external corners. Metal trim shall be galvanized.

B. ADDITIONAL FINISH MATERIALS ON FOLLOWING PAGES.

DIVISION 10 - SPECIALTIES

10.1 - 60 Degree Ships Ladder (Roof Access)

Follow ASTM B 209 - Standard Specification for Aluminum and Aluminum-Alloy Sheet and Plate. ASTM B 221 - Standard Specification for Aluminum and Aluminum-Alloy Extruded Bars, Rods, Wire, Profiles, and Tubes. OSHA 1910.27 - Fixed Ladders.

SUBMITTALS

Shop Drawings: Detail fabrication and erection of each ladder indicated. Include plans, elevations, sections, and details of metal fabrications and their connections. Provide templates for anchors and bolts specified for installation under other Sections. Provide reaction loads for each hanger and bracket.

MANUFACTURERS

1. O'Keefe's, Inc. (Tel: (415) 824-4900; info@okeeffes.com. Web: http://www.okeeffes.com.
2. National Ladder & Scaffold Co.; Upnovr; or approved equal.

WARRANTY

A. Manufacturer has responsibility for an extended Corrective Period for work of this Section for a period of 10 years commencing on the shipment date of the product against all the conditions indicated below, and when notified in writing from Owner, manufacturer shall promptly and without inconvenience and cost to Owner correct said deficiencies.

Defects in materials and workmanship.

Deterioration of material and surface performance below minimum OSHA standards as certified by independent third party testing laboratory. Ordinary wear and tear, unusual abuse or neglect excepted.

Within the warranty period, the manufacturer shall, at its option, repair, replace, or refund the purchase price of defective ladder.

Manufacturer shall be notified immediately of defective products, and be given a reasonable opportunity to inspect the goods prior to return. Manufacturer will not assume responsibility, or compensation, for unauthorized repairs or labor. Manufacturer makes no other warranty, expressed or implied, to the merchantability, fitness for a particular purpose, design, sale, installation, or use, of the ladder; and shall not be liable for incidental or consequential damages, losses of or expenses, resulting from the use of ladder products.

EXTRA MATERIALS

Furnish touchup kit for each type and color of paint finish provided.

Subconsultants
Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

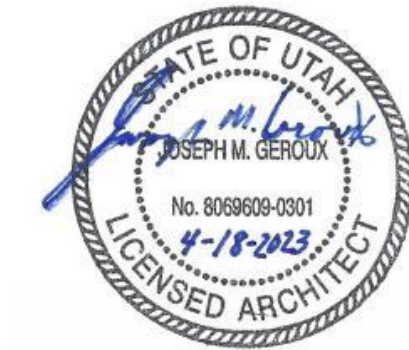
Van Boerum & Frank Assoc.
181 E. 5600 S.
Murray, Utah 84107

Hunt Electric, Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000



Issued/Revisions

No.	Description	Date
1	PERMIT SUBMISSION	04/18/22
2		
3		
4		
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6		

Notes:
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Project Name
ALBANY MECHANICAL

Sheet Title
**GENERAL NOTES
& SPECIFICATIONS**

Scale
N/A

Date
04.18.2023

Drawn
GxA

Project No.
001-23

Sheet No.

AG002

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

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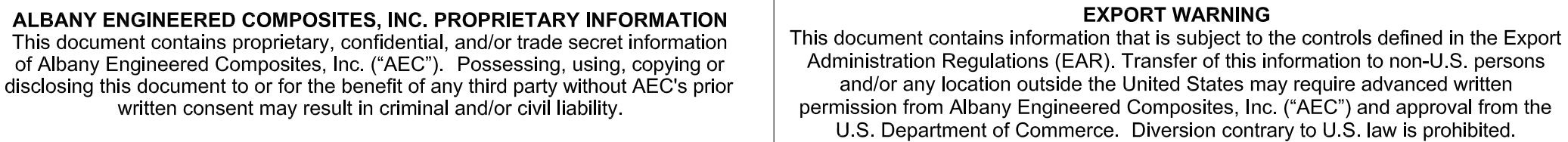
Sheet Title

EXISTING FLOOR
PLAN / SITE PLAN
PARKING CALCS

Drawn GxA	Project No. 001-23
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Sheet No. _____

A100



1. DO NOT SCALE DRAWINGS.
2. CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES.
3. CONTRACTOR TO VERIFY ALL VERTICAL CLEARANCES TO EXISTING JOISTS, GIRDERS.
4. CONTRACTOR TO PROVIDE SUBMITTAL DOCUMENTS TO ARCHITECT FOR REVIEW.
5. CONTRACTOR TO COORDINATE WITH OWNER FOR ALL CONSTRUCTION STAGING, SEQUENCING, PHASING.
6. CONTRACTOR TO PROTECT ALL CONSTRUCTION AREAS AND EXISTING FACILITY OPERATIONS.
7. CONTRACTOR TO CONFORM TO ALL OWNER REQUIRED PROVISIONS / WORK REQUIREMENTS.

No.	Description	Date
1	PERMIT SUBMISSION	04/18/23
2	PERMIT RE-SUBMISSION	05/18/23
3		
4		
5		
6		

Notes:
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Project Name
ALBANY MEZZANINE

Sheet Title
FLOOR PLANS
WALL ASSEMBLIES

Scale
1/16"=1'-0"

Date
04.18.2023

Drawn
GxA

Project No.
001-23

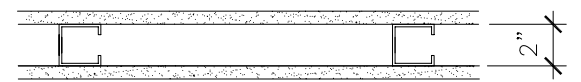
A200

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

P-2

UL - U419

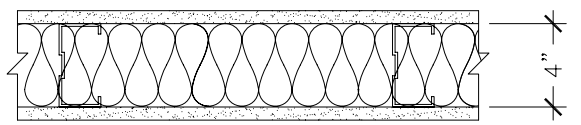
5/8" TYPE X GYPSUM SHEATHING
2" METAL STUDS (MIN. 20 GA.) @ 16" O.C.
5/8" TYPE X GYPSUM WALLBOARD
(START WALL 8" ABOVE F.F.)



P-4

UL - U419

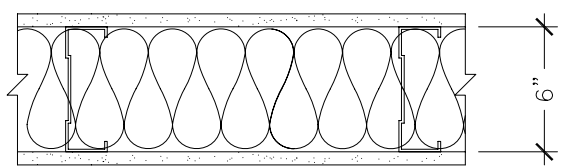
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4" METAL STUDS (MIN. 20 GA.) @ 16" O.C.
R13 SOUND BATT INSULATION IN CAVITY
5/8" TYPE X GYPSUM WALLBOARD



P-6

UL - U419

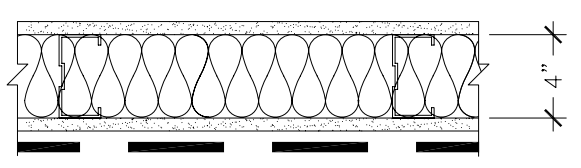
5/8" TYPE X GYPSUM SHEATHING
4" METAL STUDS (MIN. 20 GA.) @ 16" O.C.
R13 SOUND BATT INSULATION IN CAVITY
5/8" TYPE X GYPSUM WALLBOARD



P-4A

UL - U419

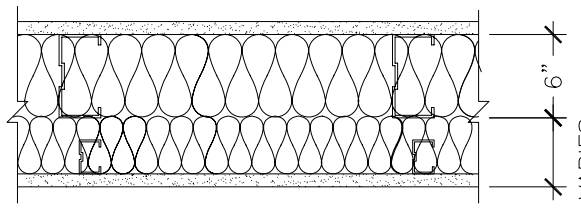
5/8" TYPE X GYPSUM WALLBOARD
4" METAL STUDS (MIN. 20 GA.) @ 16" O.C.
R13 SOUND BATT INSULATION IN CAVITY
5/8" TYPE X MOSITURE RESISTANT GYPSUM WALLBOARD
SEE PLAN NOTES FOR WALL FINISH - FRP IN JANITORS CLOSET
& FULL HEIGHT CERAMIC TILE IN RESTROOMS



P-6A

UL - U419

5/8" TYPE X GYPSUM WALLBOARD
6" METAL STUDS (MIN. 20 GA.) @ 16" O.C.
R13 SOUND BATT INSULATION IN CAVITY
5/8" TYPE X GYPSUM WALLBOARD
SEE PLAN NOTES - FURRING WITH
ADDITIONAL SOUND BATT AS SHOWN



NOTE: JANITORS CLOSET 224 WALLS SHALL BE WASHABLE FRP
PANELS TO 8'-0". COORDINATE HEIGHT w/ EXISTING TRUSS LOCATIONS

3

WALL TYPE ASSEMBLIES

A200

SCALE: N.T.S.

DESIGN NO. UL U419

FIRE RATING: 1 HOUR
STUD: 20 GA. @ 16" O.C.
SYSTEM THICKNESS: 5-1/2" (141MM)
LOCATION: INTERIOR
FRAMING TYPE: STEEL STUD (NAILING BEARING)

ASSEMBLY REQUIREMENTS:

GYPSUM PANELS: TWO LAYERS 5/8" (15.8 MM) SHEETROCK ECOMART GYPSUM PANEL (UL TYPE ULX17)
STEEL STUDS: 20 GA. @ 16" O.C.
INSULATION: 5-1/2" (141 MM) FIBERGLASS INSULATION
GYPSUM PANELS: ONE LAYER 5/8" (15.8 MM) SHEETROCK ECOMART GYPSUM PANEL (UL TYPE ULX17)

GENERAL NOTES:

1. REFER TO APPLICABLE CODES REQUIREMENTS TO ENSURE COMPLIANCE PRIOR TO CONSTRUCTION.
2. FOR THE MOST UP-TO-DATE DETAILS, INCLUDING CONSTRUCTION VARIATIONS, REFER TO THE PUBLISHED DESIGN.
3. WHERE DESIGN INDICATES "TYP", THE FIRE RATING IS BASED ON LABORATORY TEST DATA OF THE REFERENCED ASSEMBLY.
4. STUD SIZE AND INSULATION THICKNESS ARE MINIMUM UNLESS OTHERWISE STATED IN THE PUBLISHED ASSEMBLY.
5. STUD SIZE AND INSULATION THICKNESS ARE MINIMUM UNLESS OTHERWISE STATED IN THE PUBLISHED ASSEMBLY.
6. STUD SIZE AND INSULATION THICKNESS ARE MINIMUM UNLESS OTHERWISE STATED IN THE PUBLISHED ASSEMBLY.
7. STUD SIZE AND INSULATION THICKNESS ARE MINIMUM UNLESS OTHERWISE STATED IN THE PUBLISHED ASSEMBLY.
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9. STUD SIZE AND INSULATION THICKNESS ARE MINIMUM UNLESS OTHERWISE STATED IN THE PUBLISHED ASSEMBLY.
10. STUD SIZE AND INSULATION THICKNESS ARE MINIMUM UNLESS OTHERWISE STATED IN THE PUBLISHED ASSEMBLY.

UL U419

SN-P-1-11

4

UL WALL ASSEMBLY

A200

SCALE: N.T.S.

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2

MEZZANINE LEVEL - FLOOR PLAN

A200

SCALE: 1/16" = 1'-0"

FLOOR FINISH MATERIAL

1. LANDING 200 & STORAGE 202 = SEALED CONCRETE. PROVIDE ALTERNATE PRICE FOR PATCRAFT ADMIX VCT - 36"x36" - COLOR CLASSIC 00119-V1.
2. ALL OFFICE AREAS = PATCRAFT HEIRLOOM TWEED - COLOR 00530 SHADOW LOOP.
3. EMPLOYEE BREAKROOM 220 = PATCRAFT ADMIX VCT - 36"x36" - COLOR CLASSIC 00119-V1 BORDER w/ INSET COLOR SOAR 00437-V1
4. RESTROOMS = DAL-TILE 18"x24" CERAMIC TILE. ARCHITECT TO SELECT.
5. 4" RUBBER BASE (THROUGHOUT) = PATCRAFT - Totalworx - COLOR 66 SLATE
6. NOTE ABOVE PLAN FOR MISC. ITEMS

PATCRAFT CONACT: ZACHARY HAYS - 385.499.0965 - ZACHARY.HAYS@PATCRAFT.COM



1

GROUND LEVEL - FLOOR PLAN

A200

SCALE: 1/16" = 1'-0"

GRAPHIC LEGEND

CLEAR FLOOR AREA - CIRCULATION WAY

GENERAL NOTES

1. DO NOT SCALE DRAWINGS.
2. CONTRACTOR TO FIELD VERIFY ALL DIMENSIONS AND NOTIFY ARCHITECT OF ANY DISCREPANCIES.
3. CONTRACTOR TO VERIFY ALL VERTICAL CLEARANCES TO EXISTING JOISTS, GIRDERS.
4. CONTRACTOR TO PROVIDE SUBMITTAL DOCUMENTS TO ARCHITECT FOR REVIEW.
5. CONTRACTOR TO COORDINATE WITH OWNER FOR ALL CONSTRUCTION STAGING, SEQUENCING, PHASING.
6. CONTRACTOR TO PROTECT ALL CONSTRUCTION AREAS AND EXISTING FACILITY OPERATIONS.
7. CONTRACTOR TO CONFORM TO ALL OWNER REQUIRED PROVISIONS / WORK REQUIREMENTS.





Issued/Revisions

No.	Description	Date
1	PERMIT SUBMISSION	04/18/23
2		
3		
4		
5		
6		

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Project Name
ALBANY MEZZANINE

Sheet Title
**REFLECTED
CEILING PLANS
DETAILS**

Scale 1/16"=1'-0"	Date 04.18.2023
Drawn GxA	Project No. 001-23

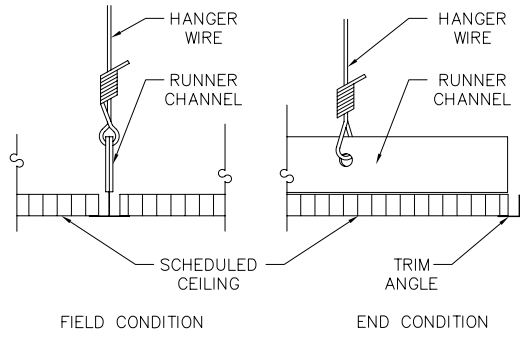
Sheet No.

A201

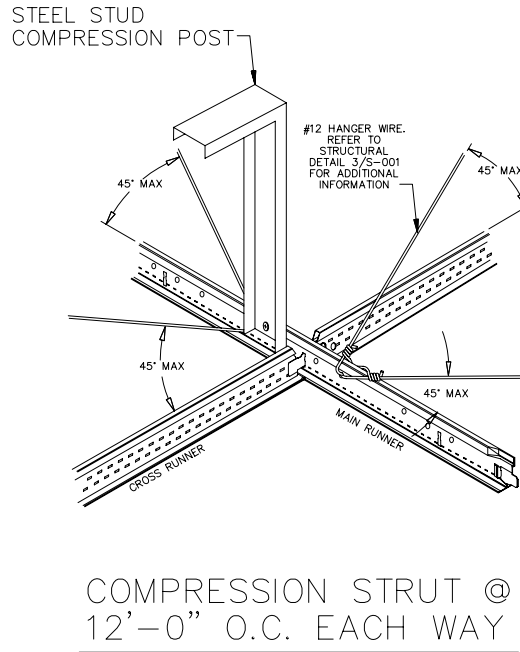
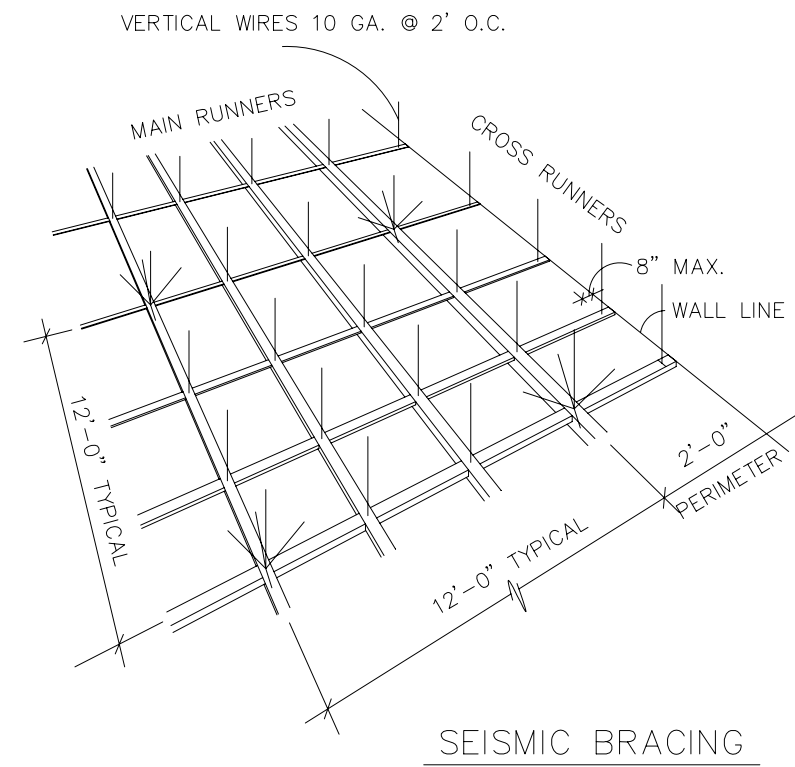
ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

CEILING BRACING NOTES:

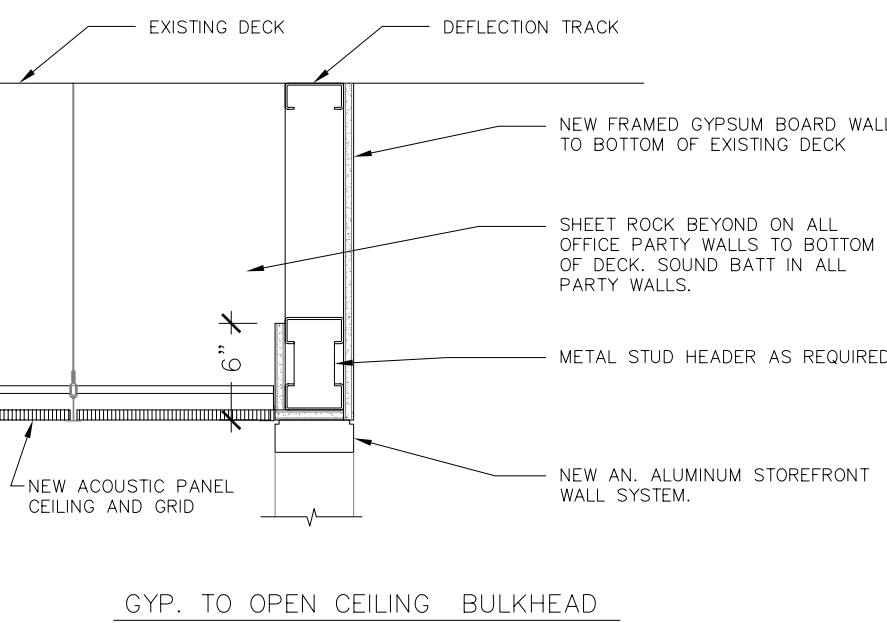
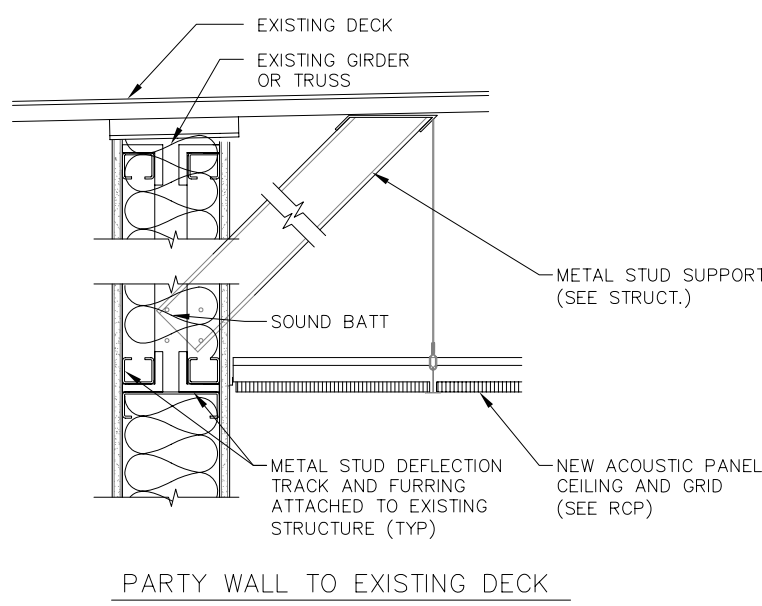
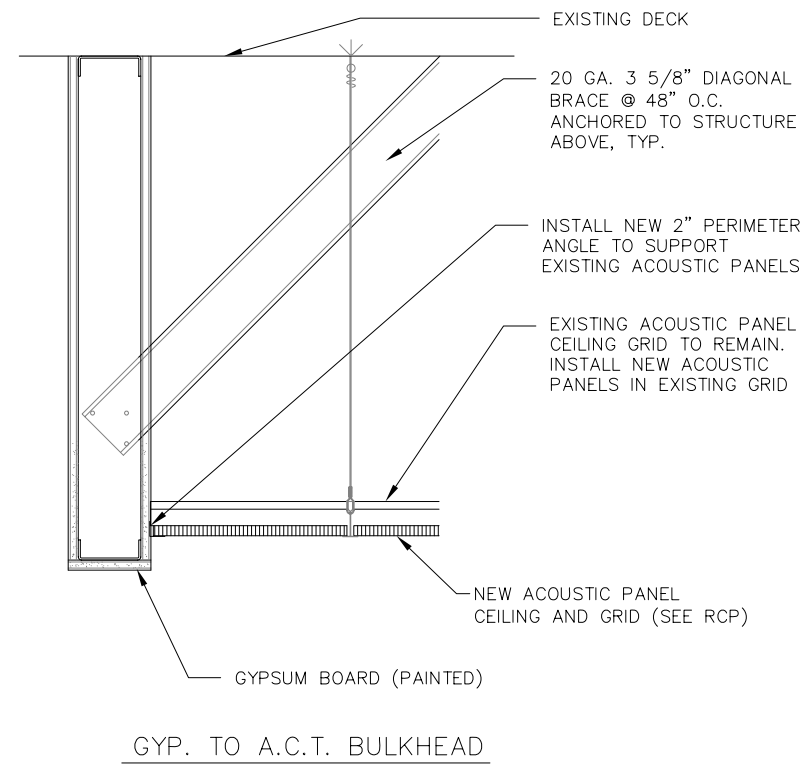
- 1.A CEILING AREA OF 144 SQ. FT. OR LESS SURROUNDED BY WALLS THAT CONNECT DIRECTLY TO THE STRUCTURE ABOVE SHALL BE EXEMPT FROM LATERAL LOAD DESIGN REQUIREMENTS OF THESE STANDARDS.
- 2.IN EACH ORTHOGONAL DIRECTION, ONE END OF THE CEILING SHALL BE ATTACHED WITH AN ICC EVALUATED & APPROVED SEISMIC CLIP SYSTEM AND 0.75" OF CLEARANCE TO ALLOW FREE HORIZONTAL MOVEMENT.
- 3.LATERAL CEILING BRACING IS REQUIRED @ 12'-0" O.C. IN BOTH DIRECTIONS FOR ALL CEILINGS GREATER THAN 1,000 S.F.
- 4.LIGHT FIXTURES, MECHANICAL EQUIPMENT, ETC. MUST BE SUPPORTED INDEPENDENT OF THE CEILING SUPPORT / BRACKETING SYSTEM.



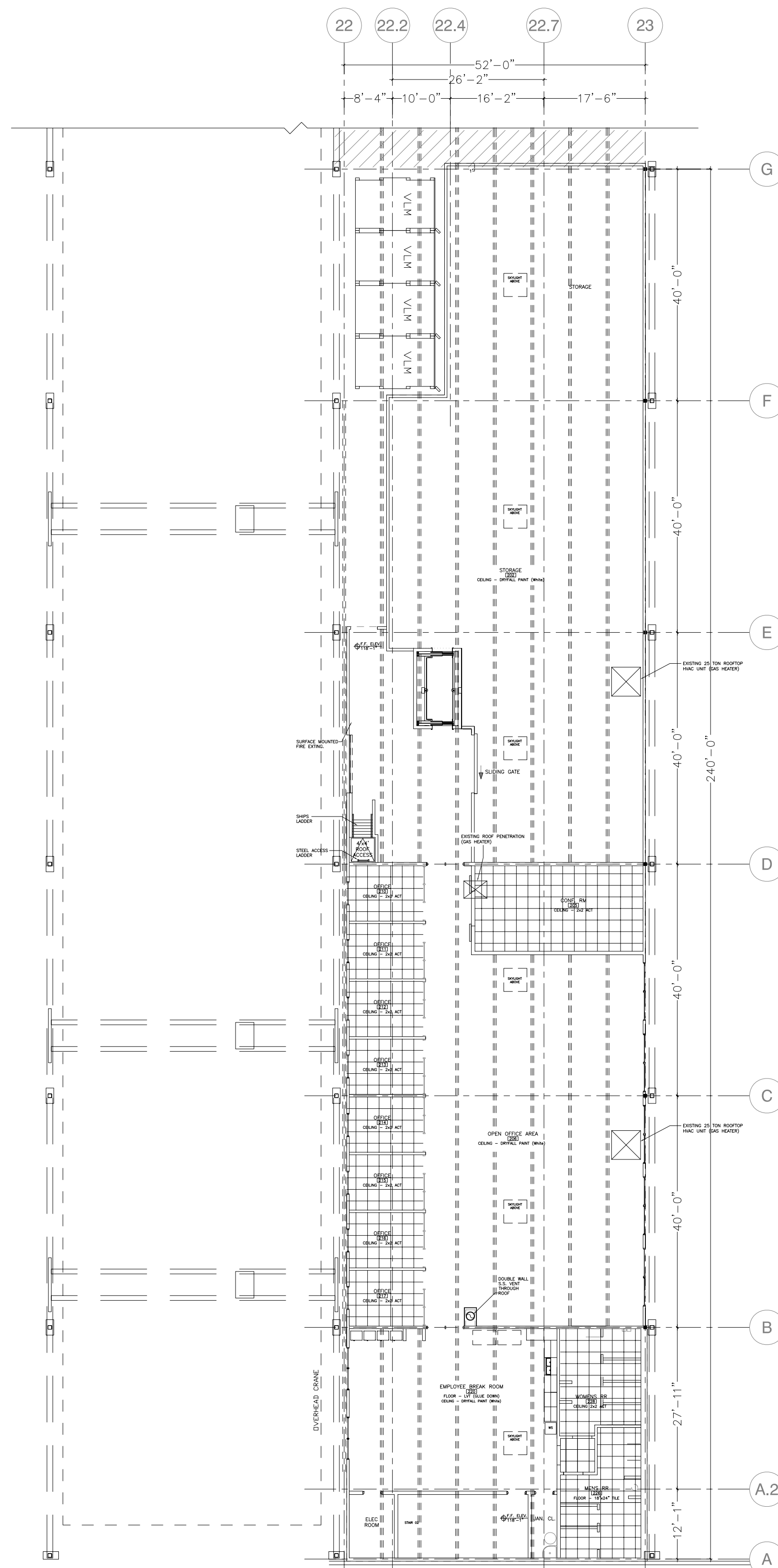
ACOUSTICAL TILE



4 ACOUSTICAL TILE DETAILS
A201 SCALE: N.T.S.



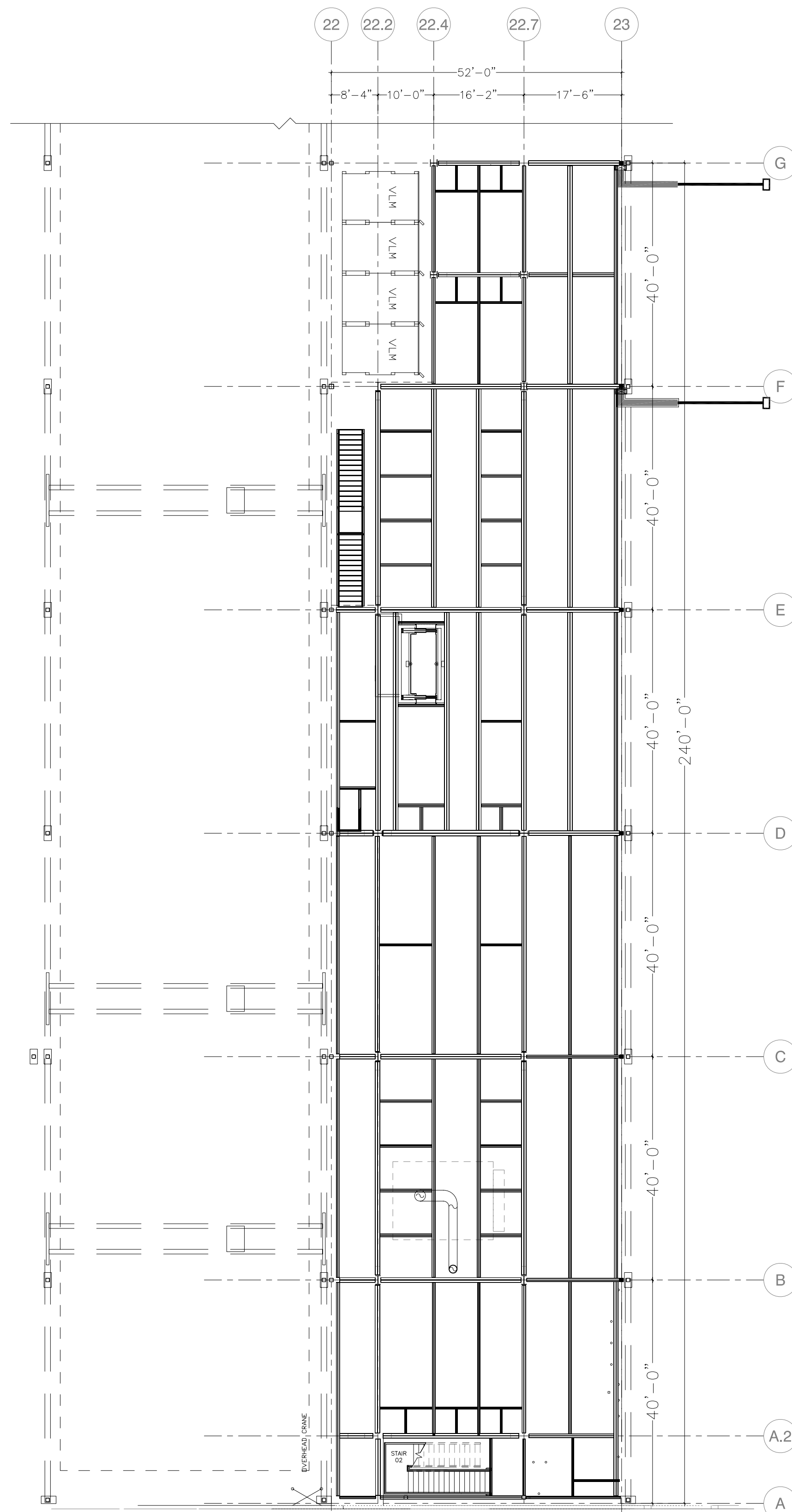
3 CEILING DETAILS
A201 SCALE: 1"=1'-0"



2 MEZZ. LEVEL - REF. CEILING PLAN
A201 SCALE: 1/16" = 1'-0"

CEILING FINISH MATERIALS

1. ACOUSTICAL CEILING TILES SHALL BE ARMSTRONG DUNE 2x2 (3/8) BEVELED TEGULAR (3/8).
2. ALL CEILING PAINT TO BE WHITE (DRYFALL).
3. ALL EXPOSED DUCTWORK TO BE SPIRAL.
4. ELEVATOR CAB CEILING TO BE STAINLESS STEEL.



1 GROUND LEVEL - REF. CEILING PLAN
A201 SCALE: 1/16" = 1'-0"

GENERAL NOTES

1. DO NOT SCALE DRAWINGS.
2. NOT FOR CONSTRUCTION. FOR PRICING ONLY.
3. CONTRACTOR TO VERIFY ALL QUANTITIES FOR TAKE OFF AND COST ESTIMATING.
4. SEE ELECTRICAL PLANS FOR LIGHTING.
5. SEE MECHANICAL PLANS FOR GRILLES, LOUVERS,VENTS.



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Subconsultants
Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Van Boerum & Frank Assoc.
181 E. 5600 S.
Murray, Utah 84107

Hunt Electric, Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

Issued/Revisions

No.	Description	Date
1	PERMIT SUBMISSION	04/18/23
2	PERMIT RE-SUBMISSION	05/18/23
3		
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Project Name
ALBANY MECHANICAL

Sheet Title
ROOF PLAN

Scale
AS NOTED

Date
04.18.2023

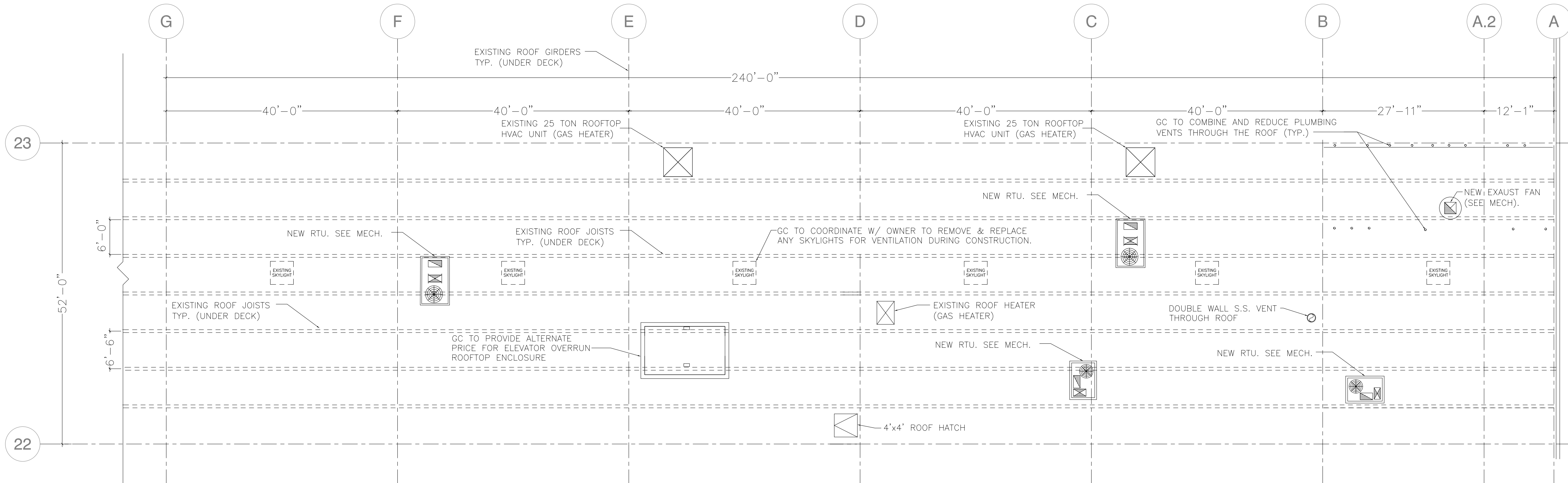
Drawn
GxA

Project No.
001-23

Sheet No.

A202

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

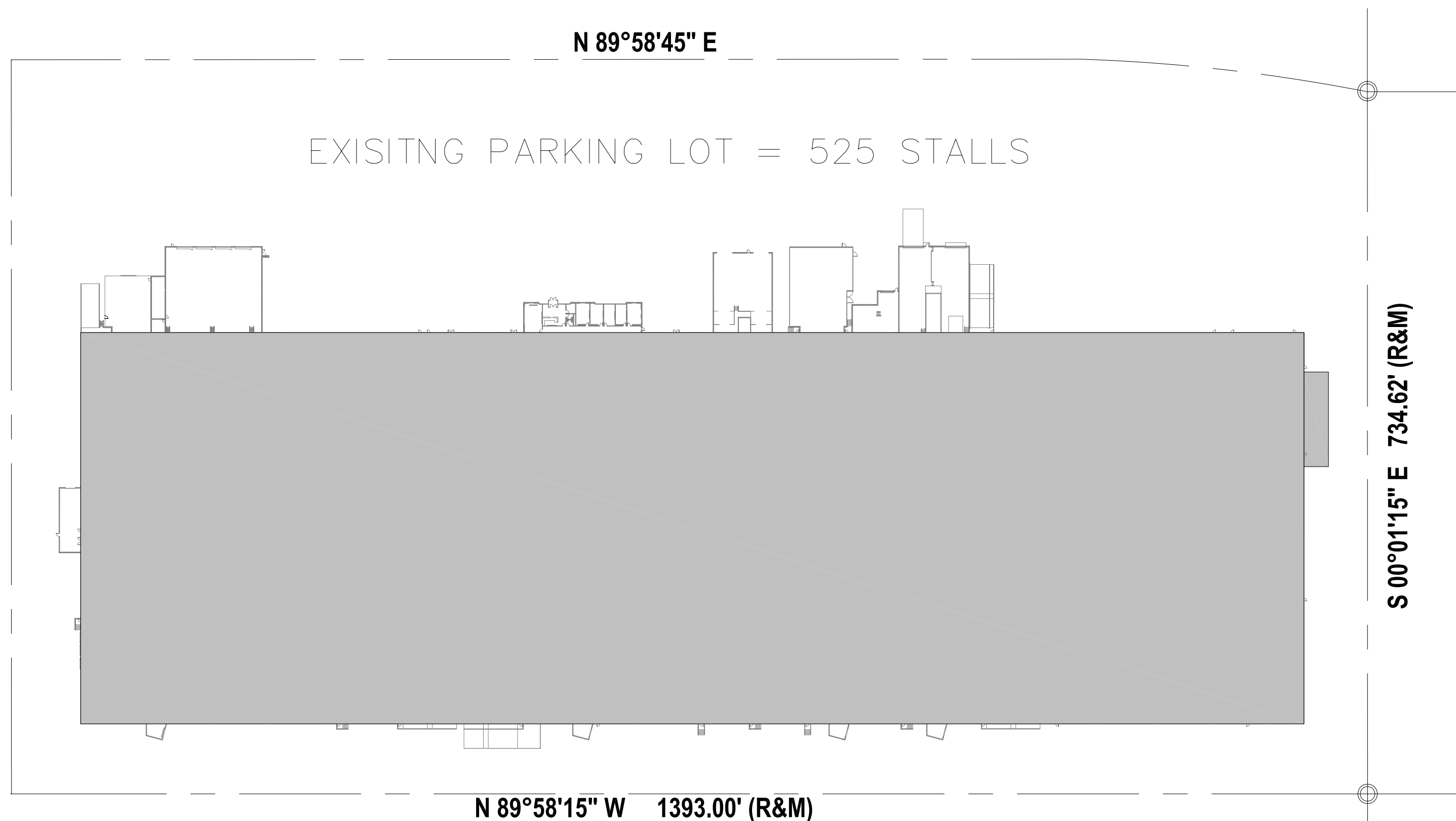


2 ENLARGED ROOF PLAN - ABOVE MEZZ.
A202 SCALE: 3/32" = 1'-0"



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- CONTRACTOR TO LEAVE JOB SITE CLEAN OF ALL DEBRIS AT ALL TIMES. COORDINATE WITH ALBANY FACILITIES DIRECTOR ON CONSTRUCTION STAGING AREA, MITIGATION PLAN AND DUMPSTER LOCATION AS NECESSARY. BUILDING OWNER TO PROVIDE RESTROOM FOR ANY CONSTRUCTION WORKERS.
- CONTRACTOR TO COORDINATE WITH LOCAL FIRE MARSHALL AS REQUIRED.
- COORDINATE ADDITIONAL DEFERRED FIRE SPRINKLER SUBMITTAL AS REQUIRED WITH FIRE MARSHALL AND AUTHORITY HAVING JURISDICTION.



1 OVERALL ROOF PLAN - ACCESS ROUTE
A202 SCALE: N.T.S



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Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

Issued/Revisions

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Project Name
ALBANY MECHANICAL

Sheet Title

**Enlarged
Floor Plan**

Scale
1/4" = 1'-0"

Date
04.18.2023

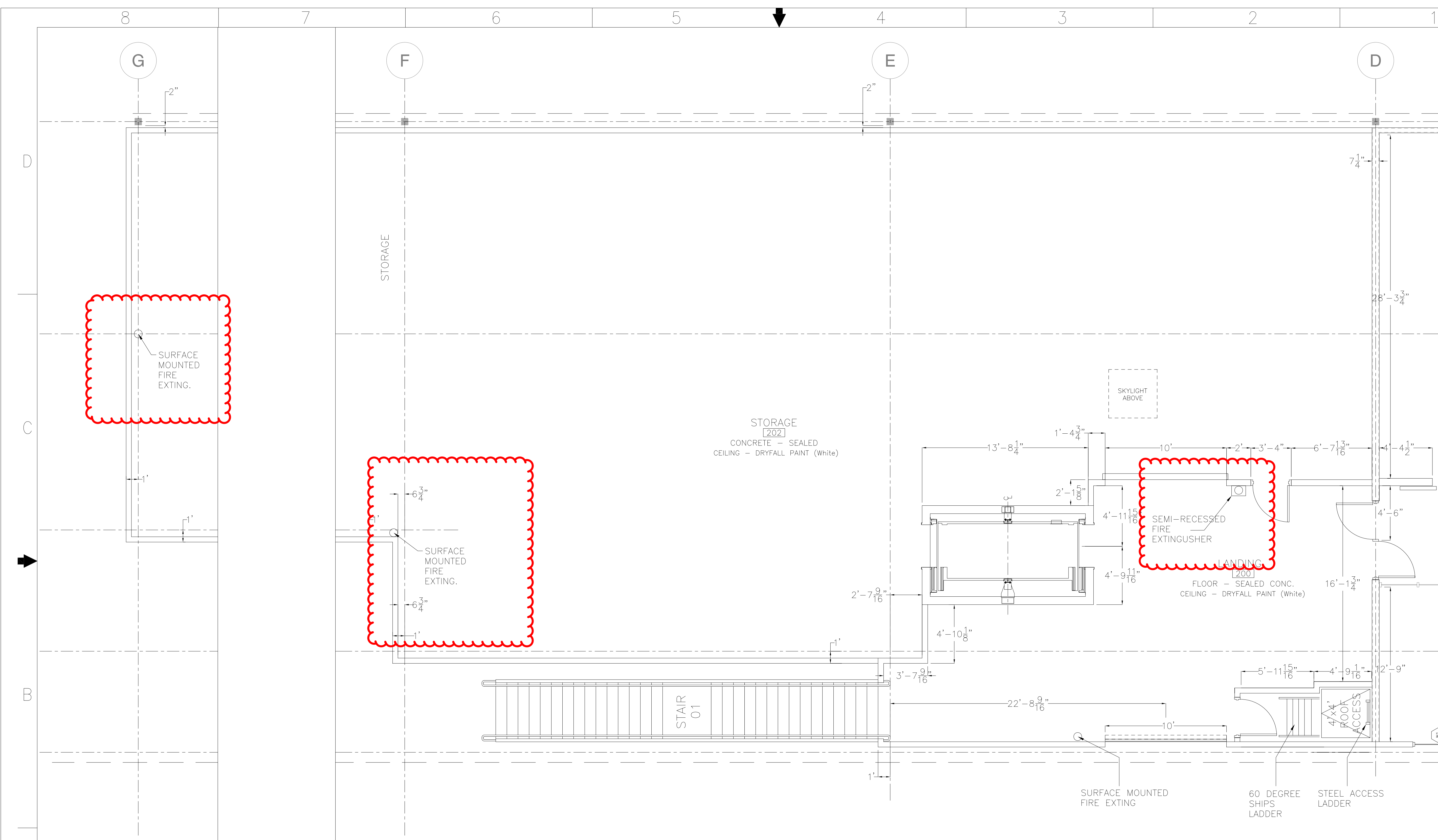
Drawn
GxA

Project No.
001-23

Sheet No.

A203

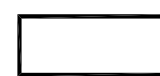
ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116



1 ENLARGED FLOOR PLAN - MEZZ. NORTH
SCALE: 1/4" = 1'-0"



GRAPHIC LEGEND



GENERAL NOTES

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Project Name
ALBANY MECHANICAL

Sheet Title

**Enlarged
Floor Plan**

Scale
1/4" = 1'-0"

Date
04.18.2023

Drawn
GxA

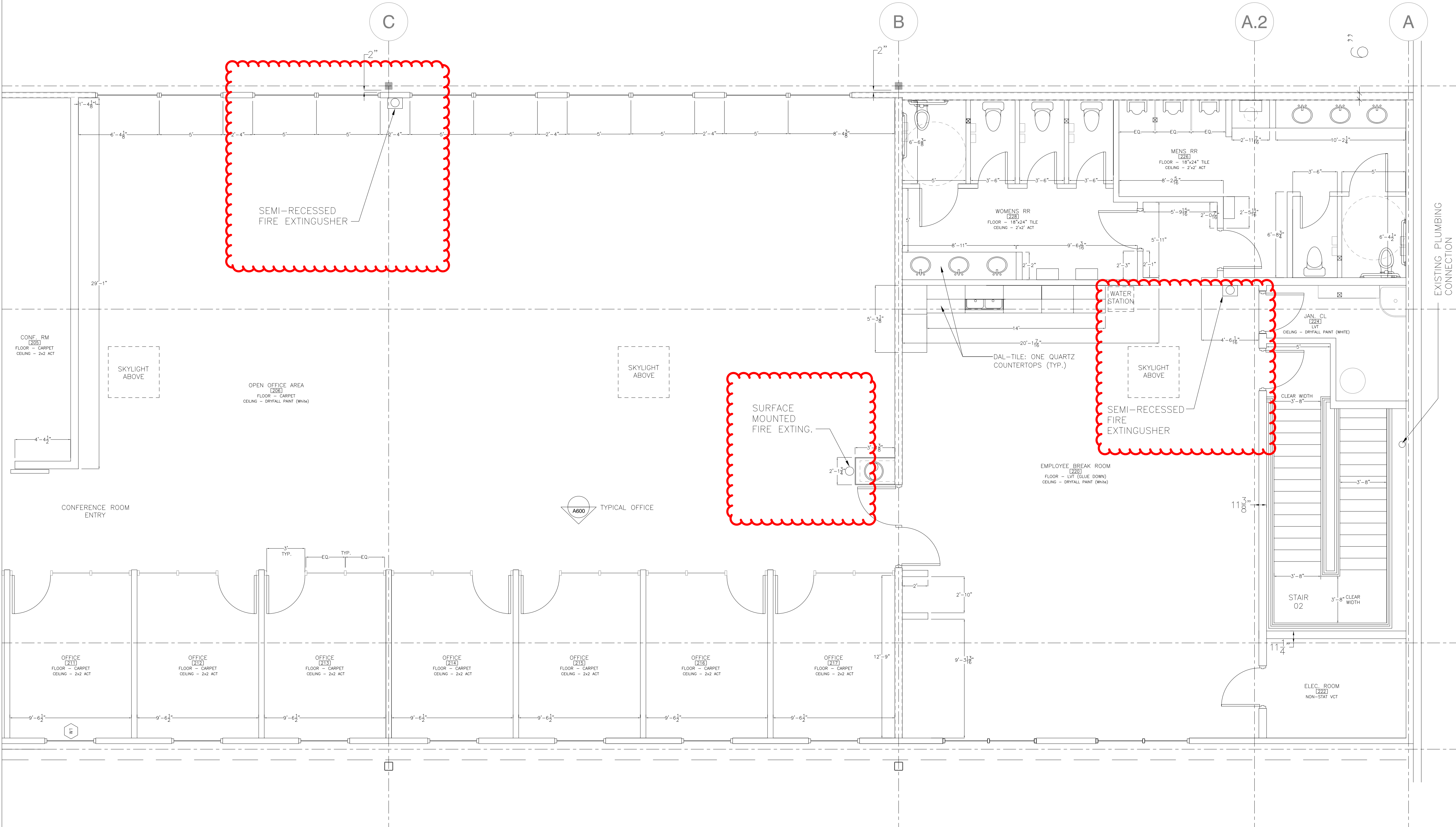
Project No.
001-23

Sheet No.

A204

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

8 7 6 5 4 3 2 1



1 ENLARGED FLOOR PLAN - MEZZ. SOUTH
A204 SCALE: 1/4" = 1'-0"

GRAPHIC LEGEND



BATHROOM SPECIFICATIONS (ALSO SEE PLUMBING)

- DO NOT SCALE DRAWINGS.
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Issued/Revisions

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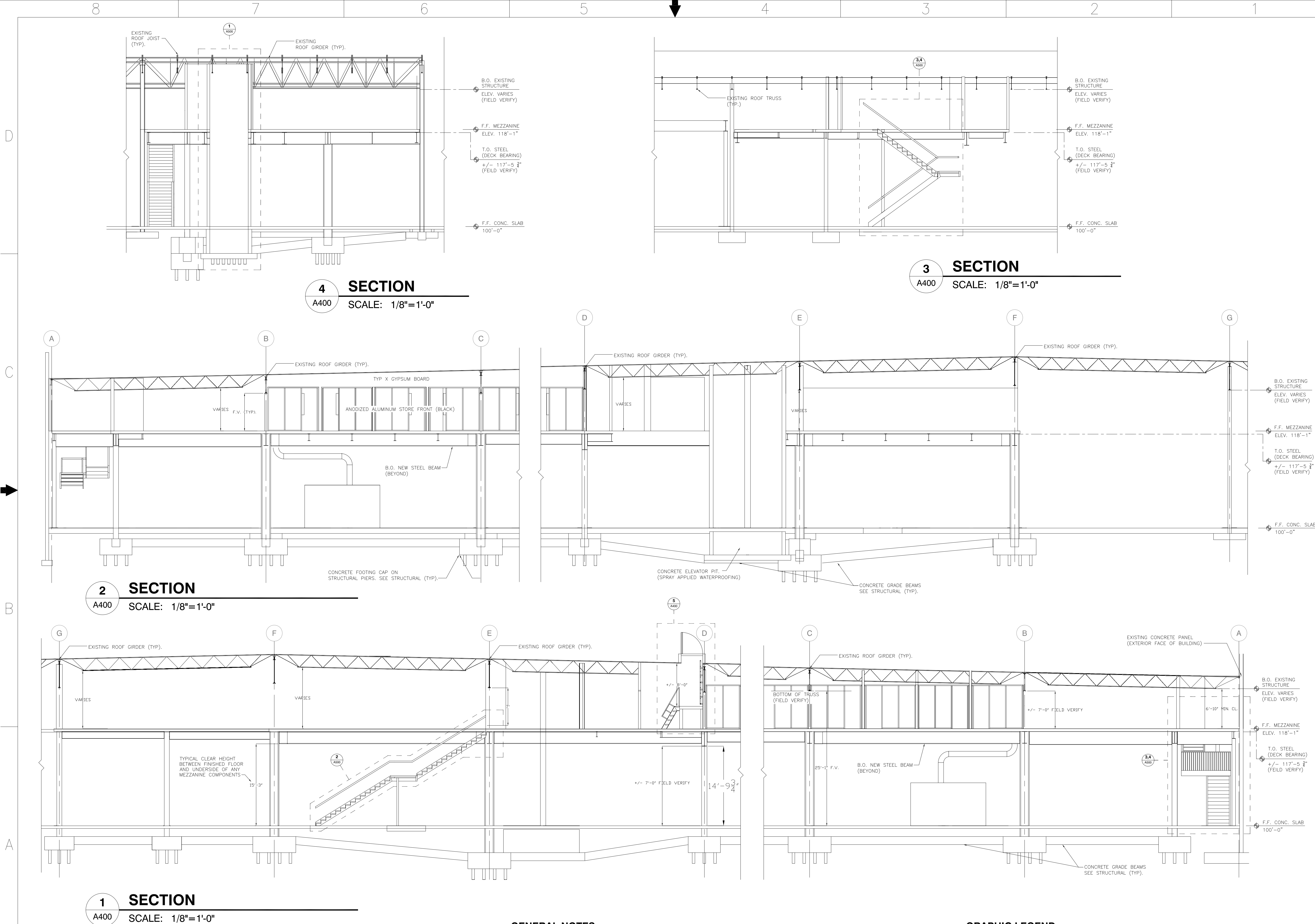
Project Name
ALBANY MEZZANINE

Sheet Title
SECTIONS

Scale 1/8"=1'-0"	Date 04.18.2023
Drawn GxA	Project No. 001-23
Sheet No.	

A400

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116



GENERAL NOTES

- DO NOT SCALE DRAWINGS.
- GC TO VERIFY ALL DIMENSIONS.

GRAPHIC LEGEND



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Issued/Revisions

No.	Description	Date
1	PERMIT SUBMISSION	04/18/23
2		
3		
4		
5		
6		

Notes
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Written dimensions shall have precedence over scaled dimensions. Contractors shall verify and be responsible for all dimensions and conditions on the job and the Architect shall be informed of any variations from the dimensions and conditions shown on the drawing. Shop drawings shall be submitted to the Architect for approval before proceeding with fabrication.

Project Name
ALBANY MECHANICAL

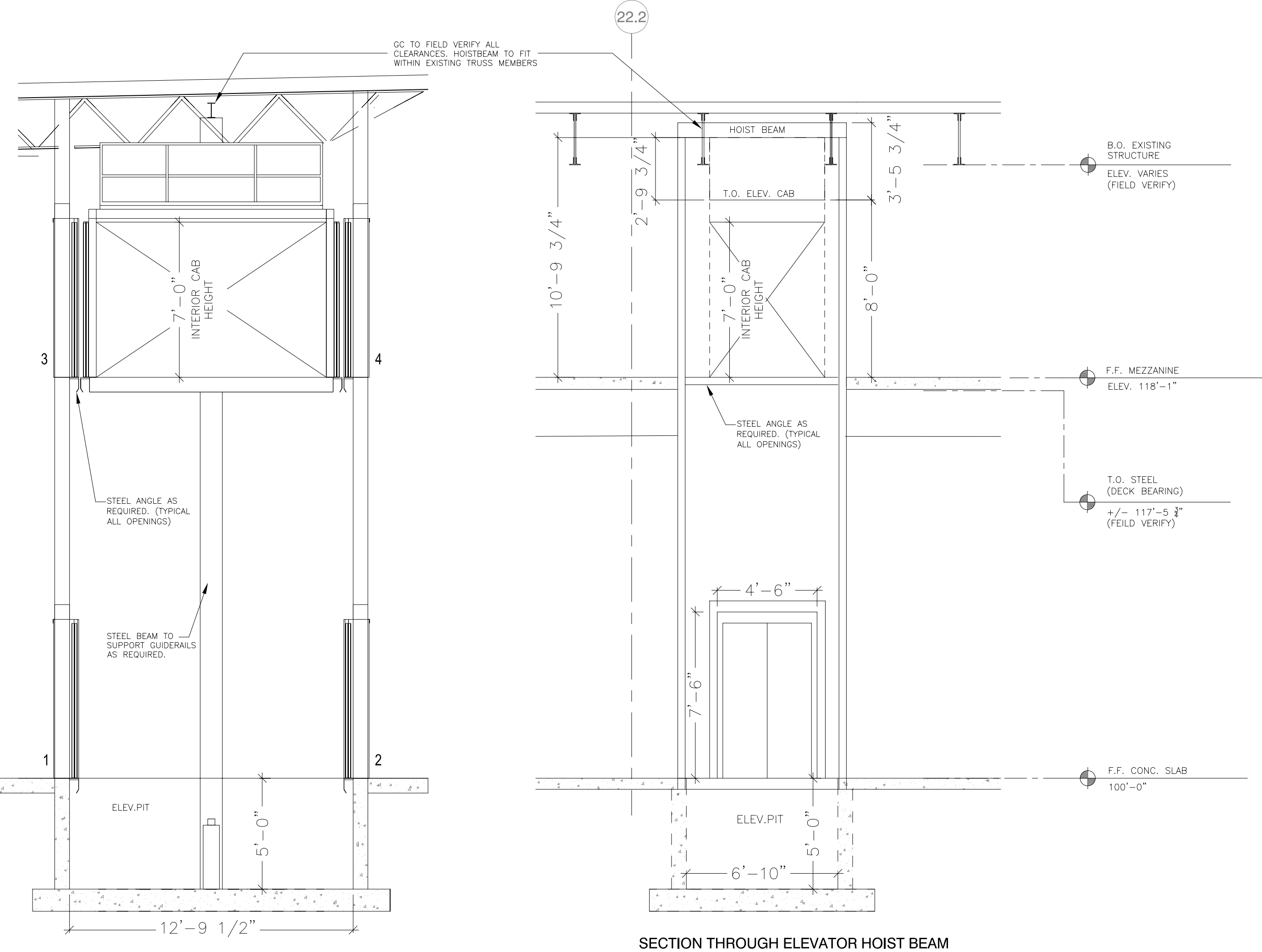
Sheet Title
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Drawn GxA	Project No. 001-23

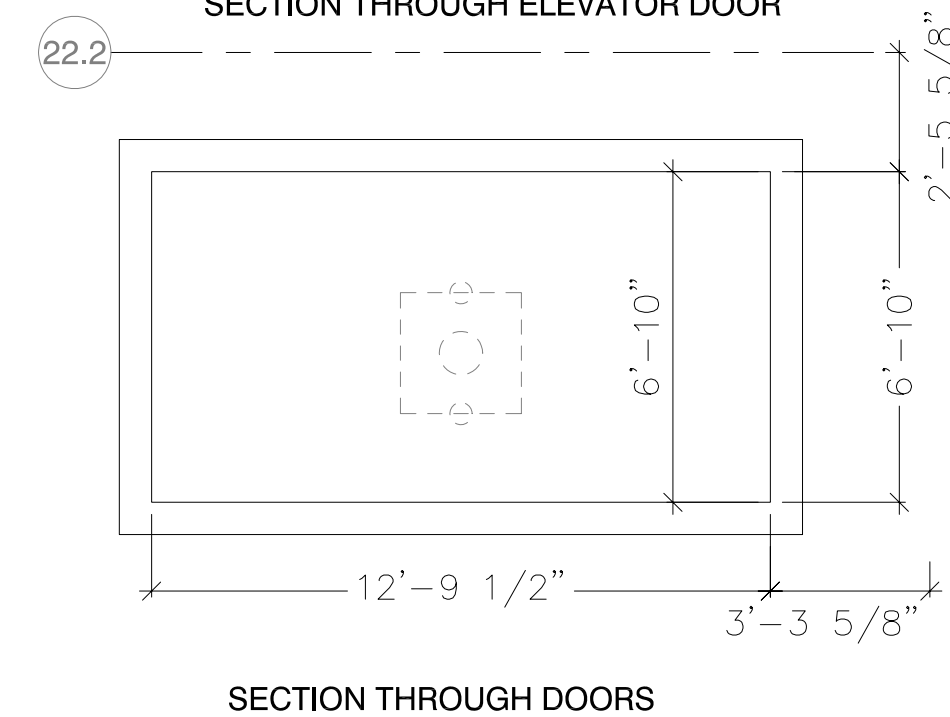
Sheet No.

A500

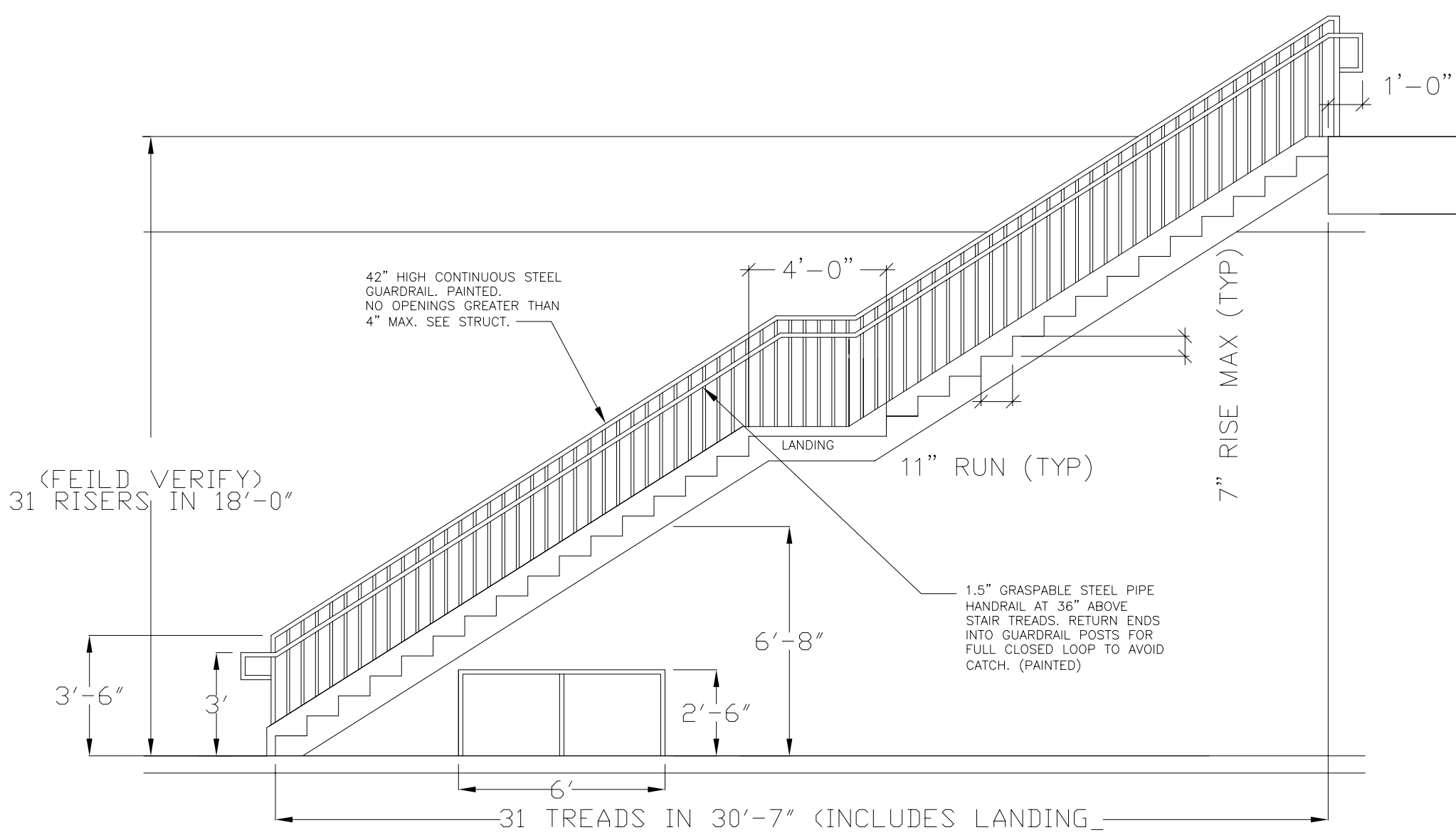
ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116



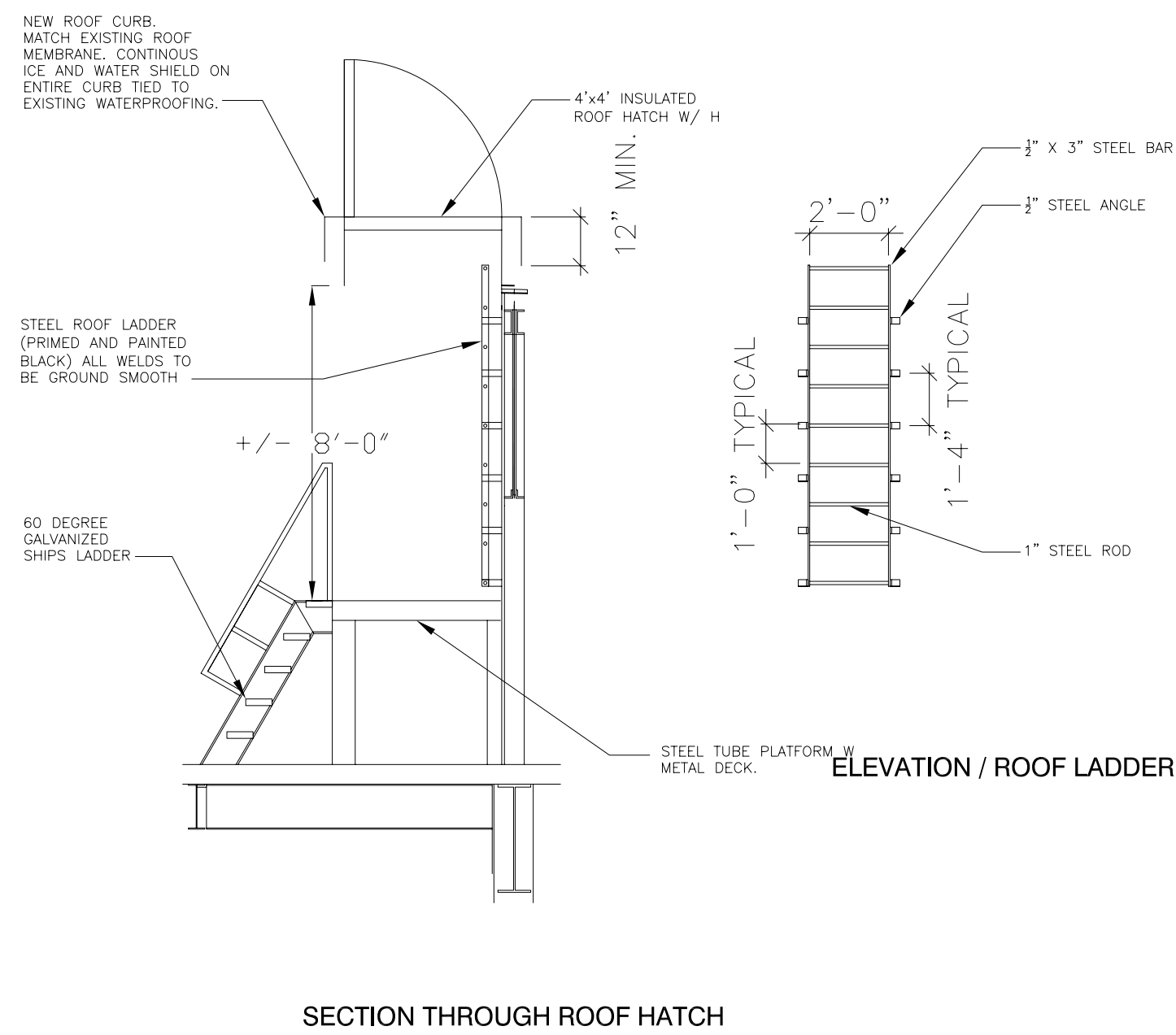
1
A500
ELEVATOR PLANS / SECTIONS
SCALE: 1/4" = 1'-0"



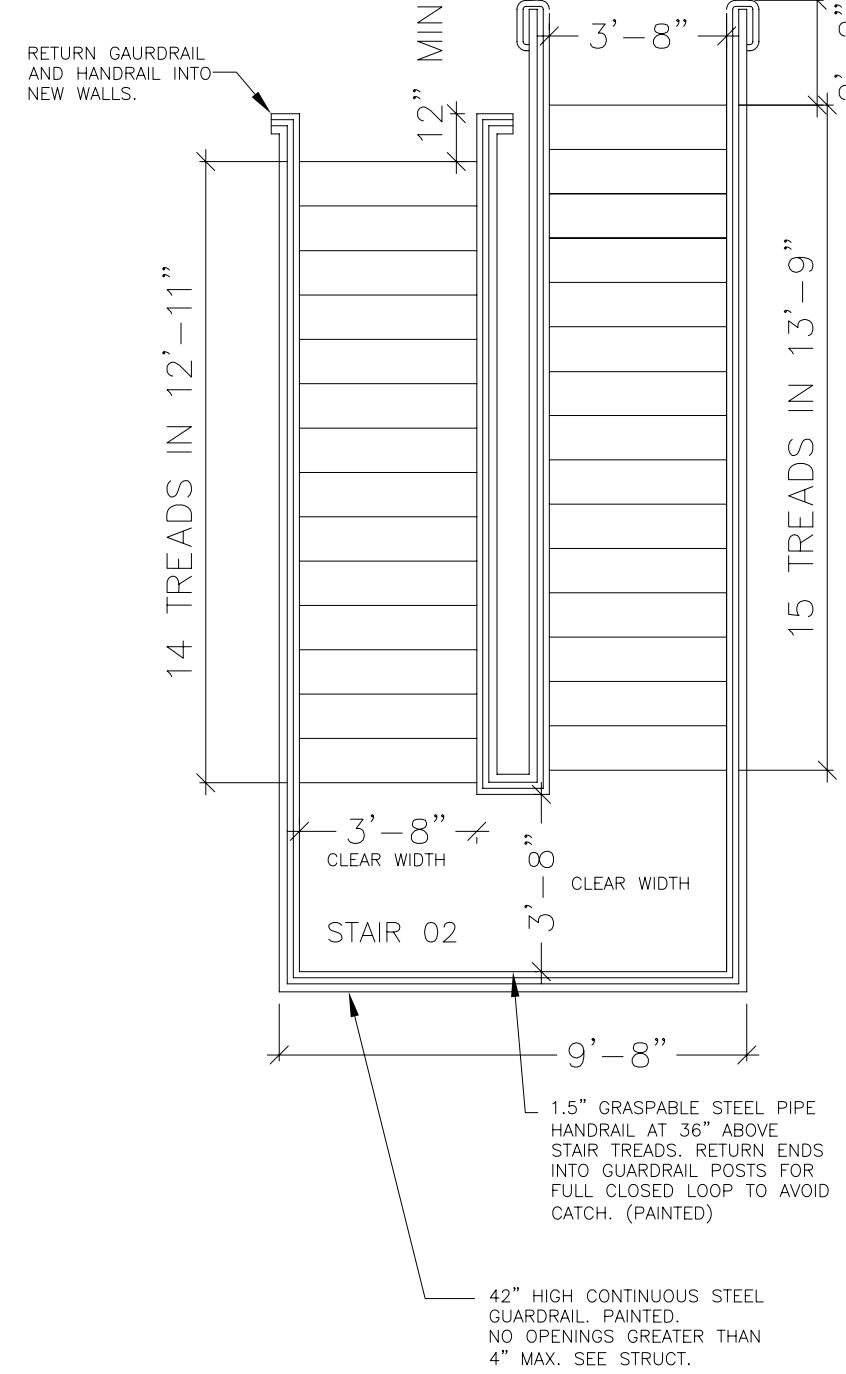
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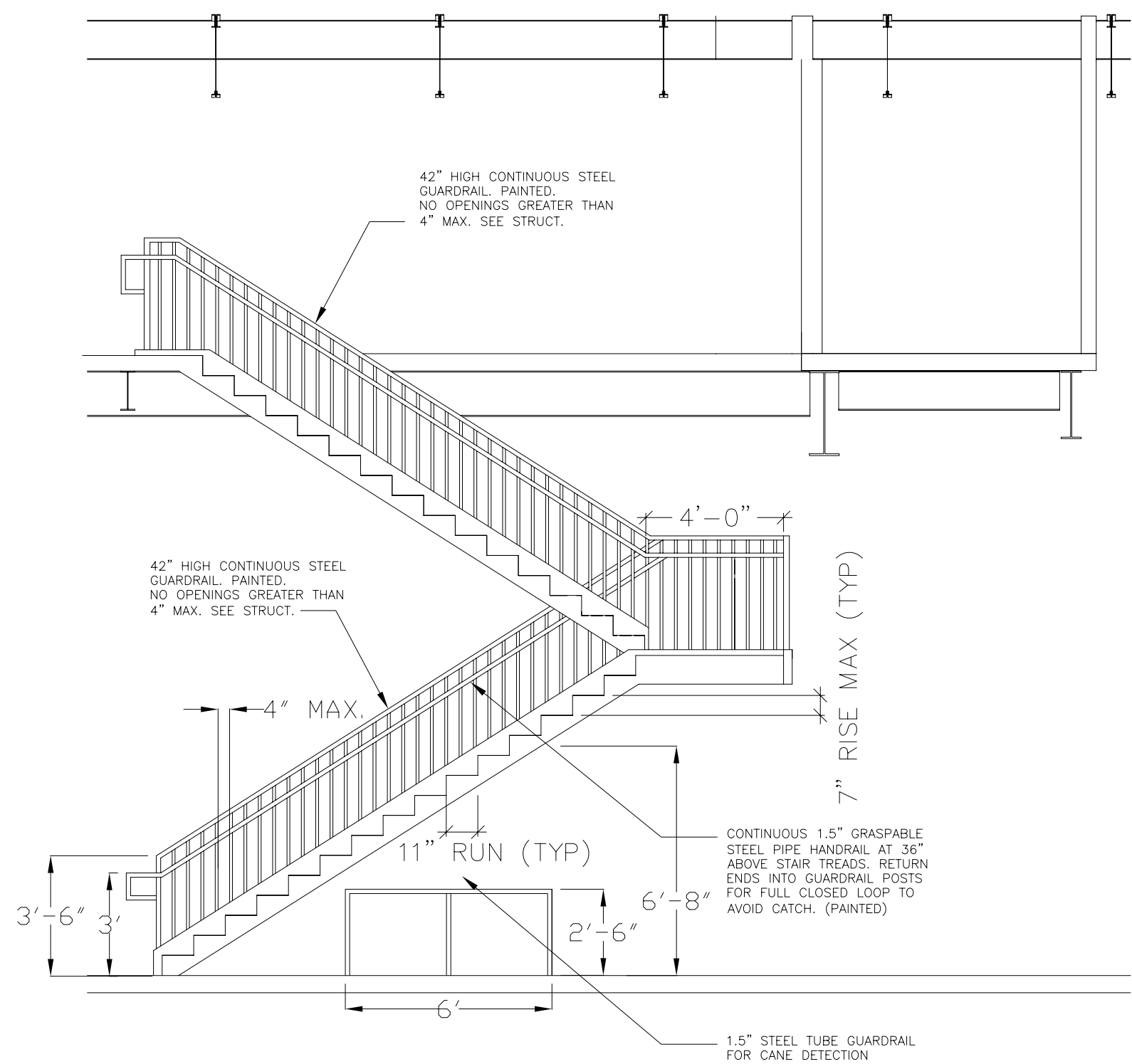
2
A500
STAIR 1 SECTION / ELEVATION
SCALE: 1/4" = 1'-0"



5
A500
ROOF HATCH / ACCESS LADDERS
SCALE: 1/4" = 1'-0"



4
A500
STAIR 2 SECTION
SCALE: 1/4" = 1'-0"



3
A500
STAIR 2 ELEVATION
SCALE: 1/4" = 1'-0"

No.	Description	Date
1	PERMIT SUBMISSION	04/18/23
2	PERMIT RE-SUBMISSION	05/18/23
3		
4		
5		
6		

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Project Name
ALBANY MECHANICAL

Sheet Title
**DOOR & WINDOW
SCHEDULES / DETAILS**

Scale
AS SHOWN

Date
04.18.2023

Drawn
GxA

Project No.
001-23

Sheet No.

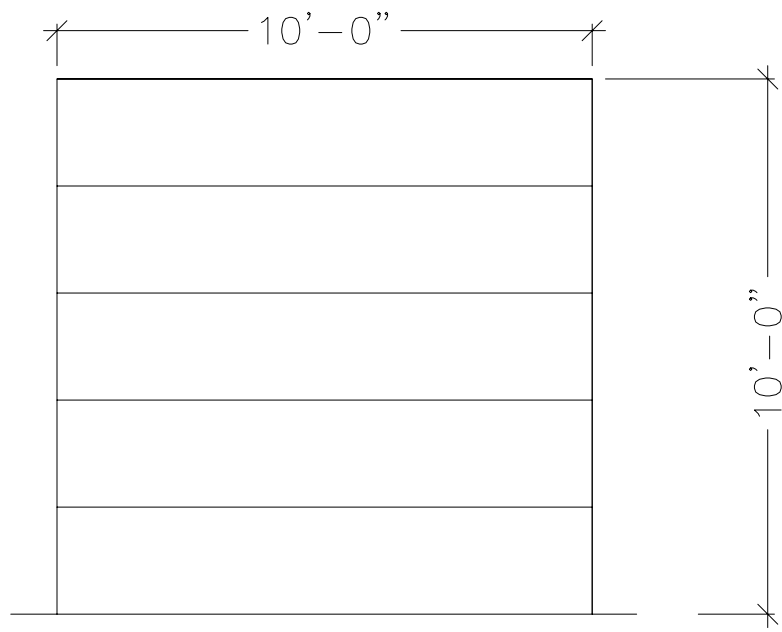
A600

DOOR SCHEDULE

DOOR #	ROOM	SIZE	DOOR			DOOR NOTES
			TYPE	MATERIAL	FINISH	
100A	ELEV. MACH.	3'-6"x7'-0" (x) 2	1	HM	PAINT	90 MIN RATING / SECURITY LOCK
100B	ELEV. MACH.	3'-6"x7'-0"	1	HM	PAINT	90 MIN RATING / SECURITY LOCK
200A	LANDING (ROLL UP)	10'X10'	5	METAL	PAINT	MOTORIZED
200B	LANDING (SWING)	3'X7'	1	HM	PAINT	PRIVACY / DEADBOLT
200C	ROOF HATCH (LIFT)	4'x4'		METAL (INSUL)	PAINT	MANUFACTURER
202A	LANDING (ROLL UP)	10'X10'	5	METAL	PAINT	MOTORIZED
202B	STORAGE (SWING)	3'X7'	1	HM	PAINT	EXIT / LATCH-LEVER w/ PANIC
205A	CONFERENCE (SLIDE)	3'x8'	4	STEEL GLASS	PAINT	1/2" TEMPERED DOUBLE GLASS DOORS ON SLIDING OVERHEAD TRACK. RUSTICA OR APPROVED EQUAL. CRL 36LPMBL HANDLES.
205B	CONFERENCE (SLIDE)	3'x8'	4	STEEL GLASS	PAINT	
206A	OFFICE (SWING)	3'X7' (PAIR)	2	WOOD	PAINT	PRIVACY / LEVER
206B	OFFICE (SWING)	3'X7' (PAIR)	2	WOOD	PAINT	PRIVACY / LEVER
210	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
211	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
212	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
213	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
214	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
215	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
216	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
217	OFFICE (SWING)	3'X8'	3	WOOD	PAINT	PRIVACY / LEVER
220	STAIR 02 (SWING)	3'X7'	1	HM	PAINT	EXIT / LATCH-LEVER w/ PANIC
222	ELECTRICAL (SWING)	3'X7'	1	HM	PAINT	PRIVACY / DEADBOLT
224	JANITOR (SWING)	3'X7'	1	HM	PAINT	PRIVACY / DEADBOLT
226	MENS RM. (SWING)	3'X7'	3A	WOOD	STAIN	SWING / KICKPLATE ONLY
226A	TOILET (SWING)	3'X7' (ACC)	3A	WOOD	STAIN	TOILET STALL PRIVACY LOCK
226B	TOILET (SWING)	2'-4"X7'	3A	WOOD	STAIN	TOILET STALL PRIVACY LOCK
228	WOMENS RM. (SWING)	3'X7'	3A	WOOD	STAIN	SWING / KICKPLATE ONLY
228A	TOILET (SWING)	3'X7' (ACC)	3A	WOOD	STAIN	TOILET STALL PRIVACY LOCK
228B	TOILET (SWING)	2'-4"X7'	3A	WOOD	STAIN	TOILET STALL PRIVACY LOCK
228C	TOILET (SWING)	2'-4"X7'	3A	WOOD	STAIN	TOILET STALL PRIVACY LOCK
228D	TOILET (SWING)	2'-4"X7'	3A	WOOD	STAIN	TOILET STALL PRIVACY LOCK

DOOR NOTES:

- COORDINATE WITH OWNER ON MANUFACTURER, STYLE, FINISH TO COMPLY W/ ALBANY STANDARD ON ALL NEW LOCKSETS.
- PROVIDE ARCHITECT WITH SAMPLES OF ALL DOOR MATERIALS AND COMPLETE SUBMITTAL FOR REVIEW.



TYPE 35
OVERHEAD DOOR (MOTORIZED)
STEEL FRAME

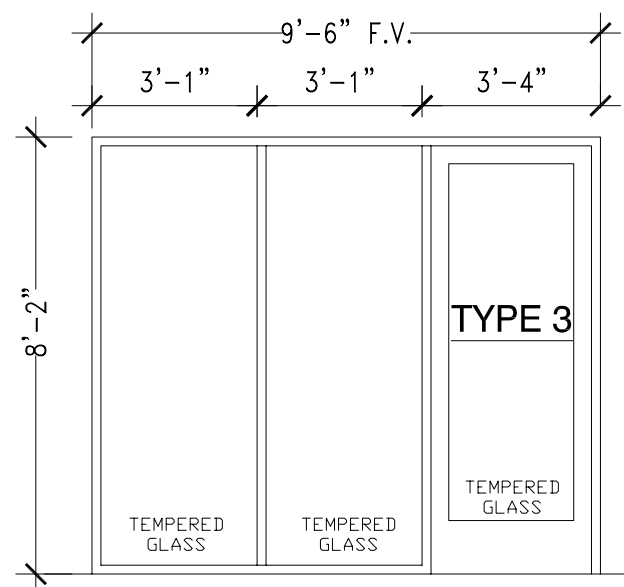
TYPE 4
1/2" TEMPERED GLASS
(CONFERENCE RM)
FRAMELESS

TYPE 3
WOOD W/ FULL LITE
(TYPICAL OFFICE DOOR)
ALUM. FRAME

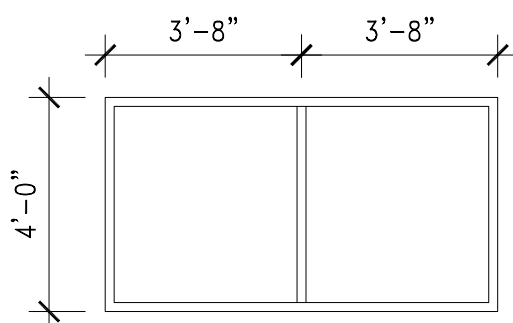
TYPE 3A
WOOD
(RESTROOM)
HM FRAME

TYPE 2
DOUBLE HOLLOW METAL
DOOR & FRAME
(PAINTED)

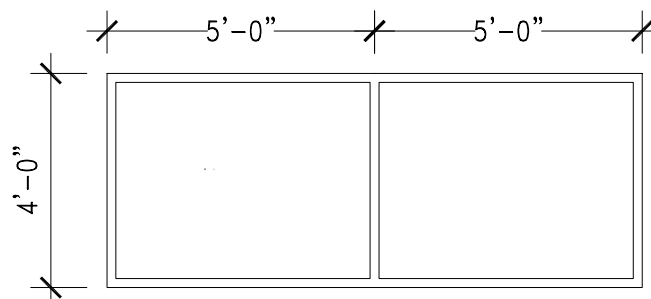
TYPE 1
HOLLOW METAL
DOOR & FRAME
(PAINTED)



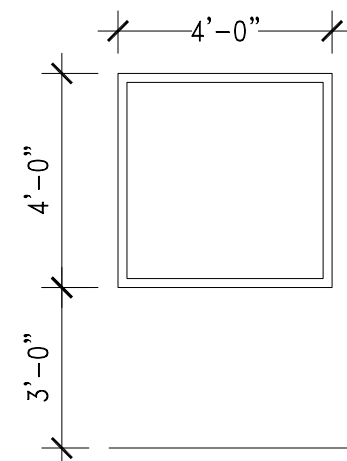
W4
PREFIN. ALUM.
(BLACK)



W3
PREFIN. ALUM.
(BLACK)



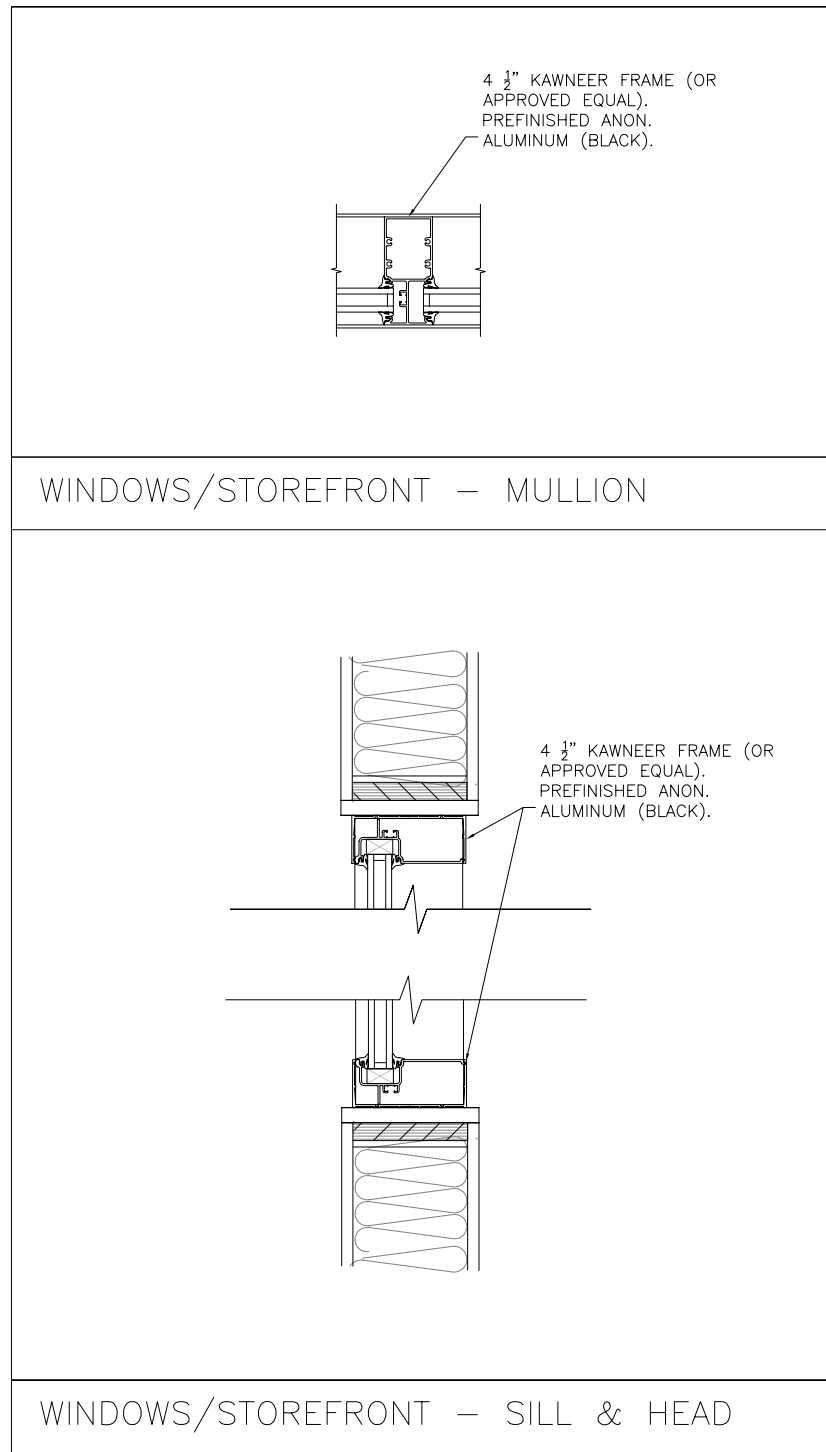
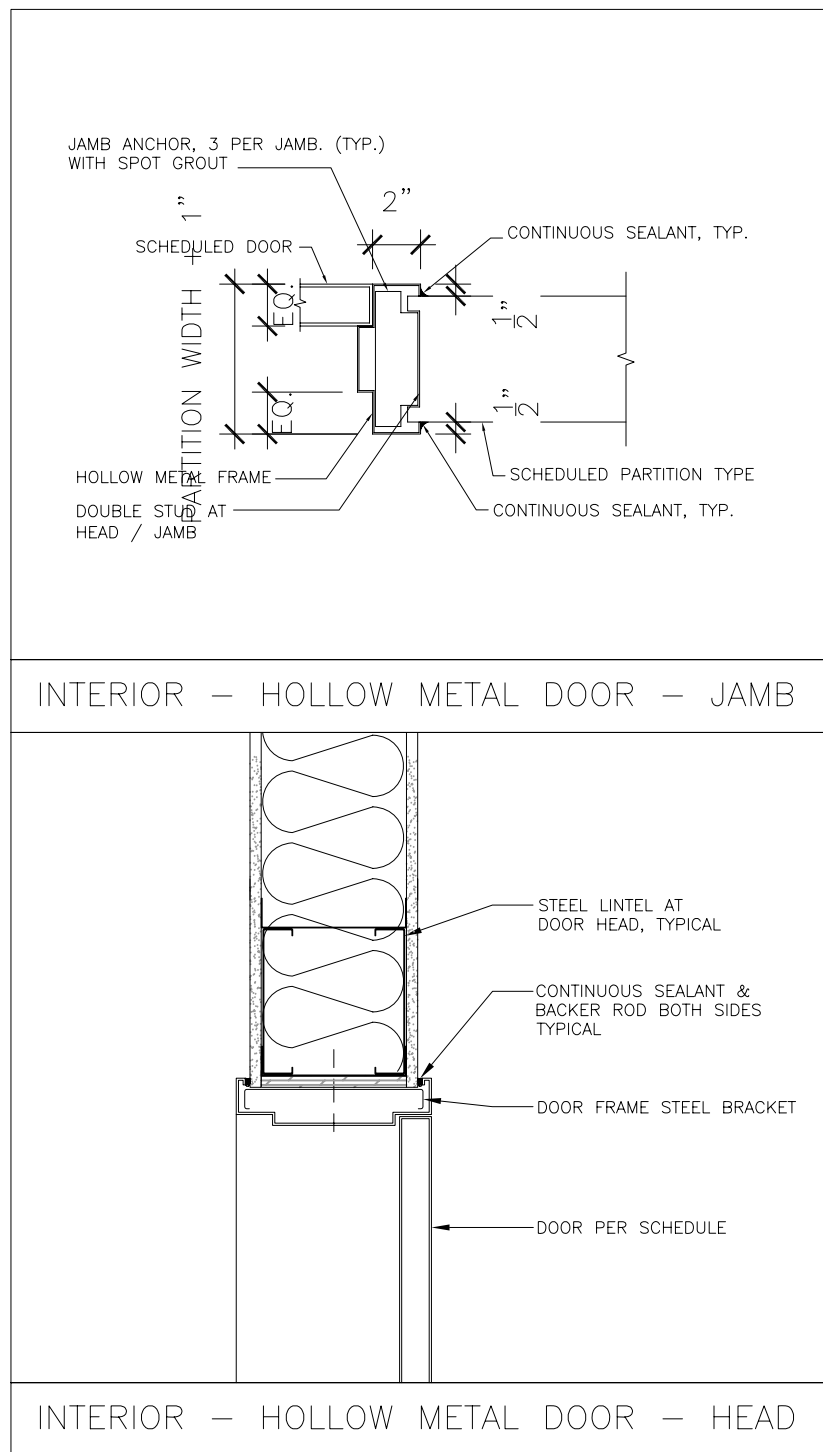
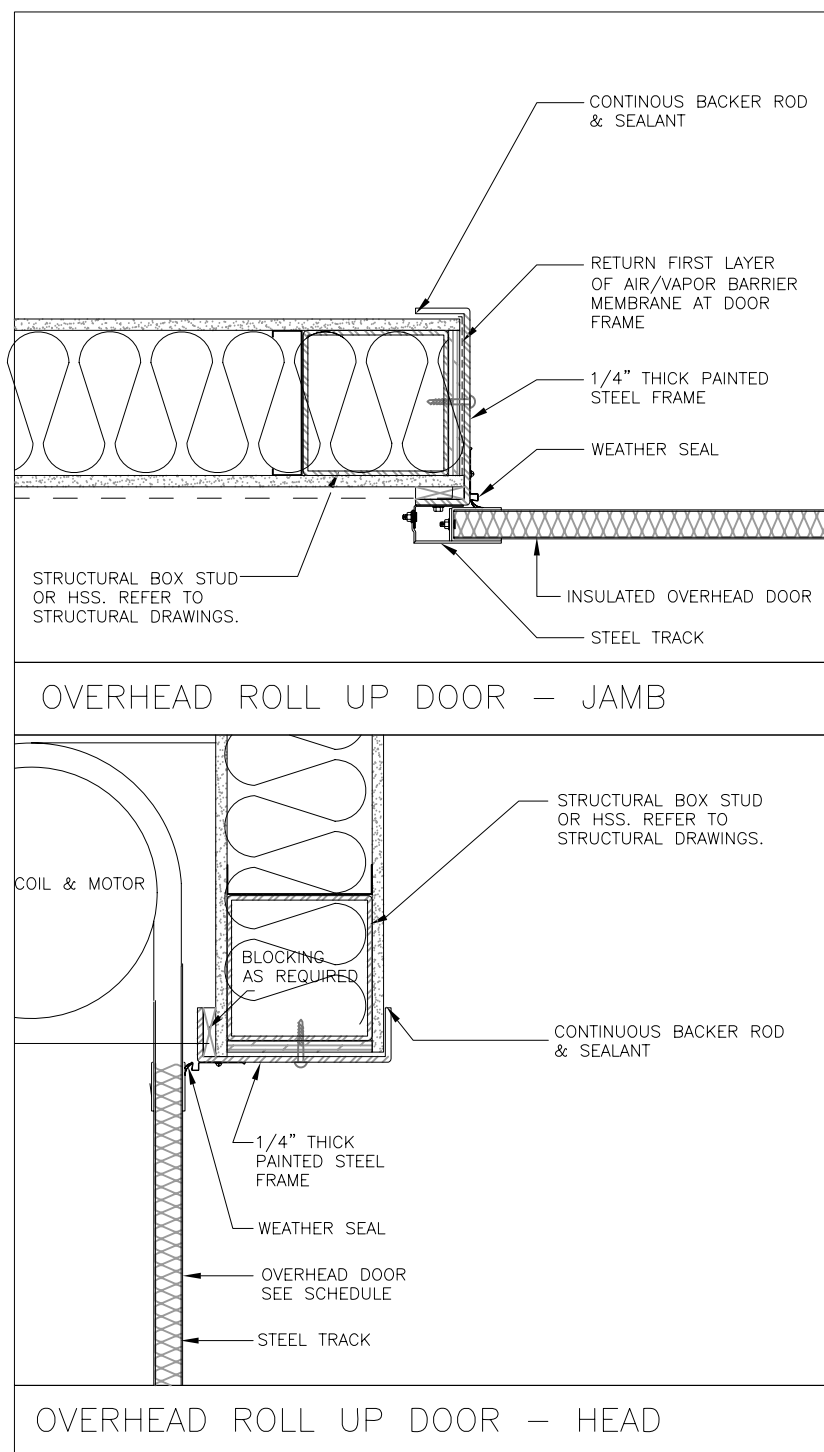
W2
PREFIN. ALUM.
(BLACK)



W1
PREFIN. ALUM.
(BLACK)

DOOR / WINDOW TYPES

SCALE: N.T.S.



DOOR / WINDOW DETAILS

SCALE: 1 1/2" = 1'-0"

DOOR & WINDOW SCHEDULE

SCALE: N.T.S.

6/7/2023 5:17:07 PM

A

B

C

D

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EXPORT WARNING

BASIS OF DESIGN

- Governing Building Code-----International Building Code 2018
- Risk Category-----II
- Floor Live Loads
 - Uniformly Distributed Loads
 - Offices + Partitions-----80 psf (reducible)
 - Heavy Storage-----250 psf (unreducible)
 - Start Loading (deferred submittal)-----100 psf (unreducible)
 - Concentrated Loads
 - Offices-----2000 lbs
- Roof Live Load (Not concurrent with Roof Snow Load)-----20 psf or 300 lbs
- Seismic Design Criteria
 - Mapped Spectral Response Accelerations
 - 0.2-Second (Short) Period Acceleration-----SS = 1.321
 - 1-Second Acceleration-----S1 = 0.469
 - Design Spectral Response Accelerations
 - 0.2-Second (Short) Period Acceleration-----SDS = 1.167
 - 1-Second Acceleration-----S01 = 0.570
 - Site Class (Soil Profile)-----D - Default
 - Seismic Importance Factor-----Ie = 1.0
 - Seismic Design Category-----D
- Analysis Procedure-----Special Steel Moment Frames
 - Response Modification Coefficient-----R = 8
 - System Overstrength Factor-----Co = 3
 - Deflection Amplification Factor-----CD = 5.5
 - Design Base Shear-----V = Ce*W = 0.132W, where W is structural weight
- Seismicity Criteria
 - Intersory Seismic/Wind Drift-----Δa = 0.02h (h is story height)
 - Deflection Limits-----Total-----Live/Snow/Wind
 - Floors-----L/240-----L/360
 - Stairs-----L/480-----L/600(1/4" max)
 - Perimeter-----L/600(3/8" max)
 - Concrete floors are limited to 2" long-term total deflection

BUILDING MAXIMUM STORY DRIFT

Level	Floor to floor height (feet)	Elastic Story Drift		Inelastic Story Drift	
		Story Drift (inch)	Drift Ratio	Story Drift (inch)	Drift Ratio
2	18' - 1"	0.434"	0.002	2.17"	0.01

- The structural notes are intended to complement the project specifications. Specific notes and details in the drawings shall govern over the structural notes and typical details.
- Typical details and sections shall apply where specific details are not shown.
- The contractor shall verify all site conditions and dimensions. If actual conditions differ from those shown in the contract drawings, the contractor shall immediately notify the architect/engineer before proceeding with the fabrication or construction of any affected elements. Discrepancies should be brought to the attention of the architect prior to fabrication or construction.
- Drawings shall not be scaled for the purpose of preparing shop drawings or for construction. Where dimensions on the design drawings are not provided or inferred, the contractor may scale drawings only to estimate member lengths for the purpose of bidding.
- Changes to these contract drawings may be made only by an authorized representative of Dunn Associates, Inc. Dunn Associates, Inc. shall not be held responsible or liable for any claims arising directly or indirectly from changes made without written authorization by an authorized representative of Dunn Associates, Inc.
- Omissions or conflicts between the contract drawings and/or specifications shall be brought to the attention of the architect/engineer before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the architect/engineer at no additional cost to the owner.
- The contractor shall submit a written request to the architect/engineer before proceeding with any changes, substitutions, or modifications. Any work done by the contractor before receiving written approval will be at the contractor's risk. These contract documents note and describe potential bid alternate details that may be requested from and approved by the Engineer of Record during the bidding and negotiation phase. The contractor may also submit to the architect/engineer for approval other substitutions or modifications to the design drawings as bid alternates during the bidding and negotiation phase. Field modifications to structural elements are not permitted without notification and approval by the Engineer of Record.
- The contractor shall coordinate with all trades any items that are to be integrated into the structural system such as openings, penetrations, mechanical and electrical equipment, etc. Structural drawings do not show such openings. Refer to other discipline drawings. Sizes and locations of mechanical and other equipment that differs from those shown on the contract drawings shall be reported to the architect/engineer. Contractor shall take measures as required to insure that construction loads shall not exceed design loads for the structure.
- Any structural items shown on other discipline's drawings that are not shown on the structural drawings, but that are noted as "refer to structural drawings" for additional information, shall be brought to the attention of the structural engineer by the contractor.
- Items such as fireproofing, waterproofing, insulation, vapor barrier, etc., may be shown or noted on structural drawings for reference only. Refer to the architectural drawings or specifications for more information.
- The contractor shall be responsible for means, methods, techniques, sequences, and procedures in order to comply with the contract drawings and specifications. The contractor shall provide adequate shoring and bracing as required for the chosen method of erection. Shoring and bracing shall remain in place until final connections for the permanent members are completed. The building shall not be considered stable until all connections are completed. Walls shall not be considered self-supporting and shall be braced until the floor/slab system is completed.
- Site observations by a field representative of Dunn Associates, Inc. shall not be construed as approval of construction, the procedures, nor special inspection.
- All work shall be done in accordance with OSHA requirements. Potential conflicts between these documents and OSHA requirements shall be brought to the attention of the structural engineer before proceeding with the work.
- Shop Drawings and submittals:
 - Shop drawings include plans, details, calculations and/or other relevant design information. Review of shop drawings and submittals by Dunn Associates, Inc. is for general compliance only and is not intended for approval. The shop drawing review shall not relieve the contractor from the responsibility of completing the project according to the contract documents.
 - Submittals for the following items shall be submitted to the Project Architect/Engineer for review prior to fabrication and/or installation:
 - Concrete Mix Design
 - Concrete Reinforcing
 - Anchorages and Embeds
 - Structural Steel
 - Deferred Design Items
 - Quality control submittals shall be submitted to special inspector for review prior to fabrication/installation. Courtesy copies shall be provided to the project architect and engineer for their records.
 - Detailed and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings. The structural drawings shall be used in conjunction with the architectural and other consultants' drawings. See the Architectural Drawings for dimensions, doors, windows, non-bearing interior and exterior walls, elevations, slopes, stairs, curbs, drains, recesses, depressions, railings, waterproofing, finishes, chamfers, kerfs, etc.
 - Shop drawings made from reproductions of the structural drawings will be rejected unless the contractor signs a release agreement prior to the shop drawings being reviewed. The contractor may also obtain electronic files of the plan sheets after signing a release agreement. Electronic files of the detail sheets and schedule sheets will not be made available.
 - The Contractor may choose to submit shop drawings and submittals for review electronically. The Contractor may do this provided a minimum of one hard copy set is submitted for review. The submittal will be stamped as received by Dunn Associates, Inc. when the hard copy is received by our office. Hard copies of small submittals need not be submitted if the Contractor receives the approval for this exception by the Engineer of Record.

FOUNDATION

- Soils Investigation Report-----None
- Soil bearing pressure-----1500 psf - Assumed for design
- Frost Protection-----30" minimum
- Clear excavations of debris and loose soil prior to placing footings. All footings shall bear on undisturbed natural sub-grade or engineered compacted fill as noted in these drawings.

EARTHWORK

- Consult the project specifications for earthwork requirements. In absence of information, refer to the following notes.
- Clearing: Remove all existing structures and associated foundations, slabs, fencing, asphalt, concrete, and incidental structures as necessary for project completion. The building area shall be stripped of all vegetation, topsoil, and debris. Following stripping, all undocumented fill soils and any remaining loose natural soils shall be excavated to expose competent natural soils.
- Contractor shall provide temporary shoring for excavations as required.
- Contractor shall provide measures necessary to prevent damage to or settlement of new or existing construction and utilities on or adjacent to project site.
- Contractor shall provide dewatering as required to protect the site from flooding.
- Proof roll the entire building pad area with normal compaction equipment to check for the presence of unsuitable fills, soft spots, or other undesirable materials or conditions. Remove sub-grade materials that are unsuitable and replace with compacted structural fill or 2,000 psi lean concrete.
- Compacted structural fill: All fill material shall be a well-graded granular material with a maximum size less than 3" and with not more than 15% passing a #200 sieve. Fill beneath slabs shall be compacted to at least 95% of the maximum laboratory density as determined by ASTM D 1557. All fill shall be tested. Compacted structural fill shall be placed in lifts not exceeding 8" in uncompacted thickness.
- Floor slabs shall be tested by a granular layer at least 4" thick. The granular layer shall have a maximum size less than 1" with not more than 5% passing a #200 sieve and shall be compacted to at least 90% of the maximum laboratory density as determined by ASTM D 1557.
- The special inspector shall review all excavations and fill placement prior to placing concrete.

STRUCTURAL STEEL

- Codes and Standards: Fabrication, Erection and Quality Control of structural steel shall comply with the latest edition of the following:
 - American Institute of Steel Construction (AISC) 360, "Specification for Structural Steel Buildings," with "Commentary".
 - AISC 341 "Seismic Provisions for Structural Steel Buildings."
 - AISC 303 "Code of Standard Practice" excluding sections 3.4, 4.4 and 4.4.1.
 - AISC "Specification for Structural Joints Using High Strength Bolts"
 - American Welding Society (AWS), Structural Welding Codes D1.1, D1.3, D1.4, and D1.8, except as modified by the "Steel Construction Manual".
- Material:
 - Wide Flange Sections-----ASTM A992 (50 ksi)
 - Angles-----ASTM A36
 - Plates-----ASTM A572 Grade 50
 - Pipes-----ASTM A53 Grade B Type EIS
 - Hollow Structural Shapes
 - Rectangular-----ASTM A500 Grade C (50 ksi)
 - Round-----ASTM A500 Grade C (45 ksi)
 - Other Structural Shapes (M, C, etc), Threaded Rod-----ASTM A36
 - Weld Connections-----ASTM F3125 Grade A325 with ASTM A563 heavy hex nuts and ASTM F436 washers.
 - Anchor Bolts
 - All Columns unless noted otherwise: ASTM F1554 Grade 105 with ASTM A563 heavy hex nuts. Nuts to be snug tight.
 - Braced Frame/Moment Frame Columns unless noted otherwise: ASTM F1554 Grade 105 (equiv to A193 Grade B7) with ASTM A563 heavy hex nuts with 5/16" min plate washers. Bottom assembly to include double heavy hex nuts with similar washers. Nuts to be snug tight.
 - Weld Fillet Metal
 - Shielded Metal Arc Welding-----AWS A5.1, low-hydrogen only
 - Low-hydrogen restrictions do not apply when welding steel sections in accordance with AWS D1.3, including attaching these steels to structural members.
 - Gas-Metal & Metal-Cored Arc Welding-----AWS A5.18
 - Flux-Cored Arc Welding-----AWS A5.20
 - E7X1.4 or E7XT-1 electrodes are not permitted.
 - Intermixing of welds made from self-shielded welding electrodes with welds made by other processes is not allowed in seismic critical welds, unless tested in accordance with AWS D1.8, annex B. The Field Erection Contractor is responsible for verifying that intermixing of self-shielded weld metal with weld metal of other processes will not occur, or alternatively, the welding procedure is qualified by testing.
 - Where demand critical welds are required, provide filler metals meeting the following minimum mechanical properties: 58ksi yield strength, 70ksi tensile strength, 12% elongation, Charvy V-notch toughness of 20ft-lbs at 0°F and 40 ft-lbs at 70°F.
 - Deformed Bar Anchors (DBAs)-----ASTM A496
 - Headed Stud Anchors (HSA)-----ASTM A108
 - Non-Shrink Grout-----ASTM C1107 Grade B
 - Non-shrink grout shall be prepackaged, non-metallic, and non-gaseous. Furnish certified independent test data to Structural Engineer. Compressive Strength in 28 days = 7,500 psi.
 - Refer to architectural drawings for structural steel fireproofing or architecturally exposed steel requirements.
 - All steel, connectors and embeds exposed to weather shall be galvanized, unless noted otherwise.
- Structural Detailing
 - Welds may be performed in the shop or the field. Designations of field welds on the Contract Documents are shown where it is anticipated field welds may be required, and are shown only for the purpose of assisting the Contractor in the bidding process. The Contractor shall coordinate the welding sequence between sub-contractors, and any costs associated with variations in the welding sequence are outside the scope of the Design Engineer, and are the responsibility of the Contractor. Field welding is to be minimized where possible. Contractor is to verify that the sequencing of welds meets all safety regulations, and the requirements of the Construction Documents and their referenced codes. Welding in the V region of wide-flange members is prohibited unless noted otherwise.
 - Provide full depth web stiffener plates at one side of all beams at all bearing points, unless noted otherwise. Stiffener plates shall be the thickness called out below unless noted otherwise. Stiffeners shall be welded on both sides of the plate-to-flange and plate-to-web interfaces.
FLANGE WIDTH-----STIFFENER THICKNESS & WELD SIZE
Less than 8 1/4"-----1/4" & 3/16"
8 1/4" to 12 1/4"-----3/8" & 1/4"
12 1/4" to 16 1/2"-----1/2" & 5/16"
16 1/2" to 20 3/4"-----5/8" & 3/8"
 - Bolting and Fasteners
 - Ordinary steel-to-steel connections, simple span framing, and beam/girder-to-bearing plates are the standard connection used throughout the design drawings, unless noted otherwise.
 - Prestressed connections are shown on the structural design drawings. They join steel-to-steel connections, unless noted otherwise:
 - Use A325N bolts or tension-controlled bolts.
 - Tighten these fasteners to a "snug tight" condition.
 - Where a steel-to-steel connection is not shown, provide a framed connection per AISC for one half the total uniform load capacity of the beam for the span and steel specified.
 - Prestressed connections are shown on the structural design drawings. They join steel-to-steel connections, unless noted otherwise:
 - Use A325N or A325X bolts or tension-controlled bolts.
 - Pretension these fasteners as required by AISC "Specification for Structural Joints Using ASTM A325 or a A490 bolts."
 - Slip Critical connections (SC) are shown on the structural design drawings. They join steel-to-steel connections in Seismic Load-Resisting Systems (SLRS).
 - Fasteners and washers shall not be reused. Scrap dirty, rusted, or water-contaminated bolt assemblies.
 - Reduced Beam Sections
 - Fabrication of the reduced flange sections of beams used in SLRS is restricted to mechanically guided thermal cutting processes. Freehand cutting is not permitted.
 - Flange cuts shall meet the requirements of AISC 308.
 - Repair of gouges, notches, mill imperfections, shall conform to the requirements of the AISC and AWS provisions.
 - Web Access Holes and Temporary Attachments
 - Fabricate beam copes and web access holes using the geometry described in AISC 360 Section J1.8.
 - Rundoff flats are to be removed unless noted otherwise.
 - Backup Bars: Remove backup bars from connections in demand critical welds, unless noted otherwise. Background the root and weld to sound metal. Reweld the gouged area and add a 5/16" reinforcing fillet weld. Backup bars may remain for top flange beam welds provided the backup bar is welded to column flange with 5/16" fillet weld.
 - Protected Zones: No connections, other than those on the design drawings, shall be made within the protected zone of the SLRS as identified in AISC 341.
 - Locate headed studs, welds, miscellaneous metal, etc. outside of the protected zone.
 - Paint the protected zones with bright paint before and after the coating operations to identify them.
 - All welds not noted on drawings shall be minimum 1/4" fillet welds.
 - All structural steel members shall be considered as an unrestrained fire-resistance-rated assembly.
 - Welding of Reinforcing Steel or Bolts
 - Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for deformed bar anchors, structural bolts, or headed stud anchors.
 - Do not weld anchor bolts, including "back" welds.
 - Headed Stud Anchor welding and Deformed Bar Anchor welding shall conform to the manufacturer's specifications.

CONCRETE

Concrete shall be supplied in accordance with ACI 318 and the following requirements:

Concrete Use	Comp. Strength f'c (psi)	Exposure Classes per ACI 318 19.3.1 (a,b,c)	Nominal Max Aggregate Size
Footings / Pile Caps	4000	F0, S0, W0, C1	1 1/2"
Other Walls	4000	F0, S0, W0, C0	3/4"
Interior Slabs on Grade (d,e,f)	4000	F0, S0, W0, C0	3/4"
Light WL Concrete on Steel Deck (g)	3000	F0, S0, W0, C0	3/4"

Table Footnotes:

- Cement type ASTM C150 or C595, max. water/cement ratio and fly ash to comply with ACI 318 Table 19.3.2.1.
 - Air content ± 1.5%, to comply with ACI 318 Tables 19.3.2.1 and 19.3.3.1, initially measured at point of final placement and point of discharge. Subsequent measurements may occur only at point of discharge provided air content is adjusted to account for placement losses. Air content shall be adjusted for the use of admixtures, fly ash and aggregate size. Air-entraining admixtures shall comply with ASTM C260 (when used).
 - Calcium chloride shall not be added to the concrete mix. Unreinforced concrete slabs on grade may use calcium chloride as permitted by ACI 318 Table 19.3.2.1.
 - For any exposed slab on grade, the contractor is to notify the engineer of record at least 7 days prior to any pours to discuss the concrete mix design being used as well as preventing means and methods of addressing concrete phenomena such as cracking, curing, spalling, etc.
 - Interior slabs on grade shall have a drying shrinkage maximum of 0.004% by ASTM C157 (7-day soak time permitted). Test results shall be submitted with mix designs.
 - For slab on grade 6" or thicker, a maximum aggregate size of 1 1/2" is permitted.
 - Contractor shall provide verification that mix design for lightweight concrete over metal deck has a maximum dry weight density of 110 ± pcf per ASTM C567.
- Materials unless noted otherwise:
 - Normal Weight aggregates-----ASTM C33
 - Light Weight aggregates-----ASTM C330
 - Fly Ash, Class C or F Pozzolan-----ASTM C618
 - Reinforcing Steel
 - General-----ASTM A615 Grade 60
 - Subject to the above requirements, ASTM A615 Grade 75 steel may be used as the contractor's option, except in special moment frames, special concrete shear walls, shear stirrups or torsional reinforcement.
 - Deformed Bar Anchors (DBA)-----ASTM A496
 - Headed Stud Anchors (HSA)-----ASTM A108
 - Anchor Bolts: See steel and/or wood section(s) of general notes.
 - No aluminum conduit or product containing aluminum or other material injurious to concrete shall be embedded in concrete.
 - Reinforce composite slabs over metal deck with the following welded wire reinforcement (minimum), based on the thickness of concrete above upper deck flutes. Welded wire reinforcement shall be placed 1" to 1 1/2" below the top of the slab:
 - Slab (above upper deck flutes) 3 1/4" or thinner-----6" x 6" - W1.4W1.4
 - Slab (above upper deck flutes) 4" or thinner, but thicker than 3 1/4"-----6" x 6" - W2.1W2.1
 - Slab (above upper deck flutes) thicker than 4"-----6" x 6" - W2.9W2.9
 - Welded wire reinforcement may be substituted with macro synthetic fibers "coarse fibers" (per ASTM C-1116), made from virgin polyolefin, with equivalent diameter between 0.016" and 0.05", having minimum aspect ratio (length/equivalent diameter) of 50, at a minimum grade of 4 lb / cubic yard. Fibers above finished slab shall not be buried off. Do not use fibers in architecturally finished or colored concrete.
 - The contractor shall be responsible for the design, detailing, care, placement and curing of all formwork and shores.
 - Supporting forms and shoring shall not be removed until structural members have acquired sufficient strength to safely support their own weight and any construction load to which they may be subjected. In no case, however, shall forms and shoring be removed in less than 24 hours after concrete placement.
 - Suspended slabs shall be re-supported after form removal until concrete reaches its 28-day specified compressive strength.

- Reinforcement shall have the following concrete clear cover:
 - Cast-in-place Concrete
 - Cast against and permanently exposed to earth-----3"
 - Formed concrete exposed to earth or weather:-----2"
#6 thru #18 bars-----1 1/2"
#5 and smaller bars-----1"
 - Concrete not exposed to weather or in contact with ground:-----3/4"
Slabs, Walls, Joists: #1 bars and smaller-----3/4"
Beams, Columns: Primary Reinforcement, Ties-----1 1/2"
Stirrups, Spirals-----1 1/2"
 - Construction Joints and Control Joints:
 - Provide a beveled 2" x 4" x continuous or intermittent keyway in all horizontal and vertical construction joints including between top of footing and foundation walls. In addition, all joints shall be intentionally roughened to a full amplitude of approximately 1/4".
 - Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. Control joints shall be completed within 12 hours of concrete placement. Control joints may be installed by either:
 - Saw cut with depth of 1/4 the thickness of the slab
 - Tooled joints with depth of 1/4 the thickness of the slab
 - Install control joints in slabs on grade at a spacing not to exceed 30 times the slab thickness in any direction, unless noted otherwise. Construction joints in slabs on grade shall not exceed a distance of 125'-0" on center in any direction.
 - Install construction joints in walls at a spacing not to exceed 30 times the wall thickness, except in concrete shear walls. For masonry walls above, align joints in concrete walls with masonry control joints.
 - Construction joints are not permitted in suspended slabs or beams unless specifically noted on the construction documents or submitted by the Contractor to the Engineer of Record for review.
 - Construction
 - Use chains or other support devices recommended by the CRSI to support bar and tie reinforcement bars and WWR prior to placing concrete. WWR shall be continuously supported at 36" or center maximum. Reinforcing steel for slabs on grade shall be adequately supported on precast concrete units. Lifting the reinforcing off the grade during placement of concrete is not permitted.
 - Contractor shall coordinate placement of all openings, curbs, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
 - All embeds and dowels shall be securely tied to formwork or to adjacent rebar prior to concrete placement.
 - No pipes, ducts, sleeves, etc. shall be placed in structural concrete unless specifically detailed or approved by the structural engineer. Penetrations through walls when approved shall be built into the wall prior to concrete placement. Penetrations will not be allowed in footings or grade beams unless detailed. Piping shall be routed around these elements and footings stopped to avoid piping.
 - Reinforcing bars shall not be welded unless specifically shown on drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for DBAs or HSAs.
 - Reinforcing bars shall not be field bent, except as shown in the contract drawings or permitted by the EOR.
 - Top of concrete columns shall be flush (±1/4") with bottom of supported cast-in-place members.
 - Detailing:
 - Lap splice lengths shall be detailed to comply with the "Reinforcing Bar Lap Splice Schedule" contained within the contract drawings.
 - Do not splice stirrups and ties. Do not splice lap splices in reinforcing walls unless specifically shown.
 - At shear wall boundary elements lap vertical bars shall be increased by 25%.
 - Splices may be made with mechanical splices capable of 125% of yield strength of the bar being spliced (Type 1). Splices located within lateral resisting elements shall also develop the tensile strength of the bar (Type 2). Mechanical splices shall be the positive connecting type coupler and shall meet all AISC requirements. Use "Cadevel", "Lertion" Standard Couplers, "Bar-Lock" or equal with internal protector. If mechanical splices are used, splices or couplers on adjacent bars shall be staggered a minimum of 24" apart along the longitudinal axis of the reinforcing bars.
 - All 90, 135 and 180° hooks shown graphically in the drawings shall be detailed as ACI standard hooks, unless noted otherwise.
 - At joints provide reinforcing dowels to match the member reinforcing, unless noted otherwise.
 - At all discontinuous control or construction slab on grade joints, provide (2) #4 x 48".
 - Provide corner bars at intersecting wall corners using the same bar size and spacing as the horizontal wall reinforcing.
 - All vertical reinforcing shall be doweled to footings, or to the structure below with the same size and spacing as the vertical reinforcing for the element above. Dowels extending into footings shall terminate with a 90° standard hook and shall extend to within 4" of the bottom of the footing. Footing dowels (#6 bars and smaller) with hooks need not extend more than 20" into footings.
 - See details for reinforcing around miscellaneous openings. All recesses that interrupt reinforcing shall be reinforced the same as an opening. H. Reference ACI 315 for additional detail requirements for review by the engineer prior to any placement of concrete.
 - Contractor required to submit concrete mix design for review by the engineer prior to any placement of concrete.
 - All concrete shall be mixed and placed per ACI 304. Contractor shall refer to and follow the recommendations in ACI 305R for hot weather concreting and ACI 308R for cold weather concreting.
 - Construction activity or storage of materials shall not take place on newly placed concrete until the concrete achieves sufficient strength to provide adequate support.

COMPOSITE STEEL BEAMS

- All beams supporting concrete over metal deck shall have headed stud anchors.
- Composite beams are indicated on the framing plans with a suffix (n). The number inside parentheses indicates the number of studs for this beam or section of beam. Beams or sections of beams shall have the studs spaced uniformly over the beam or section. The maximum spacing shall not exceed 36" on center.
- All headed stud anchors shall conform to ASTM A-108. Dimensions shall comply with AISC. Use 3/4" diameter studs. Headed studs shall extend 1 1/2" minimum (2" maximum) above the top of the steel deck after welding. Headed studs shall be applied through the metal deck to the top flange of the steel section or welded directly to the steel section.
- The minimum on center spacing of stud connectors shall be 6 diameters along the longitudinal axis of the supporting composite beam and 4 diameters transverse to the longitudinal axis of the supporting composite beam.
- Composite beams shall be precompressed as shown on plans. On the plans, c=0.00' denotes precamber dimension (upward) in inches.
- Camber tolerance shall be ±1/4" - 0".
- Slab shall be screeded to a constant thickness as indicated.

POST-INSTALLED ANCHORS

- Post-installed anchors shall only be used where specifically detailed or called for on the design drawings. If circumstances arise during construction where the Contractor desires to substitute a post-installed anchor in place of a cast-in-place anchor, the Contractor shall submit a formal written request for each circumstance to the Architect and Engineer for review.
- Follow all ICC Evaluation Report and manufacturers' requirements and recommendations for post-installed anchor installation. Where conflicts may exist, the most stringent requirement applies.
- Post-installed anchors that are exposed to exterior conditions, or interior spaces where moisture can accumulate, shall be either galvanized or stainless steel anchors.
- All holes in hollow, brick, or stone masonry shall be drilled in the "rotary-only" mode with the hammer function off.
- For installation of adhesive anchors horizontally or vertically inclined, installers must have AMI(CRSI) Adhesive Anchor Installer Certification.
- Adhesive anchors shall be as specified in the Contract Documents. If no specific adhesive is specified, or if a particular product is preferred, the Contractor may submit a request for an adhesive from the following list prior to design of the anchor. Follow manufacturer and ICC evaluation report requirements for installation temperature of adhesive anchors. Adhesive anchors shall not be installed or cured outside of approved temperature ranges. Adhesive anchors may not be installed in concrete less than 21 days old without prior approval.
 - Epigle adhesive anchors in concrete (normal weight only)
 - HIT-RE 500V3 by Hilti (ESR-3814)
 - SET-3G by Simpson (ESR-4057)
 - SET-XP by Simpson (ESR-2508)
 - AT-XP by Simpson (AFMCO ESR-40263)
 - Pure 110n by Dewart (ESR-3288)
 - AC208+ Gold by Dewart (ESR-4027)
 - HIT-HY 200 V3 by Hilti (ESR-4688)
 - Mechanical anchors shall be as specified in the Contract Documents. If no specific mechanical anchor is specified, or if a particular product is preferred, the Contractor may submit a request for an anchor from the following list prior to design of the anchor.
 - Epigle mechanical anchors in concrete
 - Kwik Bolt T22 by Hilti (ESR-4268)
 - Kwik HUS-E2 by Hilti (ESR-3027)
 - HDI-PT T2 by Hilti (ESR-4236)
 - Strong-Bolt 2 by Simpson (ESR-3037)
 - Titen HD by Simpson (ESR-2713)
 - Torq-out by Simpson (ESR-2705)
 - Trubolt+ by ITW (ESR-2427)
 - Tapcon/Sammy Anchors by ITW (ESR-2202)
 - Power-Stub+ S02 by Dewart (ESR-2502)
 - Power-Stub+ S04 and S06 Stainless by Dewart (ESR-2502)
 - Snake+ by Dewart (ESR-2272)
 - Screw-Bolt+ by Dewart (ESR-3889)
 - Mini Undercut+ by Dewart (ESR-3812)
 - The Contractor may also submit for review and approval, the manufacturer's ICC evaluation report of alternate anchor systems not listed above. The alternate system shall provide minimum capacities equal to or greater than the specified anchor system. The alternate system shall be approved by the engineer of record prior to the substitution.

METAL DECKING

- Steel deck shall comply with the latest requirements of the Steel Deck Institute.
- Steel deck material shall comply with the manufacturer's ICC Report and have a minimum yield strength of 33ksi.
- All deck shall be 3-span continuous minimum. In areas where 3-span conditions are not possible, the contractor shall provide heavier gauge deck as required to provide the equivalent loading of the specified deck under a 3-span condition.
- Loads from plumbing, fire sprinklers, HVAC ducts, light fixtures, architectural elements, or equipment of any kind, may only be attached to the roof deck structural attachment and loading meets the "Suspended Loads from Metal Deck" detail supplied in the drawings.
- Conduits are permitted in deck slabs subject to local code requirements and fire rating considerations. When conduit is installed in the slab, it shall be limited to conduits the lesser of 2" in diameter or less than 1/3 the concrete thickness over the deck flutes, and that no crossovers occur, and that conduit is spaced at least 18" apart with a 3/4" minimum cover and placed a minimum of 1" above top of deck flutes. Conduits shall not be run in bottom deck ribs. For conduits not able to meet spacing requirements, see typical detail for conduits in reinforced concrete over metal deck. Aluminum conduits in concrete slabs shall be coated or covered to prevent aluminum - concrete reaction and electrolytic action between aluminum and steel.
- All members supporting deck shall be dry before welding.
- Crimp seams before button punching or welding interlocking seams.
- Where deck is to receive sprayed-on fire proofing, painted deck shall be coated with special paint that will allow the sprayed-on fire proofing to adhere to the painted deck.
- All welds performed on roof deck or galvanized deck are to be painted.
- Steel deck shall be galvanized (G60) when used above or below mechanical equipment rooms.
- Steel floor deck shall be phosphatized/painted, composite, with interlocking side seams with the following minimum properties (LW = Light weight concrete, NW = Normal weight concrete):
Type/Ga-----S(n)3(n)-----I(n)4(n)-----Concrete (Total)-----Allowable shear value-----Notes
W3/20-----0.510-----0.907-----3 1/4"(6 1/4") LW-----1900 plf for 11'-0" span-----Typ. Floor
- Weld deck to supporting framing members with 3/4" diameter puddle welds at the following spacing (Closer spacings may be used to develop minimum shear requirements):
 - 12" on center to supports perpendicular to deck corrugations (4 welds per 36" wide sheet).
 - 12" on center to all supports parallel to deck corrugations.
- Attach interlocking seams with 3/16" diameter button punch at 18" on center or 1 1/2" top seam weld at 36" on center between adjacent pieces of deck.
- Provide a 2" minimum bearing at all supports.
- Butt all end splices.

DEFERRED SUBMITTALS

- Deferred submittals are items that are not part of our scope which require architectural and/or engineering review. Deferred submittals include plans, details, calculations and/or other relevant design information stamped by a Professional Engineer licensed in the state in which construction will occur.
- Deferred submittals shall first be submitted to the project architect and/or engineer for review and coordination. Upon completion of the architect/engineer review, the architect/engineer will submit the deferred submittals to the Building Official for review and approval. The submittal to the Building Official shall include a notation stating that the architect/engineer review has been performed and that the plans and calculations for the deferred submittal items are found to be in general conformance with the design drawings with no exceptions.
- Construction related to deferred submittals shall not commence until the Building Official has approved the submittal. Approved deferred submittals shall be available at the jobsite throughout construction.
- Items requiring deferred submittals are listed below. These items shall be designed and fabricated by the manufacturer according to specifications given in the construction documents.
 - Exterior Façade Framing and Connections (by supplier) showing compliance with drift requirements
 - Steel Stairs and handrails (by steel stair manufacturer). Stair suppliers to provide any additional required support steel beyond that of the main building framing system shown on the design drawings. Steel stairs shall be designed to the deflection limits in the Basis of Design notes, and shall accommodate lateral building drifts listed in the Maximum Story Drift Table between adjacent floors, both perpendicular and parallel.
 - Seismic Bracing for mechanical, electrical and plumbing components per ASCE 7, Chapter 13 (by MEP consultant)
 - Gouged Mesopiles (by supplier). The jurisdiction may require this submittal to be included with permit.
 - Building Restraint Braces and their connections to structures (by supplier) to meet the applicable requirements of ASCE 34.

ALBANY - BID SET
08/02/2023

ALBANY
Engineered Composites

GxA
GerouxArchitects@gmail.com

Subconsultants

Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Van Boerum & Frank Assoc.
181 E. 5600 S.
Murray, Utah 84107

Hunt Electric, Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

Issued/Revisions

NO	Description	Date
1	PERMIT SUBMISSION	2023.06.07

Notes:
This drawing, as an instrument of service, is the property of the Architect and may not be reproduced without their permission and unless the reproduction carries their name. All design and other information shown on this drawing are for the use on the specified project only and shall not be used on other projects without written permission of the Architect.

Written dimensions shall have precedence over scaled dimensions. Contractors shall verify and be responsible for all dimensions and conditions shown on the drawing. Shop drawings shall be submitted to the Architect for approval before proceeding with fabrication.

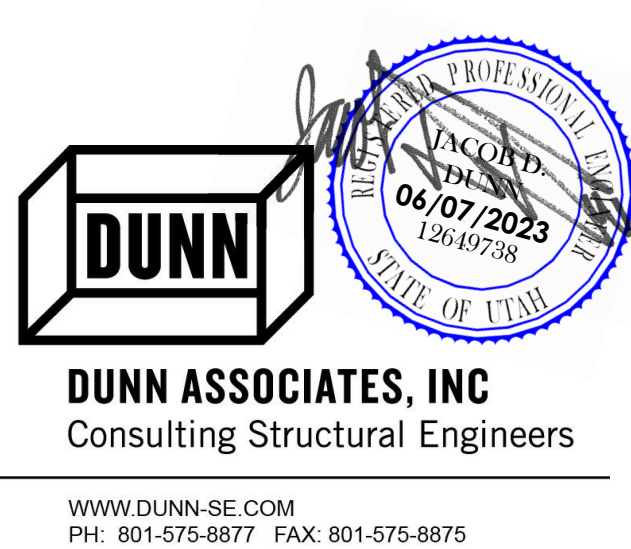
Project Name
ALBANY MEZZANINE

Sheet Title

**GENERAL
STRUCTURAL NOTES**

Scale	Date
JDD	2023.03.28
Project No.	22159

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GENERAL STRUCTURAL NOTES

SPECIAL INSPECTION, TESTING AND STRUCTURAL OBSERVATION REQUIREMENTS

1. Special Inspections and Testing
- A. Special inspections and testing as required per the approved construction documents and per IBC Chapter 17 shall be provided for this project unless waived by the Building Official.
- B. An independent agency, or agencies, employed by the Owner, shall perform the special inspection and testing services required.
- C. The special inspection and testing requirements of this section of the General Structural Notes and the special inspection tables serve as the Engineer of Record's statement of special inspections and structural observations required by IBC Chapter 17.
2. Contractor Responsibilities (1704.4)
- A. Each Contractor responsible for the construction of a main wind or seismic force-resisting system, a designated seismic system, or a wind or seismic force-resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the Building Official and Owner prior to commencing the work involved. It shall contain acknowledgment of awareness of the special requirements contained in the statement of special inspection.
- B. The Contractor shall coordinate and cooperate with all the required inspections, testing, and/or structural observations required for the project.
- C. The Contractor shall maintain access to and exposure of the work which requires special inspection or testing.
- D. The Contractor shall not proceed with subsequent work until required inspections, testing, and/or structural observations have been provided.
- E. The Contractor shall correct all work found to be deficient, and re-test at no additional cost to the Owner.
- F. The Contractor shall notify the Engineer of Record at least (7) days prior to any required structural observations.
- G. Submit all required documentation to the Special Inspector for review.
3. Special Inspector Responsibilities (1704.2)
- A. Prior to the start of the construction, each approved agency shall provide written documentation to the Building Official, demonstrating the competence and relevant experience or training of the special inspectors who will perform the special inspections and tests during construction.
- B. Special inspectors shall keep records of their inspections and testing.
- C. Inspection reports shall indicate whether the work inspected was or was not completed in conformance to the approved construction documents.
- D. Non-conforming work and/or discrepancies shall be brought to the Contractor's immediate attention for correction.
- E. The Special Inspector shall notify the Architect/Engineer of any non-conforming work or discrepancies that the Contractor cannot readily correct.
- F. Any uncorrected non-conforming work or discrepancies shall be brought to the attention of the Architect/Engineer and the Building Official prior to completion of that phase of the work.
- G. Submit the following to the Building Official:
1. Special Inspections and Testing Reports.
2. Certificates of Compliance for:
- a. Fabrication of structural elements from approved fabricators.
- b. The seismic qualifications of nonstructural components, supports, and attachments.
3. Reports of:
- a. Pre-construction tests for shotcrete.
- b. Material properties verifying compliance with the requirements of AWS D1.4 for weldability for reinforcing bars other than ASTM A706.
- c. Mill tests for ASTM A615 reinforcing bars used to resist earthquake induced forces in special moment frames, special structural walls or coupling beams in structures assigned to Seismic Design Category B, C, D, E, or F.
4. Special Inspections (1705)
- A. Special Cases (1705.1.1): Special inspection and tests shall be required for proposed work that is, in the opinion of the Building Official, unusual in its nature, such as, but not limited to, the following:
1. Construction materials and systems that are alternatives to materials and systems prescribed by the IBC.
2. Unusual design applications of materials described in the IBC.
3. Materials and systems required to be installed in accordance with additional manufacturer's instructions that prescribe requirements not contained in the IBC or in standards referenced by the IBC.
- B. Steel Construction (1705.2): The special inspections and nondestructive testing of steel construction in buildings shall be in accordance with the following:
1. Structural Steel. Special inspections and non-destructive testing of structural steel elements in buildings, structures, and portions thereof shall be in accordance with the Quality Assurance inspection requirements of AISC 360 and tables in the construction documents. Exception: Railing systems composed of structural steel elements shall be limited to welding inspection of welds at the base of cantilevered rail posts.
2. Cold-Formed Steel Deck. Special inspections and qualification of welding special inspectors for cold-formed steel floor and roof deck shall be in accordance with the Quality Assurance inspection requirements of SDI QA/QC.
- C. Quality Control Submittals for Structural Steel
1. Provide Level III non-destructive testing (NDT) personnel certifications.
2. Provide welder qualification records to verify project welders are tested in accordance with AWS D1.1 before welding structural or miscellaneous steels, D1.3 before welding sheet steels (10 gauge and thinner), and D1.4 before welding reinforcing steel. Submit documentation to the approved inspection agency for review before welding.
- a. Special, restricted welder qualification testing is required for welders joining the bottom-flange through the weld access hole connection in demand critical welds. Qualify welders for the minimum groove angle and maximum deposition rate used in production. Follow the instructions for supplemental qualification testing in AWS D1.8, Section 5.1. Qualification testing must take place within two years from the start of this project.
3. Provide welder identification methodology. The fabricator/erector shall maintain a system by which the welder who has welded a joint or member can be identified. Stamps, if used, shall be the low stress type.
4. Provide welding procedures that comply with AWS D1.1, D1.3, D1.4, D1.8, as required by the project. Welding procedures shall be made available to welders and inspectors.
- a. Provide weld filler metal product data sheets identifying optimum welding parameters and storage conditions with each welding procedure submittal.
- b. Identify the maximum welding heat input per inch of weld (KJ/in) permitted by the welding procedures.
- c. Identify the maximum deposition rate that will be used while welding on any demand critical weld.
- d. Provide typical welding filler metal Certificates of Conformance that identify the WPS Heat Input Envelope.
5. Provide a preliminary welding follow shop welding repairs be required within the Seismic Protected Zone.
6. Provide bolt storage and installation procedures to the approved inspection agency for review.
7. Provide mill/material test reports (MTR) or certificates of conformance (CC) that verify compliance of furnished materials to the requirements of the approved contract documents. MTRs or CCs are required for structural shapes, plate, metal deck, fasteners, headed studs, DBAs, weld filler metal, and bolt assemblies used as primary, load-bearing members. Maintain the heat number traceability of structural shapes and plate used as primary, load-bearing members. Maintain the heat bearing members.
- D. Structural Steel Non-Destructive Testing (NDT) Personnel Qualifications
1. NDT personnel will:
- a. Qualify in accordance with the recommended practices of the American Society of Nondestructive Testing, SNT-TC-1A, latest edition.
- b. Pass eye examinations meeting: (1) ASTM requirements at least once a year, and (2) AWS D1.1 every three years.
- c. Be certified in accordance with the AWS QC-1, latest edition.
- d. Level III must be qualified by ASNT testing in the applicable method under review.
2. Only Level II and Level III technicians, qualified by testing in the applicable method, are permitted to interpret nondestructive testing results.
3. Only Senior Certified or Certified Welding Inspectors (SCWI, CWI) are permitted to evaluate welds. Certified Associate Welding Inspectors may evaluate welds when under the direct supervision of a SCWI and/or CWI.
4. Approved Inspection Agency will certify the following:
- a. Level III inspector has reviewed the NDT procedures.
- b. Project ultrasonic testing technicians, testing demand critical welds, are trained and qualified in accordance with AWS D1.8, Annex E.
- c. That the Inspection Agencies' ultrasonic testing procedures are qualified by weld mockups similar to AWS D1.1, Annex S.
- E. Structural Steel. Special inspection and non-destructive testing (NDT) are required during the fabrication and erection of any load-bearing members and assemblies. Special inspection, except NDT, may be waived when the work is performed in a fabricating shop, or by an erector approved by the Building Official to perform work without Special Inspection. NDT of welds completed in an approved fabricator's shop may be performed by the fabricator when approved by the Building Official. When the fabricator performs the NDT, the fabricator shall submit the NDT reports for review by the Special Inspector. Special inspection and NDT shall be provided per the special inspection tables for structural steel in the construction documents.
1. Perform all welding and welding special inspection activities in accordance with AWS D1.1, D1.3, D1.4, and D1.8, AISC 360 Chapter N, and AISC 341 Chapter J, as appropriate for the material form and welding methods employed. Approved methods and acceptance criteria are established in these codes.
2. Perform all bolting and bolting inspection activities in accordance with AISC Specification for Structural Steel Joints Using High Strength Bolts, AISC 360 Chapter N, and AISC 341 Chapter J, as applicable.
3. Non-Destructive Testing (NDT) of welds is required as follows:
- a. Ultrasonic testing (UT), magnetic particle testing (MT), penetrant testing (PT), and radiographic testing (RT), where required, shall be performed in accordance with AWS D1.1/D1.1M. Acceptance criteria shall be in accordance with AWS D1.1/D1.1M for statically loaded structures, unless otherwise designated on the design drawings or project specifications.
- b. All NDT shall be documented. NDT reports shall be distributed to the fabricator/erector, the Building Official, the Contractor, and the Architect.
- c. Amount of NDT is permitted to be reduced according to AISC 360 Chapter N and AISC 341 Chapter J if appropriate criteria are met, and if approved by the Building Official and the Engineer of Record.
- d. Requirements for structures in Seismic Design Categories C thru F:
- Ultrasonic test all complete joint penetration groove
 - Magnetic particle test or penetrant test all thermally cut surfaces of access holes for flange or web thicknesses exceeding 2". Magnetic particle test or penetrant test all thermally cut surfaces of beam copes access holes for flange or web thicknesses exceeding 1 1/2" for members of the seismic force resisting system in Seismic Design Categories C thru F. Any crack shall be deemed unacceptable.
4. Special inspections and Testing for Non-Shrink Grout are required as follows:
- a. Periodic special inspection verifying the use of required mix design.
- b. Samples of non-shrink grout shall be tested for compressive strength at least daily, with additional tests required for each additional 10 bags mixed per day.
- F. Concrete (1705.3): Special inspections and tests of concrete construction shall be performed in accordance with Table 1705.3 in the construction documents.
1. Special inspections of welding of and qualifications of special inspectors for reinforcing bars shall be in accordance with the requirements of AWS D1.4 for special inspections and for special inspector qualifications.
2. In the absence of sufficient data or documentation providing evidence of conformance to quality standards for concrete materials, the building official shall require testing in accordance with the appropriate standards and criteria for the materials.

ABBREVIATIONS

AB	Anchor Bolt	JST	Joist
ABV	Above	K	Kip(s) = 1000 Pounds
ALT	Alternate	KLF	Kips Per Linear Foot
ARCH	Architect	KSF	Kips Per Square Foot
ADD'L	Additional		
BB	Bottom Bar	LB	Pounds (#)
BLDG	Building	LOC	Location
BLNG	Blocking		
BLW	Below	MAX	Maximum
BM	Beam	MECH	Mechanical
BOTT	Bottom	MEZZ	Mezzanine
BRDG	Bridging	MFB	Moment Frame Beam
BRG	Bearing	MFC	Moment Frame Column
BTWN	Between	MFR	Manufacturer
BYND	Beyond	MIN	Minimum
		MISC	Miscellaneous
		MTL	Metal
CANT	Cantilevered		
CJ	Control Joint		
CJP	Complete Joint Penetration	NTS	Not To Scale
CL	Center Line	NS	Non-shrink
COL	Column		
CONC	Concrete	OC	On Center
CONN	Connection	OPNG	Opening
CONT	Continuous	OPP	Opposite
COORD	Coordinate		
CTR	Center	PAF	Power Actuated Fastener
		PCF	Pounds per Cubic Foot
DB	Deck Bearing	PEN	Penetrate or Penetration
DBA	Deformed Bar Anchor	PERP	Perpendicular
DOUBLE	Double	PJP	Partial Joint Penetration
DCWF	Demand Critical Weld	PL	Plate
DET	Detail	PLF	Pounds per Linear Foot
DIA	Diameter	PREFAB	Prefabricated
DM	Dimension	PSF	Pounds per Square Foot
DWG	Drawing	PSI	Pounds per Square Inch
(E)	Existing	REINF	Reinforce
EA	Each	REDD	Required
EF	Each Face	RTU	Roof Top Unit
EL	Elevation		
ELEC	Electrical	SCHED	Schedule
ENGR	Engineer	SFRS	Seismic Force Resisting System
EQ	Equal	SLM	Similar
EQUIP	Equipment	SOG	Slab on Grade
EQ SP	Equally Spaced	STD	Standard
EW	Each Way	STIFF	Stiffener
EJ	Expansion Joint	STL	Steel
EXT	Exterior	STRUCT	Structural
FLR	Floor	T&B	Top and Bottom
FND	Foundation	TB	Top Bar
FTG	Footing	TEMP	Temperature
		THRU	Through
GP	Gage	TI	Top of
GALV	Galvanized	TYP	Typical
GSN	General Structural Notes	UNO	Unless Noted Otherwise
HORIZ	Horizontal		
HSA	Headed Stud Anchor	VERT	Vertical
HSS	Hollow Structural Section		
IBC	International Building Code	WI	With
ICC	International Code Council	WWR	Welded Wire Reinforcement
INT	Interior	WP	Working Point

DEFINITION OF INSPECTION TASK ABBREVIATIONS	
O	Observe: The inspector shall observe these functions on a random, daily basis. Operations need not be delayed pending observations.
P	Perform: These inspections shall be performed prior to the final acceptance of the item.
D	Document: The inspector shall prepare reports indicating that the work has been performed in accordance with the contract documents. The report need not provide detailed measurements for joint fit-up, WPS settings, completed welds, or other individual items listed in the tables. For shop fabrication, the report shall indicate the piece mark of the piece inspected. For field work, the report shall indicate the reference grid lines and floor or elevation inspected. Work not in compliance with the contract documents and whether the noncompliance has been satisfactorily repaired shall be noted in the inspection report.

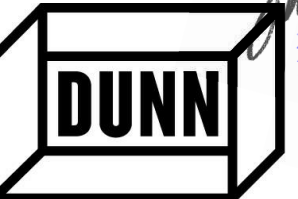
TABLE N5.6-1 COMBINED WITH TABLE J7-1 INSPECTION TASKS PRIOR TO BOLTING							
	AISC 360	AISC 341	VISUAL INSPECTION TASKS PRIOR TO BOLTING	QC		QA	
				TASK	DOC.	TASK	DOC.
1.	•		Manufacturer's certifications available for fastener materials	O	-	P	-
2.	•		Fasteners marked in accordance with ASTM requirements	O	-	O	-
3.	•	•	Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	O	-	O	-
4.	•	•	Proper bolting procedure selected for joint detail	O	-	O	-
5.	•	•	Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	O	-	O	-
6.	•	•	Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	P	D	O	D
7.	•	•	Proper storage provided for bolts, nuts, washers and other fastener components	O	-	O	-

TABLE N5.6-2 COMBINED WITH TABLE J7-2 INSPECTION TASKS DURING BOLTING							
	AISC 360	AISC 341	VISUAL INSPECTION TASKS DURING BOLTING	QC		QA	
				TASK	DOC.	TASK	DOC.
1.	•	•	Fastener assemblies of suitable condition placed in all holes and washers (if required) and nuts are positioned as required	0	-	0	-
2.	•	•	Joint brought to the snug-tight condition prior to the pretensioning operation	0	-	0	-
3.	•	•	Fastener component not turned by the wrench prevented from rotating	0	-	0	-
4.	•	•	Fasteners are pretensioned in accordance with the RCSC Specification progressing systematically from the most rigid point toward the free edges	0	-	0	-


TABLE N5.6-3 COMBINED WITH TABLE J7-3 INSPECTION TASKS AFTER BOLTING							
	AISC 360	AISC 341	VISUAL INSPECTION TASKS AFTER BOLTING	QC		QA	
				TASK	DOC.	TASK	DOC.
1.	•	•	Document acceptance or rejection of bolted connections	P	D	P	D

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NO.	Description	Date

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Project Name
ALBANY MEZZANINE

Sheet Title

GENERAL
STRUCTURAL NOTES

Scale	Date
Drawn JDD	2023.03.28
Project No.	22159

S-002

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

A

B

C

D

GENERAL STRUCTURAL NOTES

TABLE 1705.6: REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	-	X
2. Verify excavations are extended to proper depth and have reached proper material.	-	X
3. Perform classification and testing of compacted fill materials.	-	X
4. Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.	X	-
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	-	X

TABLE 1705.7: REQUIRED SPECIAL INSPECTIONS AND TESTS OF DRIVEN DEEP FOUNDATION ELEMENTS

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. Verify element materials, sizes and lengths comply with the requirements.	X	-
2. Determine capacities of test elements and conduct additional load tests, as required.	X	-
3. Inspect driving operations and maintain complete and accurate records for each element.	X	-
4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element.	X	-
5. For steel elements, perform additional special inspections in accordance with Section 1705.2.	-	-
6. For concrete elements and concrete-filled elements, perform tests and additional special inspections in accordance with Section 1705.3.	-	-
7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge.	-	-

TABLE J9-1 INSPECTION OF COMPOSITE STRUCTURES PRIOR TO CONCRETE PLACEMENT					
INSPECTION OF COMPOSITE STRUCTURES PRIOR TO CONCRETE PLACEMENT	QC		QA		
	TASK	DOC.	TASK	DOC.	
Material identification of reinforcing steel (Type/Grade)	0	-	0	-	
Determination of carbon equivalent for reinforcing steel other than ASTM A706	0	-	0	-	
Proper reinforcing steel size, spacing and orientation	0	-	0	-	
Reinforcing steel has not been bent in the field	0	-	0	-	
Reinforcing steel has been tied and supported as required	0	-	0	-	
Required reinforcing steel clearances have been provided	0	-	0	-	
Composite member has required size	0	-	0	-	

TABLE J9-2 INSPECTION OF COMPOSITE STRUCTURES DURING CONCRETE PLACEMENT					
INSPECTION OF COMPOSITE STRUCTURES DURING CONCRETE PLACEMENT	QC		QA		
	TASK	DOC.	TASK	DOC.	
Concrete: Material identification (mix design, compressive strength, maximum large aggreg size, maximum slump)	0	D	0	D	
Limits on water added at the truck or pump	0	D	0	D	
Proper placement techniques to limit segregation	0	-	0	-	

TABLE J9-3 INSPECTION OF COMPOSITE STRUCTURES AFTER CONCRETE PLACEMENT					
INSPECTION OF COMPOSITE STRUCTURES AFTER CONCRETE PLACEMENT	QC		QA		
	TASK	DOC.	TASK	DOC.	
Achievement of minimum specified concrete compressive strength at specified age	-	D	-	D	

TABLE J8-1 OTHER INSPECTION TASKS					
OTHER INSPECTION TASKS	QC		QA		
	TASK	DOC.	TASK	DOC.	
RBS requirements, if applicable <ul style="list-style-type: none">Contour and finishDimensional tolerance	P	D	P	D	
Protected zone - no holes and unapproved attachments made by fabricator or erector, as applicable	P	D	P	D	

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TABLE N5.4-1 COMBINED WITH TABLE J6-1 INSPECTION TASKS PRIOR TO WELDING

AISC 360	AISC 341	VISUAL INSPECTION TASKS PRIOR TO WELDING	QC		QA	
			TASK	DOC.	TASK	DOC.
•		Welder qualification records and continuity records	P	-	0	-
•		Welding procedure specification (WPSs) available	P	-	P	-
•		Manufacturer certification for welding consumables available	P	-	P	-
•	•	Material identification (type/grade)	0	-	0	-
•	•	Welder identification system ^a	0	-	0	-
•	•	Fit-up of groove welds (including joint geometry) <ul style="list-style-type: none">Joint preparationDimensions (alignment, root opening, root face, bevel)Cleanliness (condition of steel surfaces)Tacking (tack weld quality and location)Backing type and fit (if applicable)	P/O**	-	0	-
	•	Fit-up of CJP groove welds of HSS T-, Y-, and K-joints without backing (including joint geometry) <ul style="list-style-type: none">Joint preparationDimensions (alignment, root opening, root face, bevel)Cleanliness (condition of steel surfaces)Tacking (tack weld quality and location)	P	-	0	-
•	•	Configuration and finish of access holes	0	-	0	-
•	•	Fit-up of fillet welds <ul style="list-style-type: none">Dimensions (alignment, gaps at root)Cleanliness (condition of steel surfaces)Tacking (tack weld quality and location)	P/O**	-	0	-
•		Check welding equipment	0	-	0	-

^a The fabricator or erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be low stress type.

** Follow performance of this inspection task for ten welds to be made by a given welder, with the welder demonstrating understanding of requirements and possession of skills and tools to verify these items, the Perform designation of this task shall be reduced to Observe, and the welder shall perform this task. Should the inspector determine that the welder has discontinued performance of this task, the task shall be returned to Perform until such time as the inspector has re-established adequate assurance that the welder will perform the inspection tasks listed.

TABLE N5.4-2 COMBINED WITH TABLE J6-2
VISUAL INSPECTION TASKS DURING WELDING

AISC 360	AISC 341	VISUAL INSPECTION TASKS DURING WELDING	QC		QA	
			TASK	DOC.	TASK	DOC.
•	•	WPS followed <ul style="list-style-type: none">Settings on welding equipmentTravel speedSelected welding materialsShielding gas type/flow ratePreheat appliedInterpass temperature maintained (min/max)Proper position (F, V, H, OH)Intermix of filler metals avoided unless approved	0	-	0	-
	•	Use of qualified welders	0	-	0	-
•	•	Control and handling of welding consumables <ul style="list-style-type: none">PackagingExposure control	0	-	0	-
•	•	No welding over cracked tack welds	0	-	0	-
•	•	Environmental conditions <ul style="list-style-type: none">Wind speed within limitsPrecipitation and temperature	0	-	0	-
•	•	Welding techniques <ul style="list-style-type: none">Interpass and final cleaningEach pass within profile limitationsEach pass meets quality requirements	0	-	0	-
•		Placement and installation of steel headed stud anchors	P	-	P	-

TABLE N5.4-3 COMBINED WITH TABLE J6-3
VISUAL INSPECTION TASKS AFTER WELDING

AISC 360	AISC 341	VISUAL INSPECTION TASKS AFTER WELDING	QC		QA	
			TASK	DOC.	TASK	DOC.
•	•	Welds cleaned	0	-	0	-
•	•	Size, length and location of welds	P	-	P	-
•	•	Welds meet visual acceptance criteria <ul style="list-style-type: none">Crack prohibitionWeld/ base-metal fusionCrater cross sectionWeld profiles and sizeUndercutPorosity	P	D	P	D
•		Arc strikes	P	-	P	-
•		K-area ¹	P	D	P	D
•		Weld acceptance or rejection of welded joint or member	P	-	P	-
•	•	Placement of reinforcing or contouring fillet welds (if required)	P	D	P	D
•	•	Backing removed, weld tabs removed and finished, and fillet welds added (if required)	P	D	P	D
•	•	Repair activities	P	-	P	D
•		Document acceptance or rejection of welded joint or member	P	D	P	D
•		No prohibited welds have been added without the approval of the EOR.	0	-	0	-

¹ When welding of doubler plates, continuity plates or stiffeners has been performed in the K-area, visually inspect the web K-area for cracks within 3 in. (75mm) of the weld.

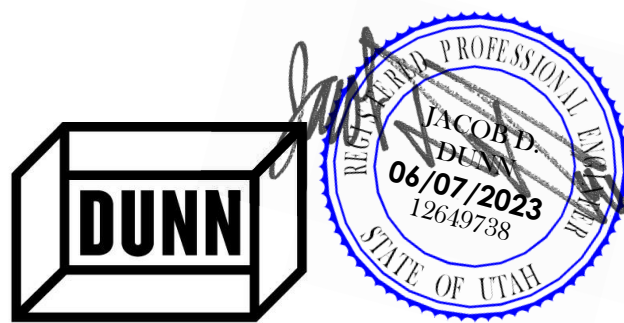
TABLE 1705.3: REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD ^a	IBC REFERENCE
1. Inspect reinforcement , including prestressing tendons, and verify placement	-	X	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. Reinforcing bar welding <ul style="list-style-type: none">a. Verify weldability of reinforcing bars other than ASTM A 706;b. Inspect single-pass fillet welds, maximum 5/16"; andc. inspect all other welds	- X	X -	AWS D1.4 ACI 318: 26.6.4	-
3. Inspect anchors cast in concrete.	-	X	ACI 318; 17.8.2	-
4. Inspect anchors post-installed in hardened concrete member: <ul style="list-style-type: none">a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loadsb. Mechanical anchors and adhesive anchors not defined in 4.a.	X -	- X	ACI 318: 17.8.2.4 ACI 318: 17.8.2	-
5. Verify use of required design mix.	-	X	ACI 318: Ch 19, 26.4.3, 26.4.4	1904.1, 1904.2 1908.2, 1908.3
6. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	-	ASTM C 172 ASTM C 31 ACI 318: 26.5, 26.12	1908.10
7. Inspect concrete and shotcrete placement for proper application techniques.	X	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. Verify maintenance of specified curing temperature and techniques.	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9. Inspect prestressed concrete for: <ul style="list-style-type: none">a. Application of prestressing forces; andb. Grouting of bonded prestressing tendons.	X X	- -	ACI 318: 26.10	-
10. Inspect erection of precast concrete members.	-	X	ACI 318: Ch. 26.9	-
11. Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	-	X	ACI 318: 26.11.2	-
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	-	X	ACI 318: 26.11.1.2(b)	-

For Sl: 1 inch = 25.4 mm.

a. Where applicable, see Section 1705.12. Special inspection for seismic resistance.

b. Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with 17.8.2 in ACI 318, or other qualification procedures. Where specific requirements are not provided, special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.



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Project Name
ALBANY MEZZANINE

Sheet Title

GENERAL
STRUCTURAL NOTES

Scale Date
2023.03.28

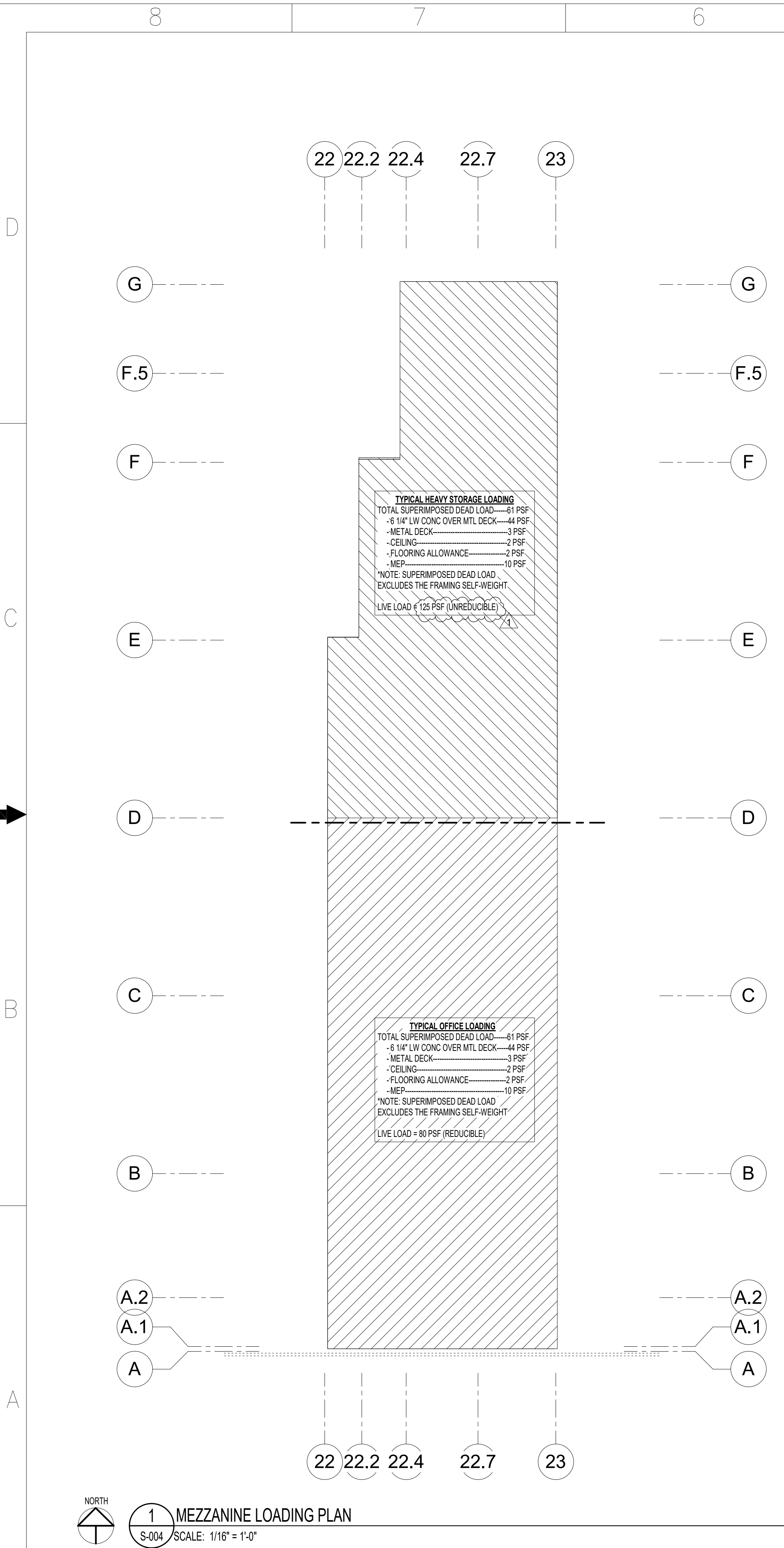
Drawn Project No.
JDD 22159

Sheet No.

S-003

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

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1 MEZZANINE LOADING PLAN
S-004 SCALE: 1/16" = 1'-0"

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Project Name
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Sheet Title
LOAD PLANS

Scale	Date 2023.03.28
Drawn JDD	Project No. 22159

S-004

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No.	Description	Date
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Project Name

ALBANY MEZZANINE

Sheet Title

FOOTING AND FOUNDATION PLAN

Scale

Date

2023.03.28

Drawn

JDD

Project No.

22159

Sheet No.

S-101

FOOTING AND FOUNDATION PLAN NOTES:

- COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
- SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS AND DEPRESSIONS.
- SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
- SEE SCHEDULES ON (S-800) SHEETS FOR:
 - FOOTINGS
 - REINFORCING SPLICE LENGTHS
 - STEEL COLUMNS
 - PILE CAPS
 - PILE CAP LOADING CRITERIA
- SEE TYPICAL FOOTING AND FOUNDATION DETAILS ON (S-500) SHEETS FOR:
 - SLAB CONSTRUCTION AND CONTROL JOINTS
 - FOOTING STEPS
 - CORNER BARS
 - PIPES PERPENDICULAR AND PARALLEL TO FOOTINGS
 - DEPRESSED SLABS
 - REINFORCING AT MISCELLANEOUS OPENING
 - REINFORCING AT SLAB DISCONTINUITIES
 - FROST COVER AND STRUCTURAL FILL
 - FLOOR OFFSETS

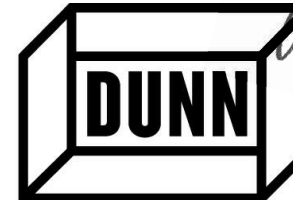
MARKS AND SYMBOL LEGEND

- SECTION MARK SHEET NUMBER
- FRAME ELEVATION SHEET NUMBER
- FTG EL FOOTING DESIGNATION TOF ELEVATION
- DEPRESSED FND WALL POUR SLAB OVER
- S S FOOTING STEP, SEE DETAILS
- FLOOR OFFSET, SEE DETAILS
- DEPRESSED SLAB, SEE ARCHITECTURAL PLANS FOR EXACT LOCATION AND ELEVATION
- CONCRETE WALL
- STEEL COLUMN
- MICROPILE, BY SUPPLIER
- CONTROL JOINT
- CP-X CONCRETE PIER, SEE SCHEDULE
- FC-X CONTINUOUS FOOTING, SEE SCHEDULE
- FD FLOOR DRAIN, SEE ARCHITECTURAL FOR EXACT LOCATION
- GB GRADE BEAM, SEE DET (8 / S-501)
- FS-X SPOT FOOTING, SEE SCHEDULE
- FTS-X THICKENED SLAB FOOTING, SEE SCHEDULE
- SBP-X STEEL BASE PLATE, SEE SCHEDULE
- SC-X STEEL COLUMN, SEE SCHEDULE
- CW-X CONCRETE WALL, SEE SCHEDULE

2023.03.28
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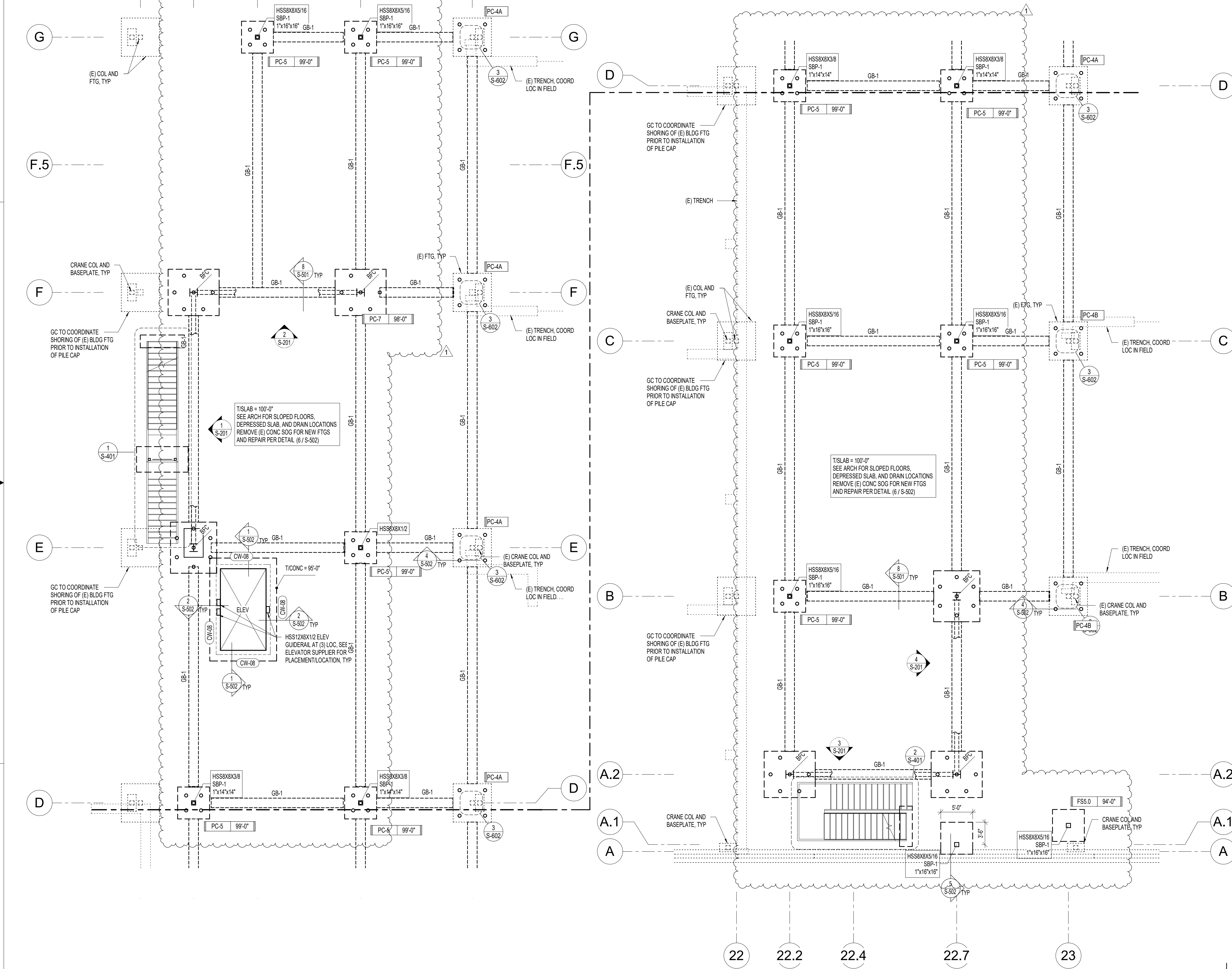
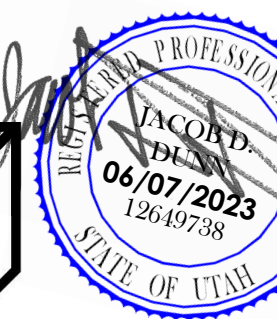
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1 FOOTING AND FOUNDATION PLAN - TOP HALF
S-101 SCALE: 1/8" = 1'-0"

2 FOOTING AND FOUNDATION PLAN - BOTTOM HALF
S-101 SCALE: 1/8" = 1'-0"

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No.	Description	Date
1	PERMIT RESUBMISSION	2023.06.07

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Project Name
ALBANY MEZZANINE

Sheet Title

MEZZANINE FRAMING
PLAN

Scale	Date
Drawn	Project No.
JDD	22159

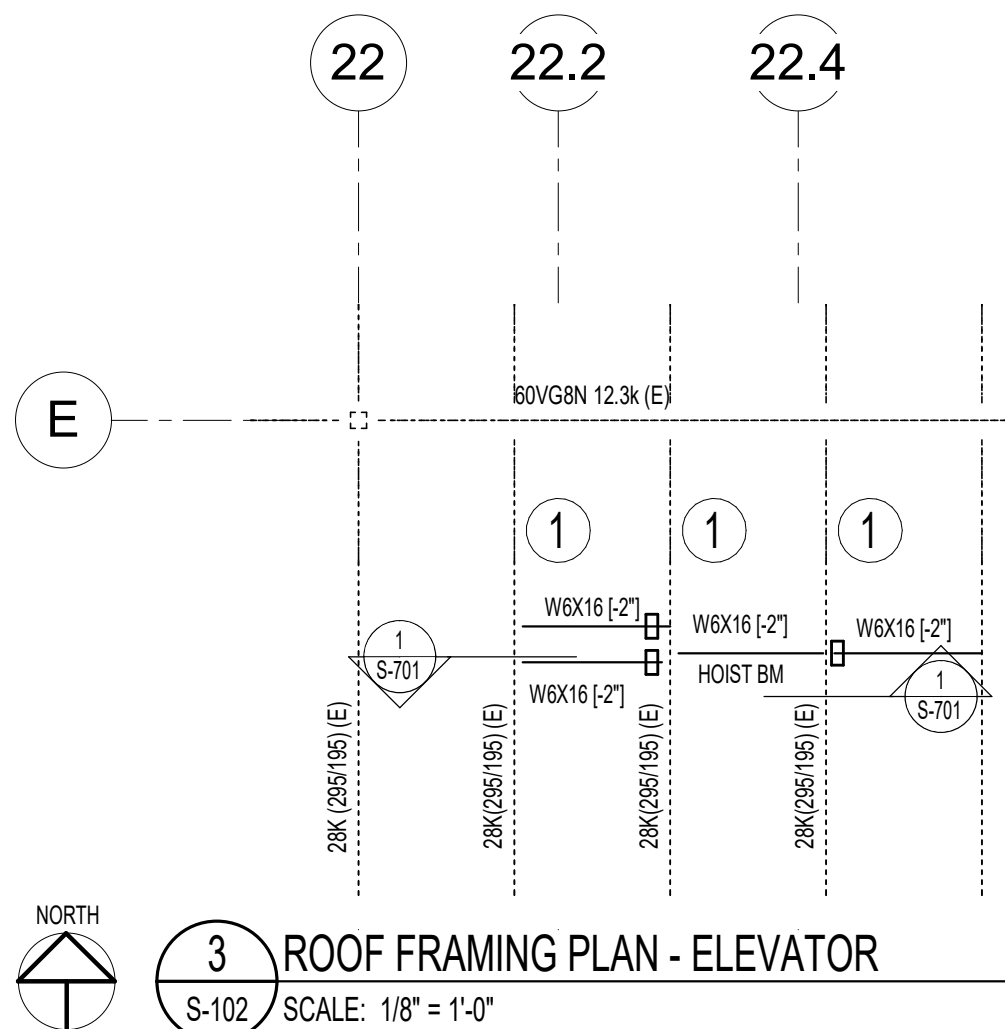
S-102

FLOOR FRAMING PLAN NOTES:

1. VERIFY WALL FLOOR OPENINGS FOR MECHANICAL SHAFTS, STAIRS, ETC. WITH ARCHITECTURAL DRAWINGS.
2. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS AND DEPRESSIONS.
3. SEE SCHEDULES ON (S-800) SHEETS FOR:
 - STEEL COLUMNS.
 - SINGLE SHEAR BEAM CONNECTIONS, TYPICAL UNLESS NOTED OTHERWISE.
 - REINFORCING SPICE LENGTHS.
4. SEE FLOOR FRAMING DETAILS ON (S-800) SHEETS FOR:
 - FRAMING AROUND MISCELLANEOUS OPENINGS.
 - DECK REINFORCEMENT AT OPENINGS.
 - DECK BEARING ANGLES AT COLUMNS.

MARKS & SYMBOLS LEGEND

- SECTION MARK SHEET NUMBER
- FRAME ELEVATION SHEET NUMBER
- CONCRETE OVER METAL DECK. SEE GENERAL STRUCTURAL NOTES.
- ADDITIONAL CONC REINF OF (2) #5 BARS x 6'-0" CTR ON CORNERS, MID-HEIGHT OF SLAB. T&B AT SLABS THICKER THAN 7".
- DEPRESSED SLAB. SEE ARCHITECTURAL PLANS FOR EXACT LOCATION AND ELEVATION.
- CONCRETE WALL
- (E) CONCRETE WALL
- STEEL COLUMN
- (E) STEEL COLUMN
- LATERAL FRAME MOMENT CONNECTIONS
- GRAVITY MOMENT CONNECTIONS
- SFRSC TOP FLANGE CJP WELD CONNECTION. SEE DETS ON (S-200) SHEETS
- LATERAL BEAM BRACING CONN PER DET (6 / S-201)
- W_X COMPOSITE STEEL BEAMS. SEE GENERAL STRUCTURAL NOTES
- (X) NUMBER OF HSA FOR SECTION OF COMPOSITE STEEL BEAM. SEE GENERAL STRUCTURAL NOTES
- (C=) REQUIRED PRE-CAMBER AT MID-SPAN OF BMS. CAMBER TOLERANCE SHALL BE +1/4", -0"
- [± X"] DIFFERENCE IN TOP OF BEAM ELEVATION FROM TYPICAL TOP OF FLOOR STEEL
- SFRSC SEISMIC FORCE RESISTING SYSTEM COLLECTOR. SEE DETS ON SHEET (S-203) FOR CONNECTIONS



3 ROOF FRAMING PLAN - ELEVATOR
S-102 SCALE: 1/8" = 1'-0"



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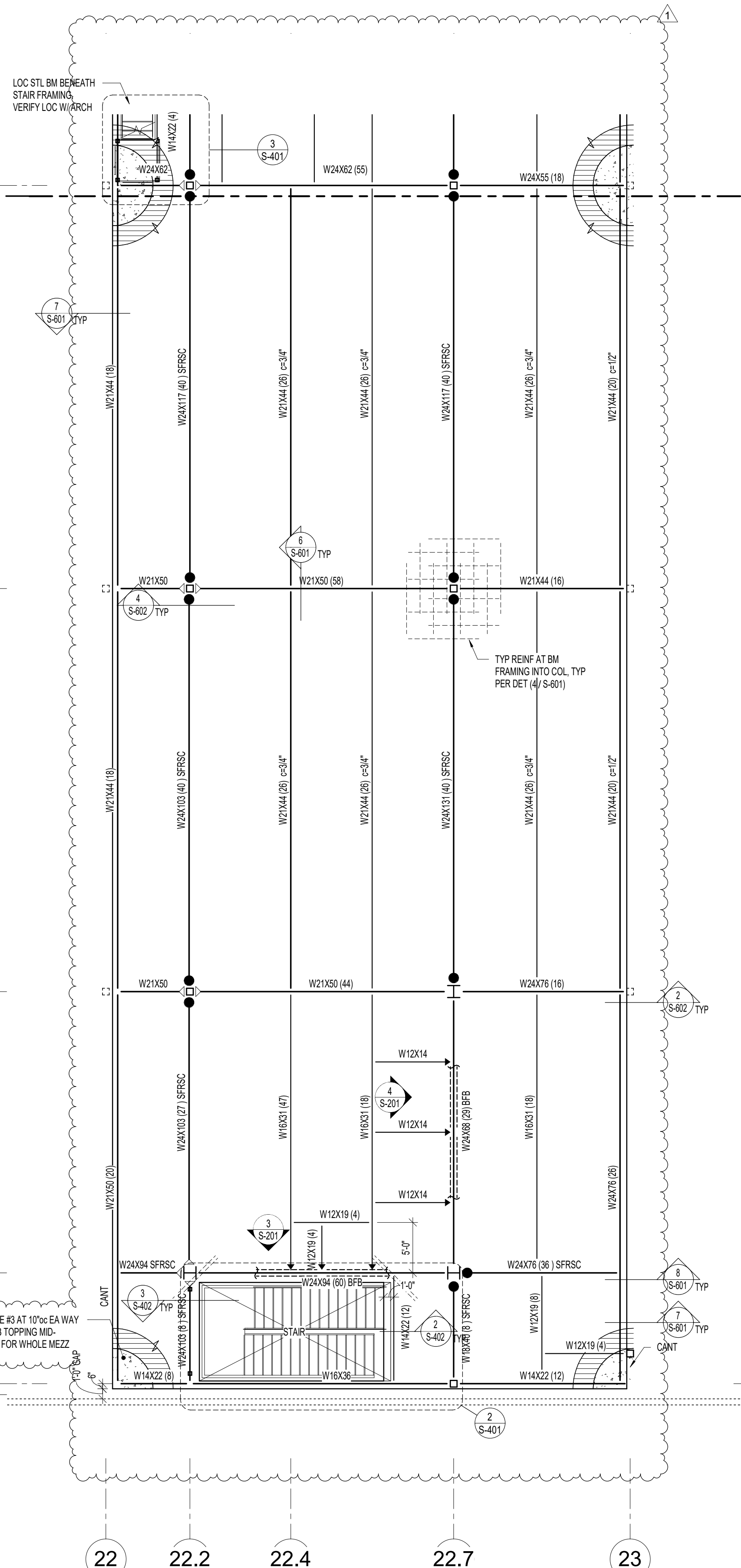
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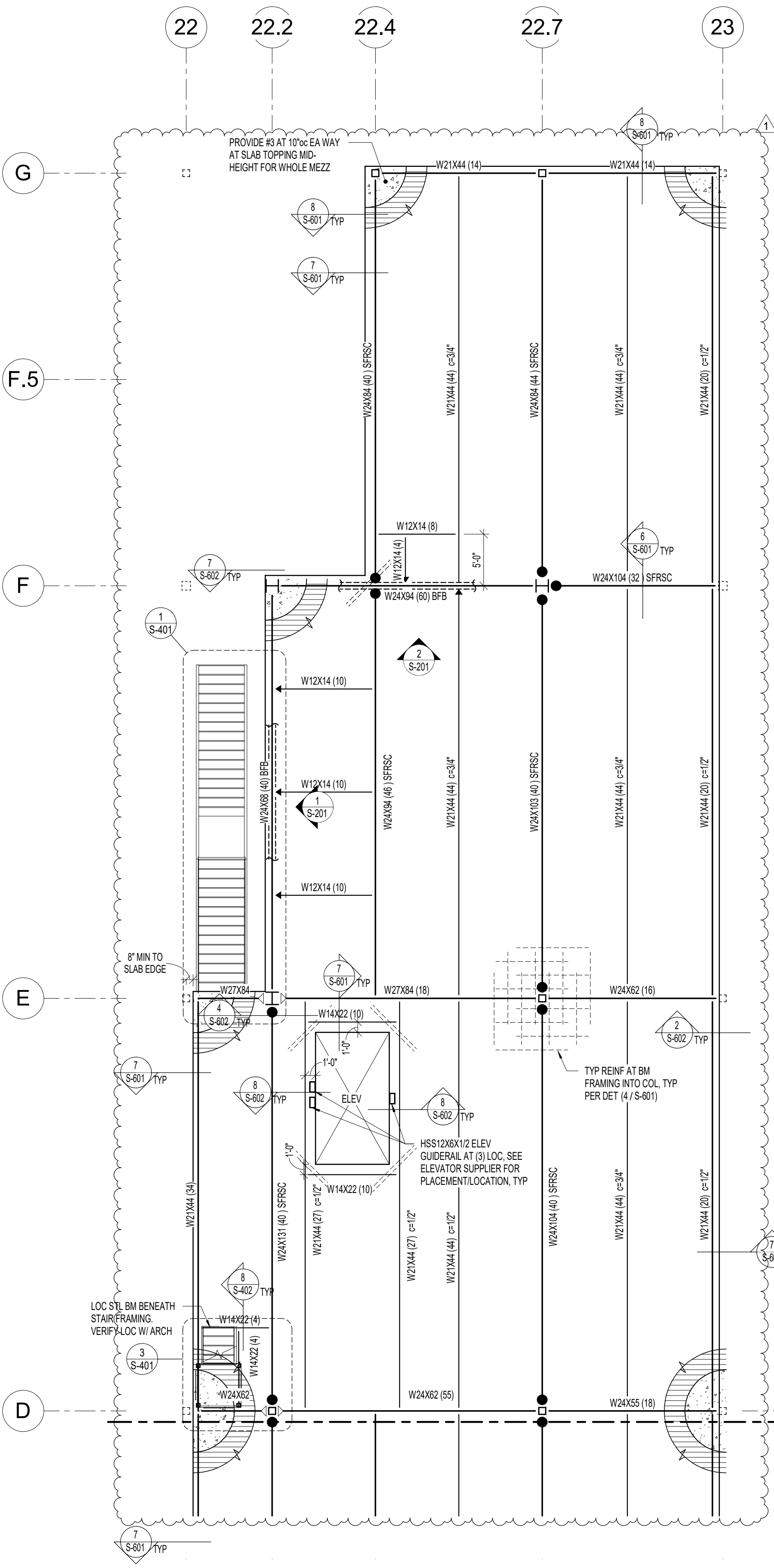
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2 MEZZANINE FRAMING PLAN - BOTTOM HALF
S-102 SCALE: 1/8" = 1'-0"



1 MEZZANINE FRAMING PLAN - TOP HALF
S-102 SCALE: 1/8" = 1'-0"

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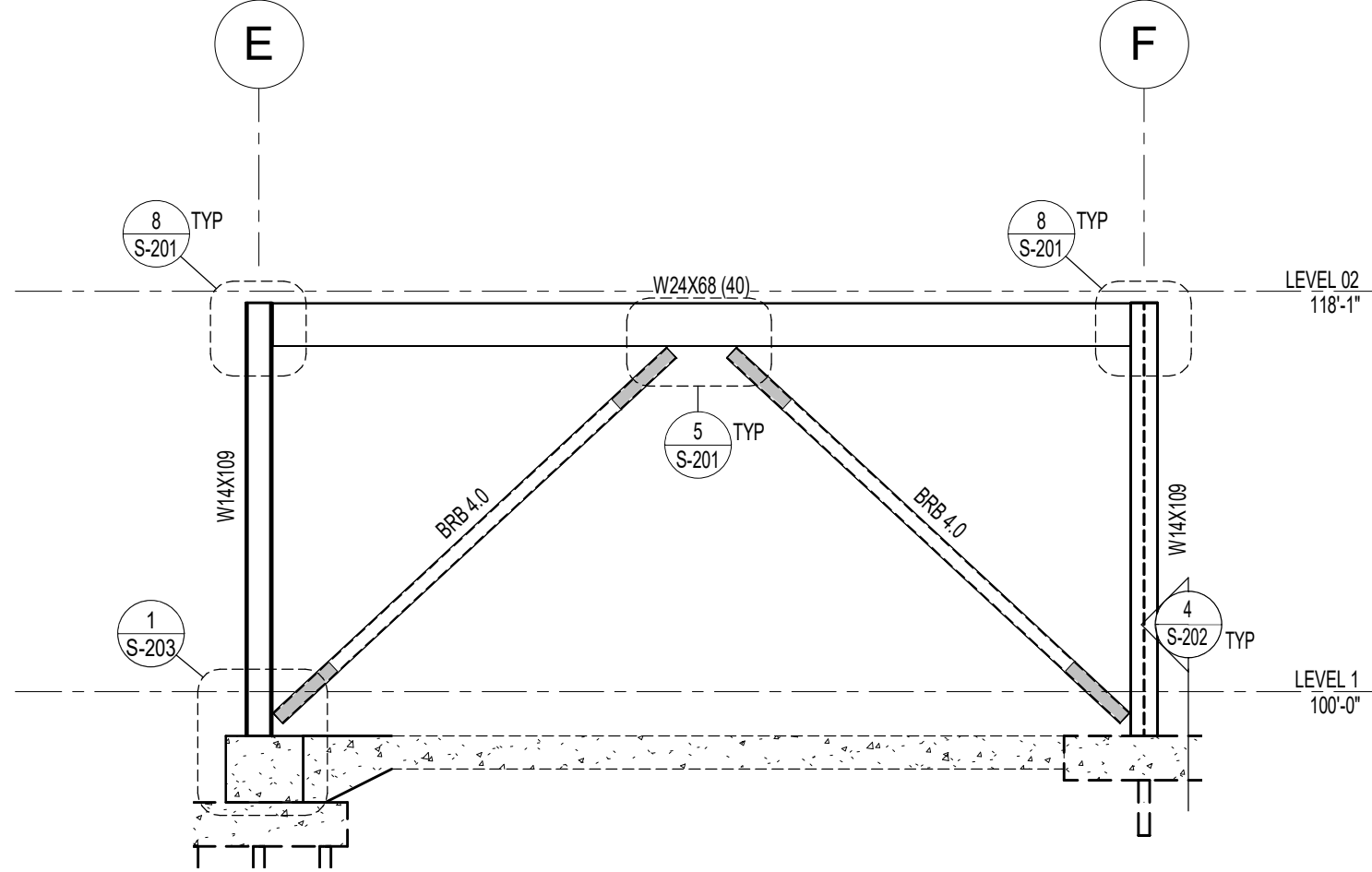
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Project Name
ALBANY MEZZANINE

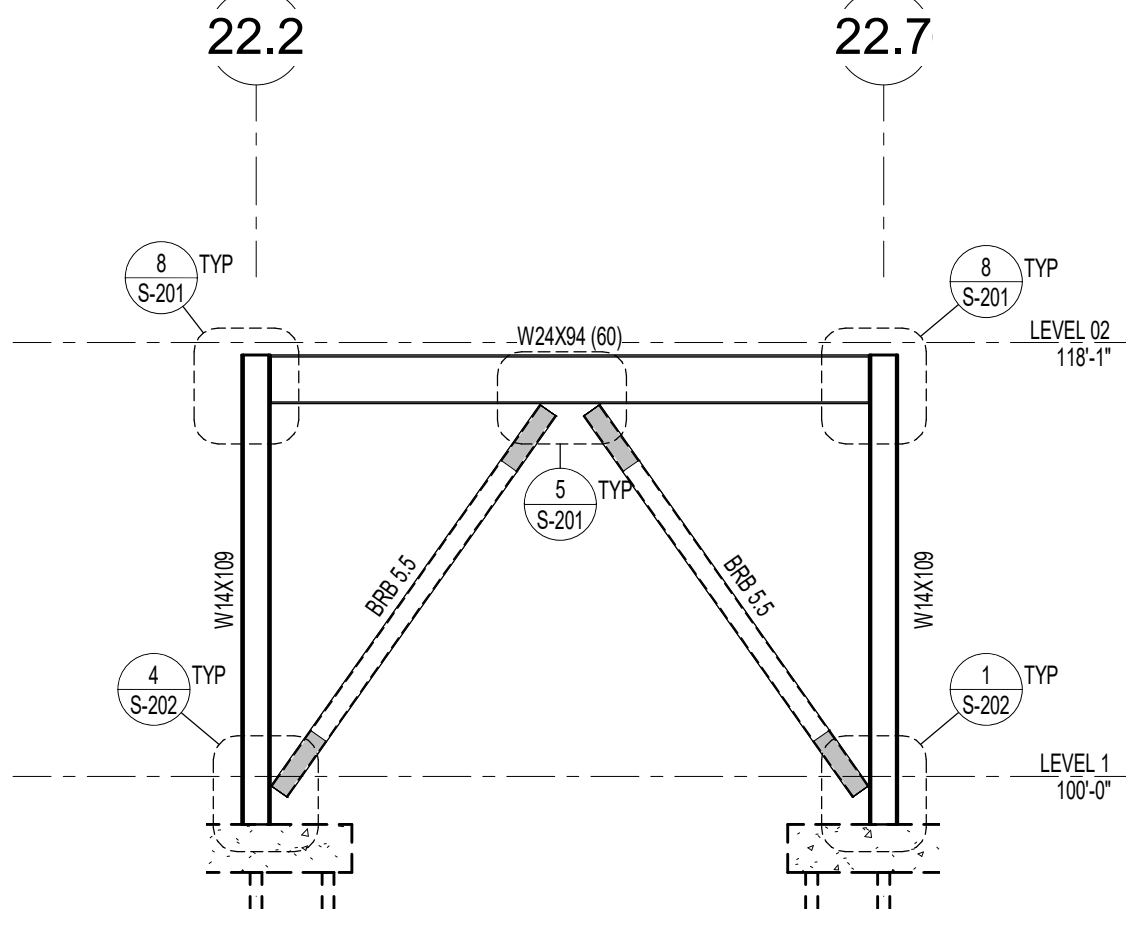
BRACED FRAME
ELEVATIONS

Scale	Date
Drawn	2023.03.28
JDD	Project No.
	22159

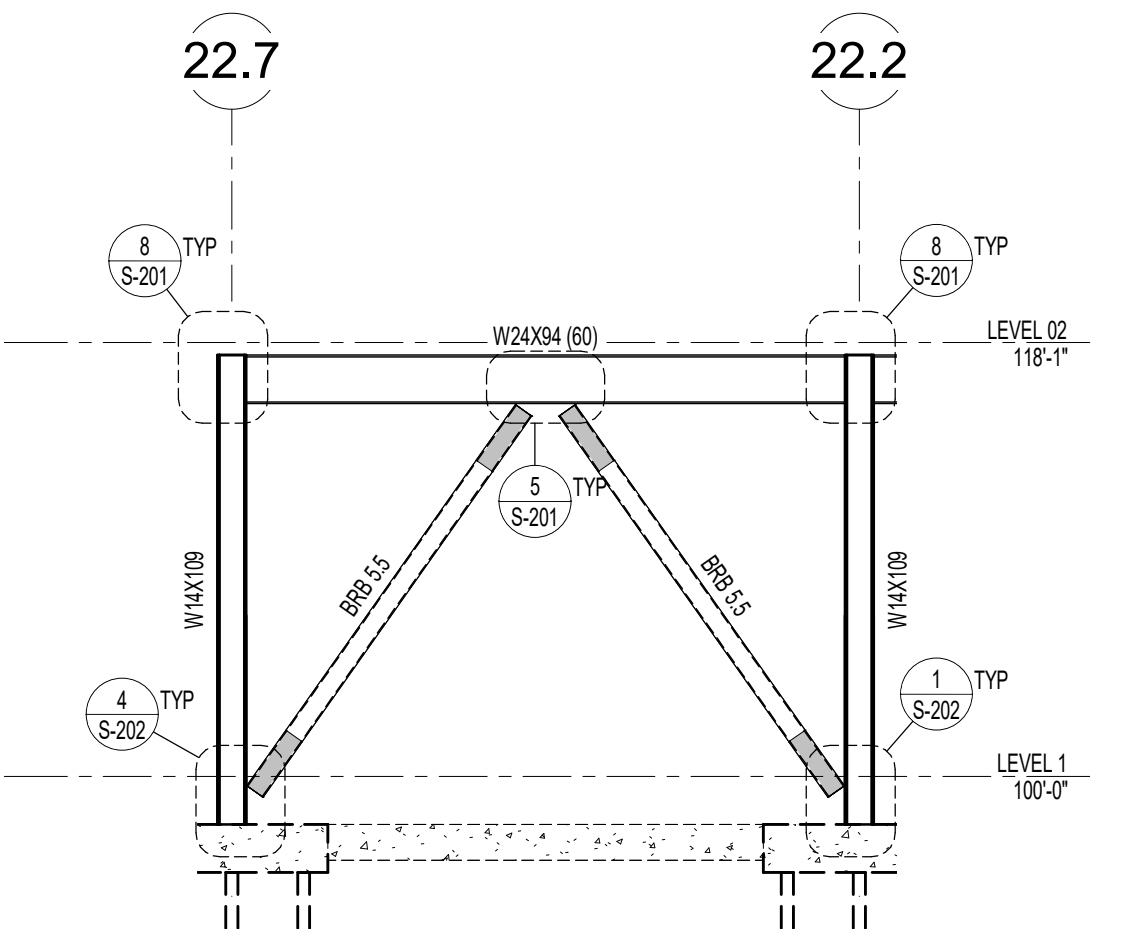
S-201



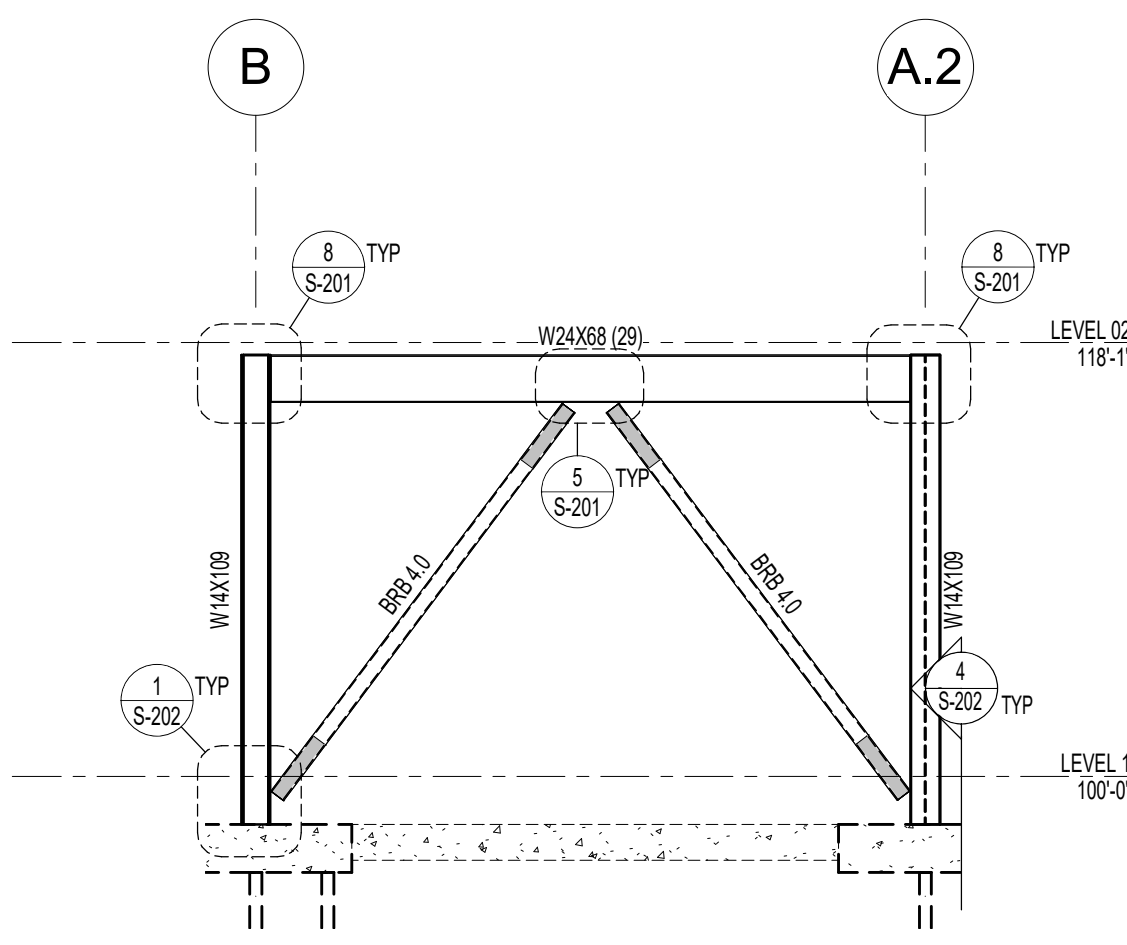
1 BRACED FRAME ELEVATION - GRID '22.2'
S-201 SCALE: 1/8" = 1'-0"



2 BRACED FRAME ELEVATION - GRID 'F'
S-201 SCALE: 1/8" = 1'-0"



3 BRACED FRAME ELEVATION - GRID 'A.2'
S-201 SCALE: 1/8" = 1'-0"



4 BRACED FRAME ELEVATION - GRID '22.7'
S-201 SCALE: 1/8" = 1'-0"

BUCKLING RESTRAINED BRACE FRAME SCHEDULE							
BRACE DESIGNATION	STEEL CORE AREA	STIFFNESS RATIO MIN*	F _y MINIMUM	F _y MAXIMUM	CORE STRENGTH PN kips	MAXIMUM BETA	MAXIMUM OMEGA
BRB 4.0	4.0	1.40	38	46	152	1.15	1.50
BRB 5.5	5.5	1.40	38	46	209	1.15	1.50

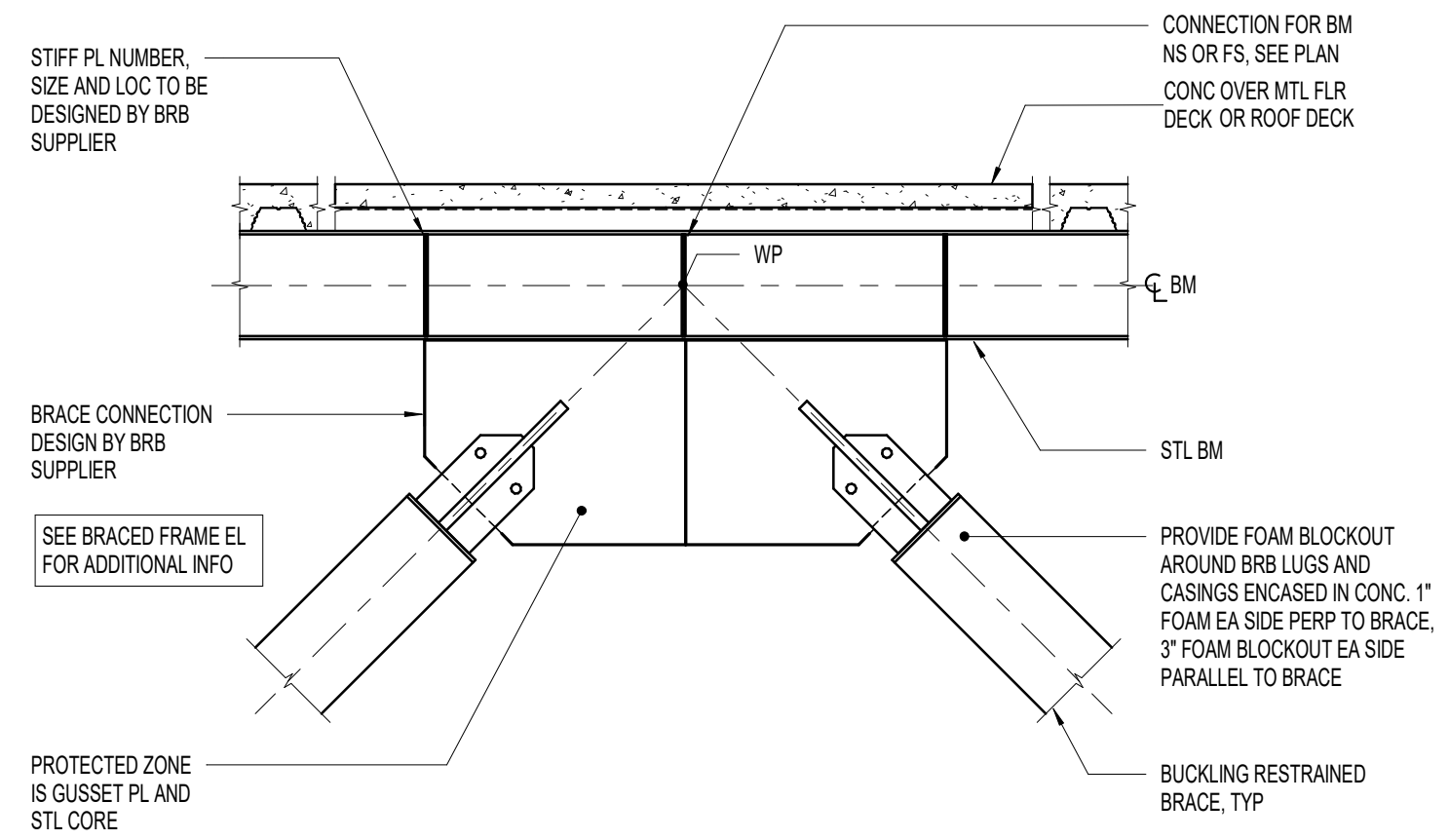
BUCKLING RESTRAINED BRACE FRAME NOTES

- CORE MATERIAL SHALL BE ASTM A36 MATERIAL SELECTED TO PROVIDE A MINIMUM TENSILE YIELD STRENGTH OF 38ksi AND MAXIMUM TENSILE YIELD STRENGTH OF 46ksi. LUG PLATE, GUSSET PLATES AND ANY REPAID PLATES SHALL BE ASTM GR50. CASINGS SHALL BE A500 GRADE B OR A50 GRADE B.
- BRB SUPPLIER SHALL DESIGN BRACE CONNECTIONS IN ACCORDANCE WITH AISC 341-16. BRB SUPPLIER IS RESPONSIBLE FOR THE DESIGN OF THE CONNECTION OF THE BRB'S TO THE GUSSET AND THE GUSSET TO THE FRAME COLUMNS AND BEAMS.
- BRB SUPPLIER SHALL PROVIDE TEST RESULTS FOR SIMILAR BRACES IN ACCORDANCE WITH AISC 341-16.
- BRB SUPPLIER SHALL PROVIDE TESTED MATERIAL ADJUSTMENT FACTORS FOR COMPRESSION STRENGTH (Ø) AND STRAIN HARDENING (Ø).
- DESIGN VALUE STIFFNESS MODIFICATION FACTOR (K_{st}) SHALL BE WITHIN ±10% OF ACTUAL STIFFNESS MODIFICATION FACTOR (K_{st}). CALCULATE WORK POINT TO WORK POINT.

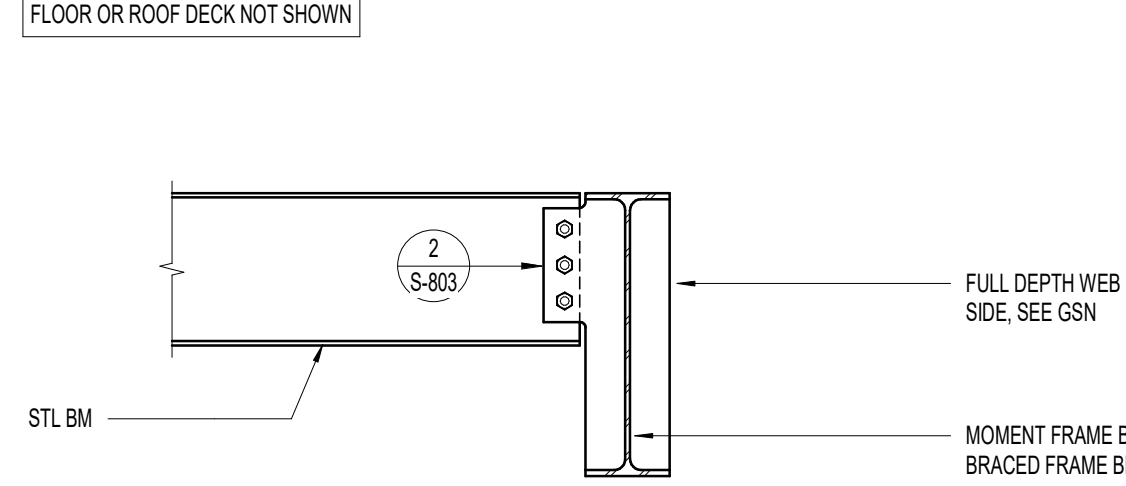
$K_{st} = K_{st} \times A_{st} \times E_{st}$, WHERE L IS THE WP LENGTH AND A IS THE SPECIFIED CORE AREA.

- MINIMUM STROKE TO OCCUR AT EACH END OF BRACE IN TENSION AND COMPRESSION DIRECTIONS. CORE EXTENSION TO REMAIN STABLE OVER 2x'S THIS LENGTH.
- BRB PROTECTED ZONES SHALL INCLUDE THE STEEL CORE AND CONNECTION ELEMENTS AND SHALL SATISFY THE REQUIREMENTS OF SECTION F4 OF THE AISC 341-16 ADDITIONALLY ATTACHMENTS WHICH PENETRATE THE CASING (SCREWS, PINS, PAF, ETC) OR ANY ATTACHMENT SUPPORTING OVER 50lbs SHALL BE APPROVED BY THE ENGINEER OF RECORD AND THE BRB MANUFACTURER.

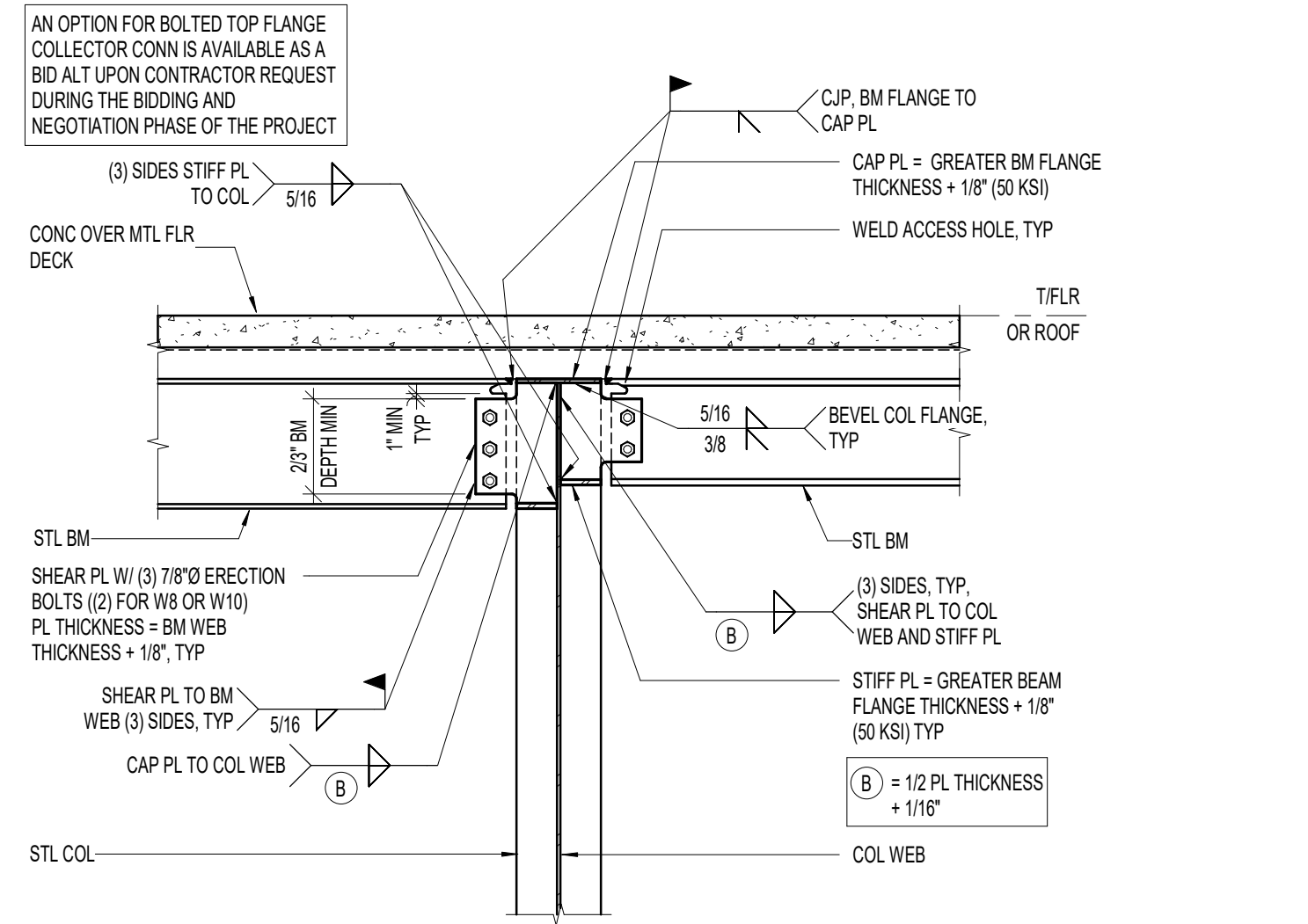
INDICATES PROTECTED ZONE: PROTECTED ZONES FOR BRB FRAMES ARE GUSSET PLATES AND BRB STEEL CORES. ANY ATTACHMENT TO BRB'S SHOULD BE APPROVED BY THE ENGINEER OF RECORD AND THE BRB SUPPLIER. SEE DETAIL (xx/xx) FOR LIMITS OF PROTECTED ZONE OF MOMENT FRAMES. WELDED, BOLTED, SCREWED OR SHOT IN ATTACHMENTS FOR PERIMETER EDGE ANGLE, EXT FACADES, PARTITIONS, DUCTWORK, PIPING, OR OTHER CONSTRUCTION SHALL NOT BE PLACED IN PROTECTED ZONES.



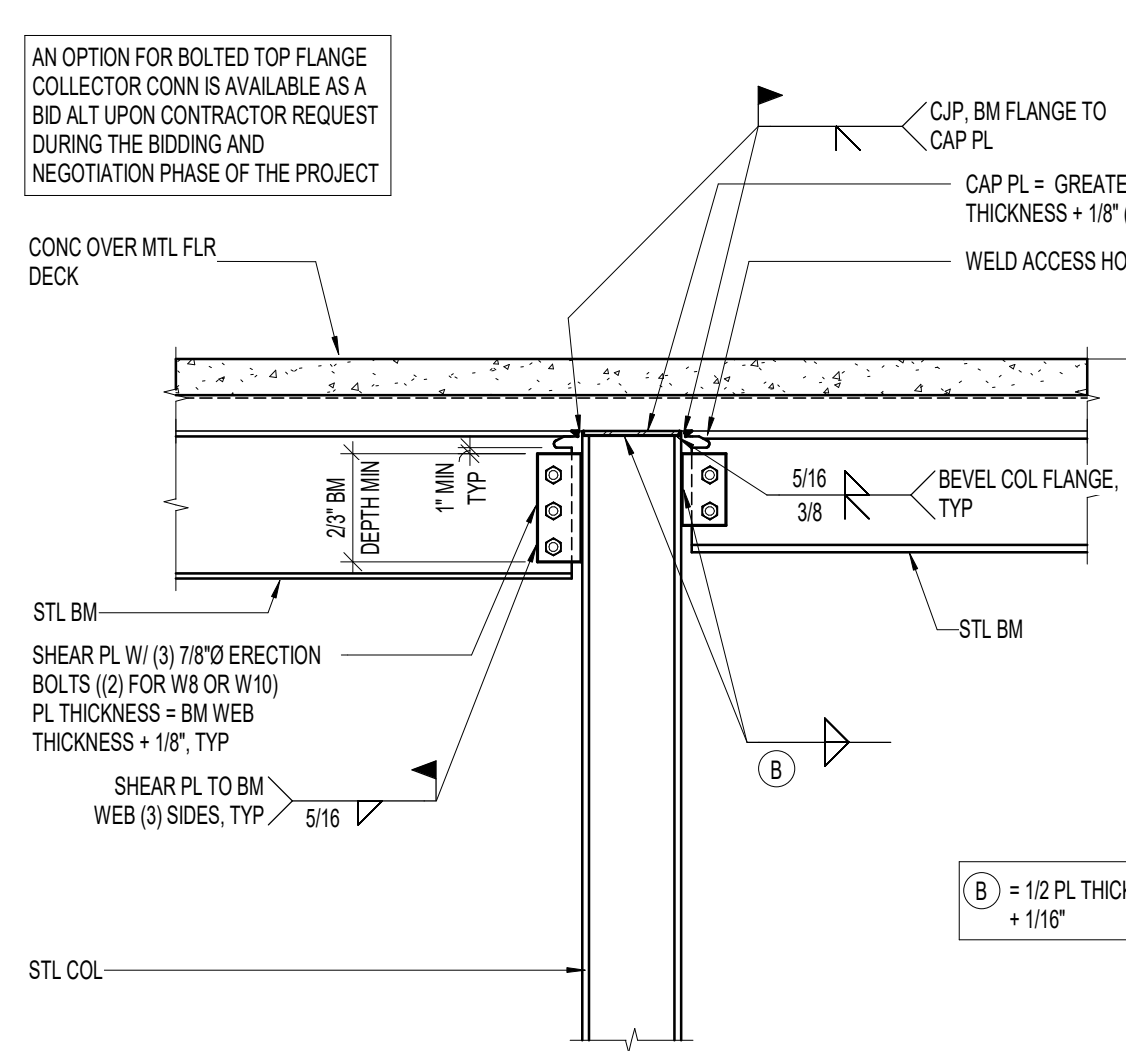
5 TYPICAL BUCKLING RESTRAINED BRACE CONNECTION AT INVERTED "V" DETAIL
S-201 NO SCALE:



6 TYPICAL STEEL BRACE BEAM
S-201 NO SCALE:



7 TYPICAL SFRS COLLECTOR CONNECTOR
S-201 NO SCALE:



8 TYPICAL SFRS COLLECTOR CONNECTOR
S-201 NO SCALE:

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2023.03.28
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1	PERMIT RE-SUBMISSION	2023.06.07

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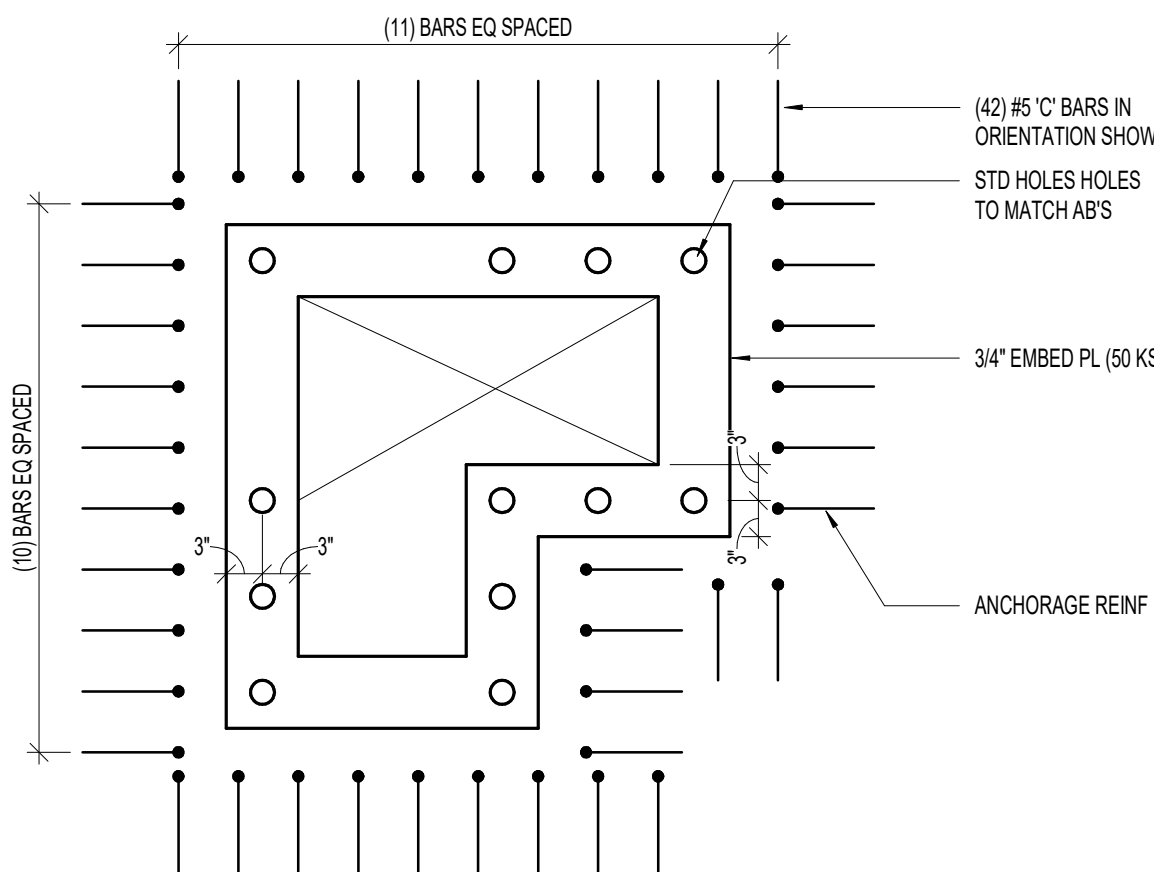
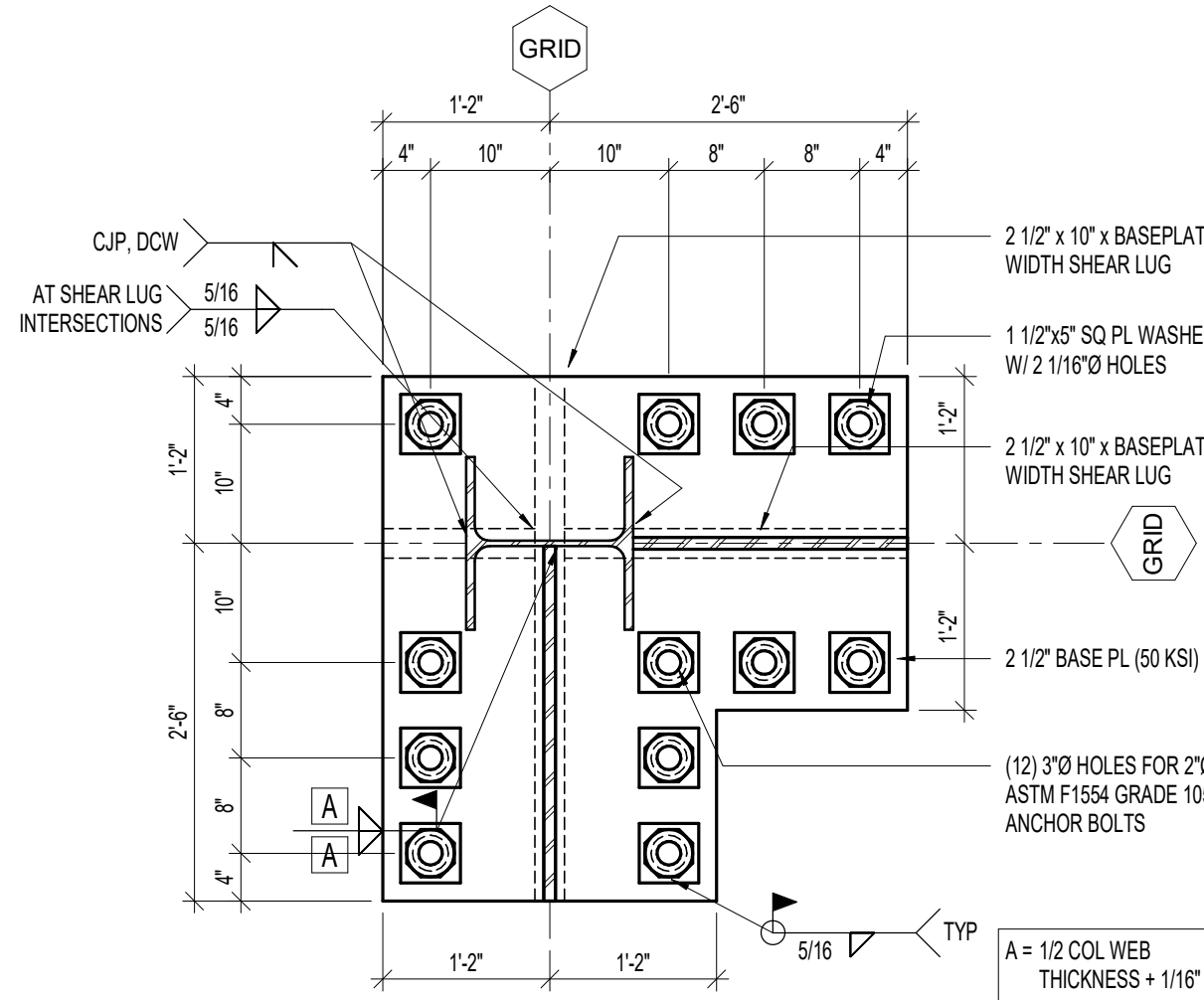
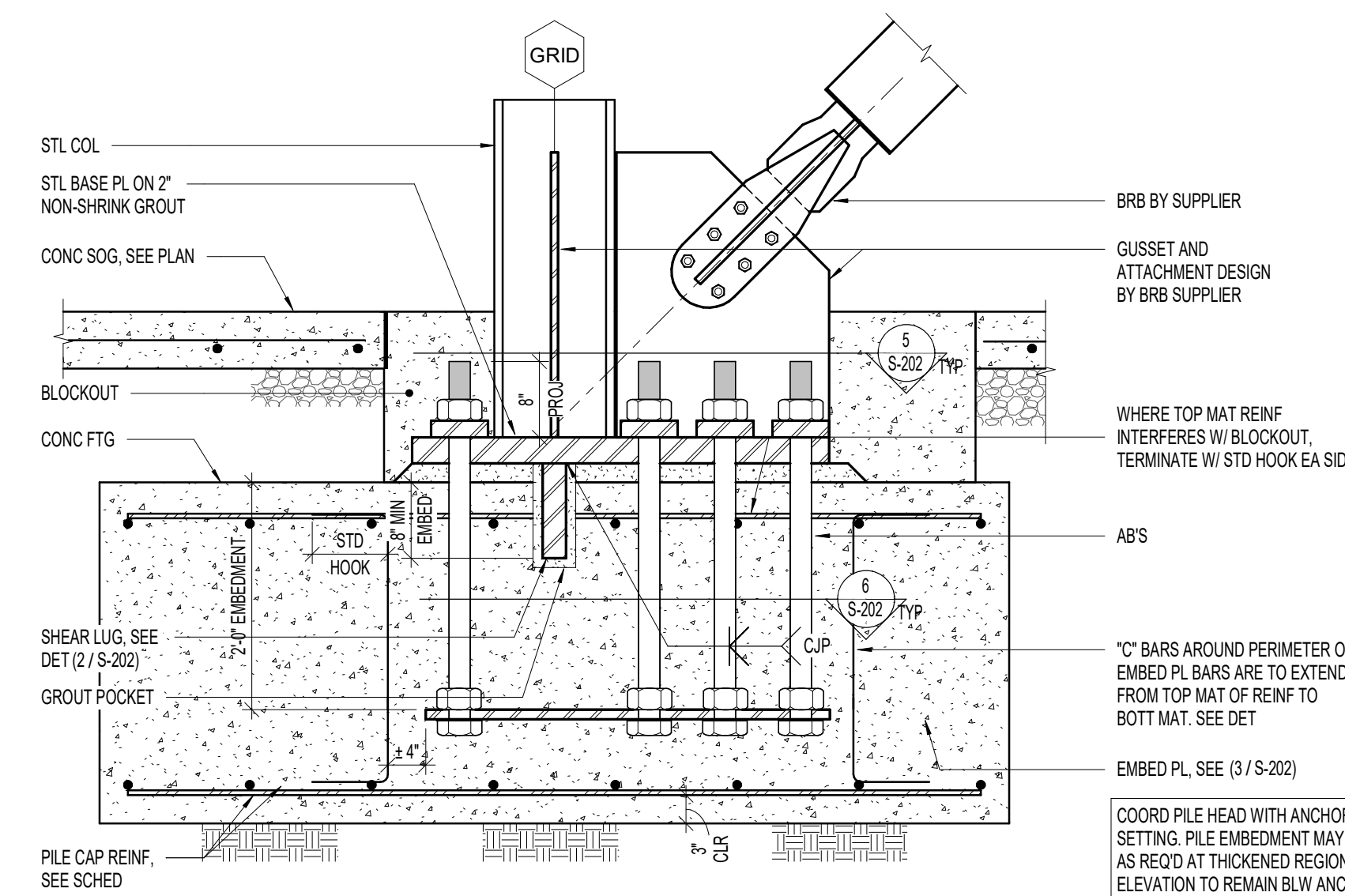
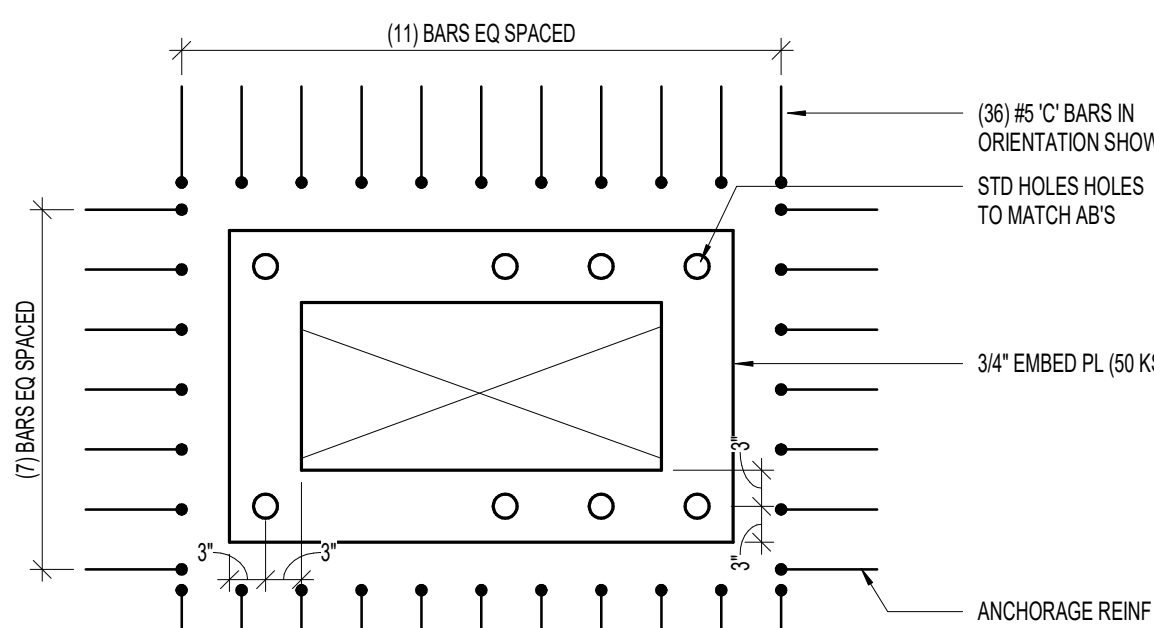
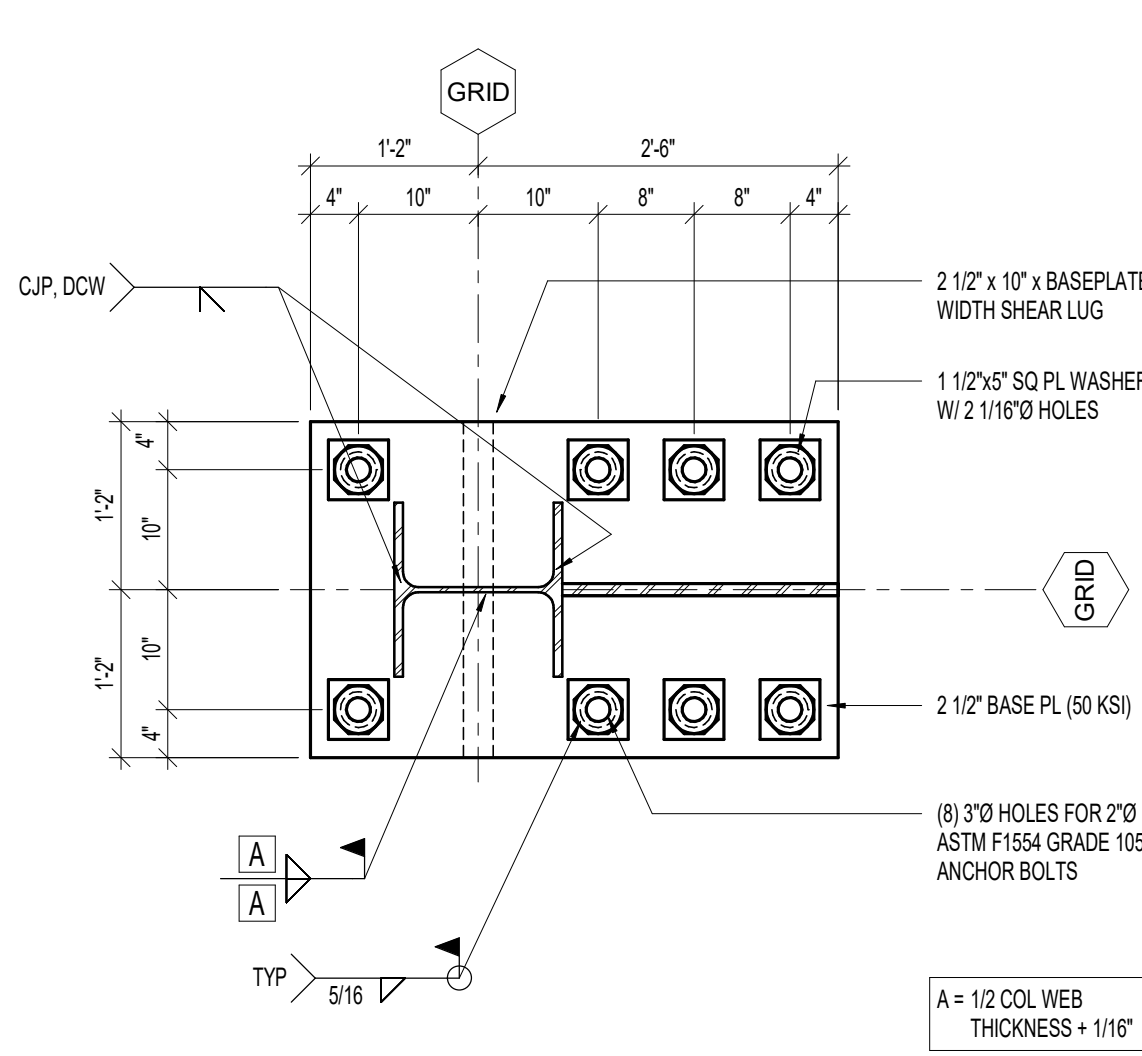
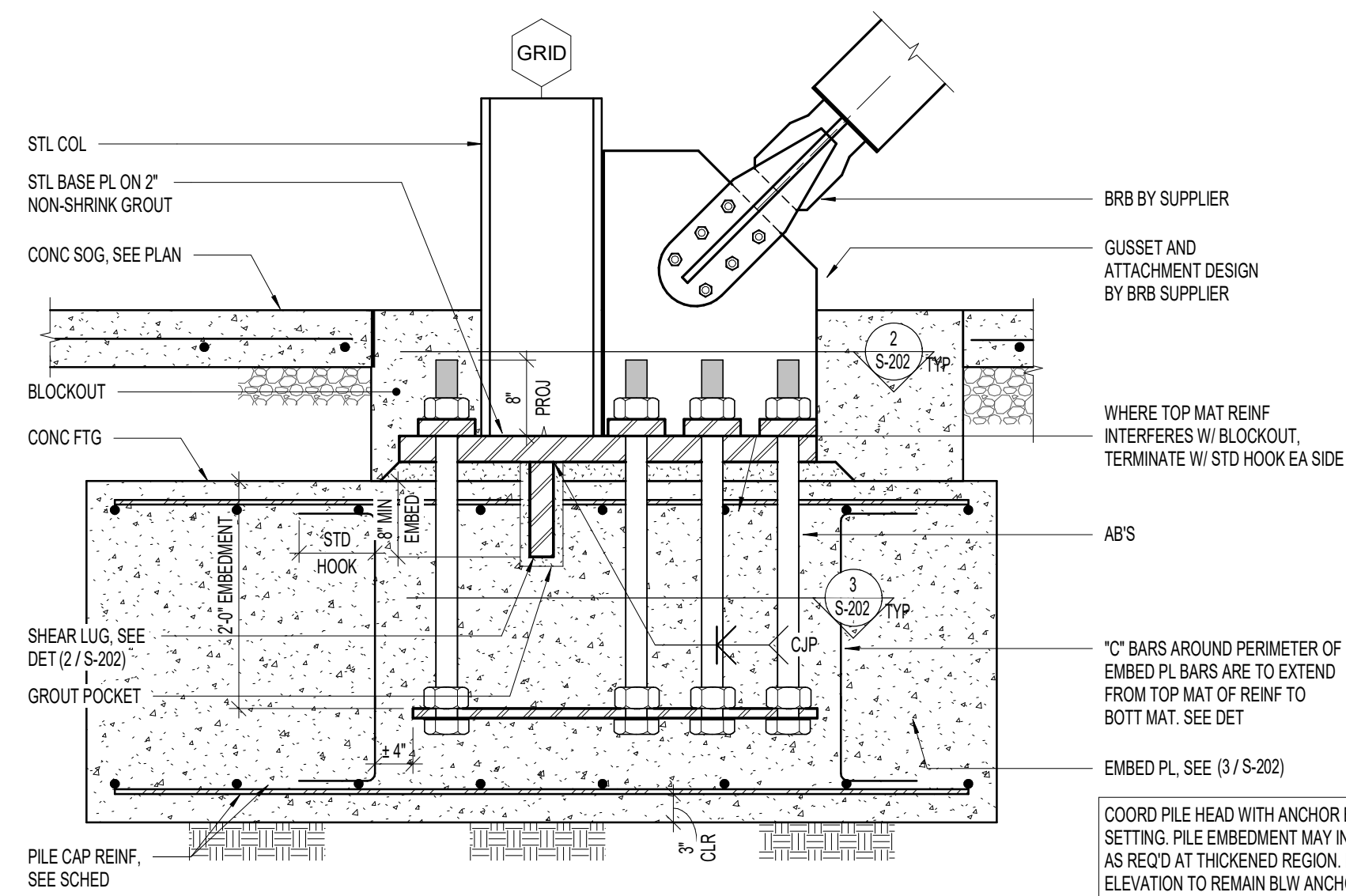
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Project Name
ALBANY MEZZANINE

BRACED FRAME
DETAILS

Scale	Date
Drawn	2023.03.28
JDD	Project No.
	22159

S-202



Issued/Revisions

No.	Description	Date
1	PERMIT RESUBMISSION	2023.06.07

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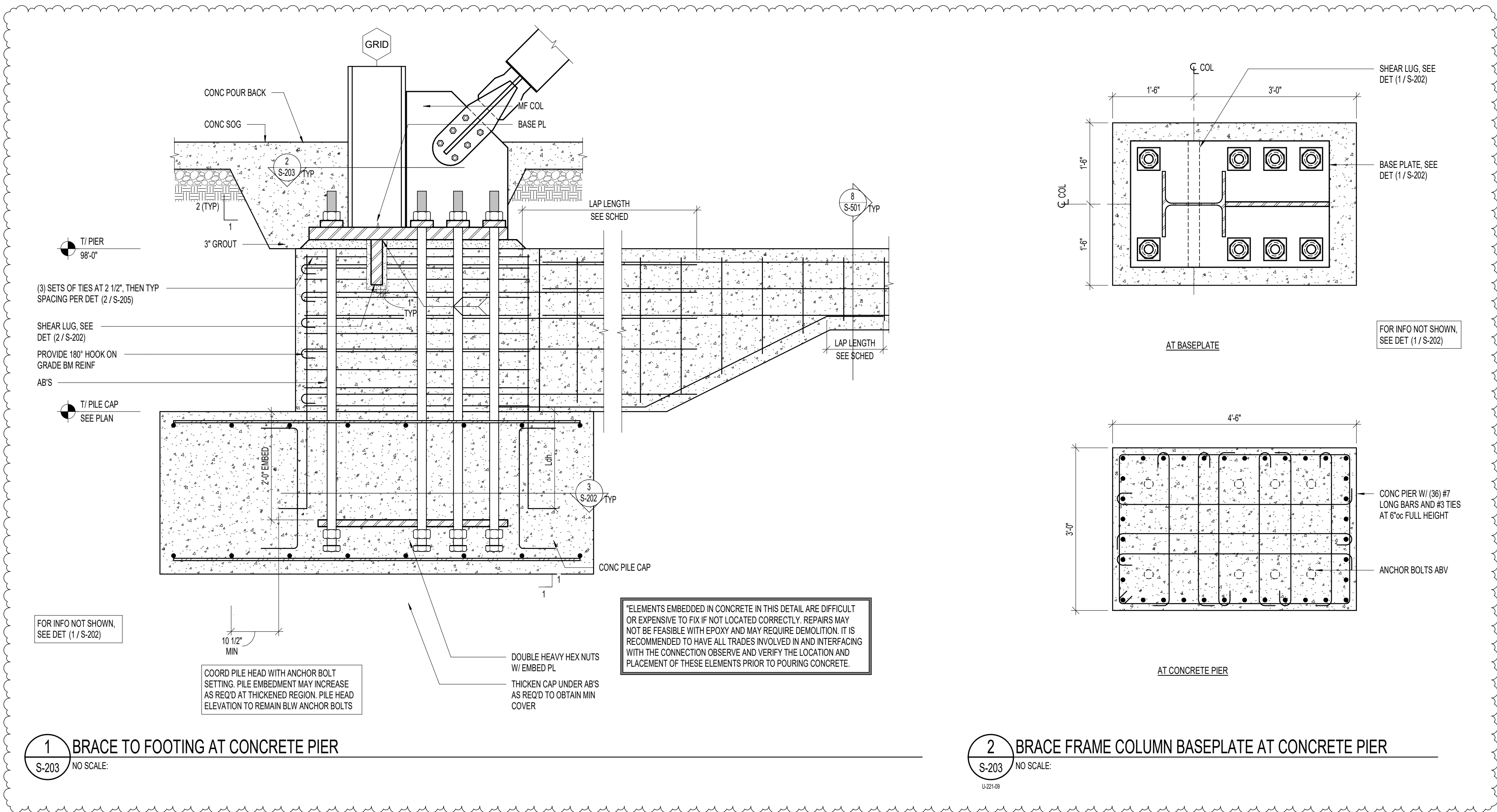
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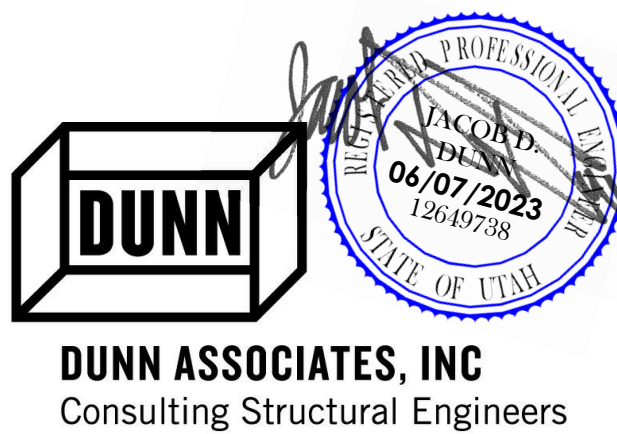
Scale	Date 2023.03.28
Drawn Author	Project No. 22159

S-203



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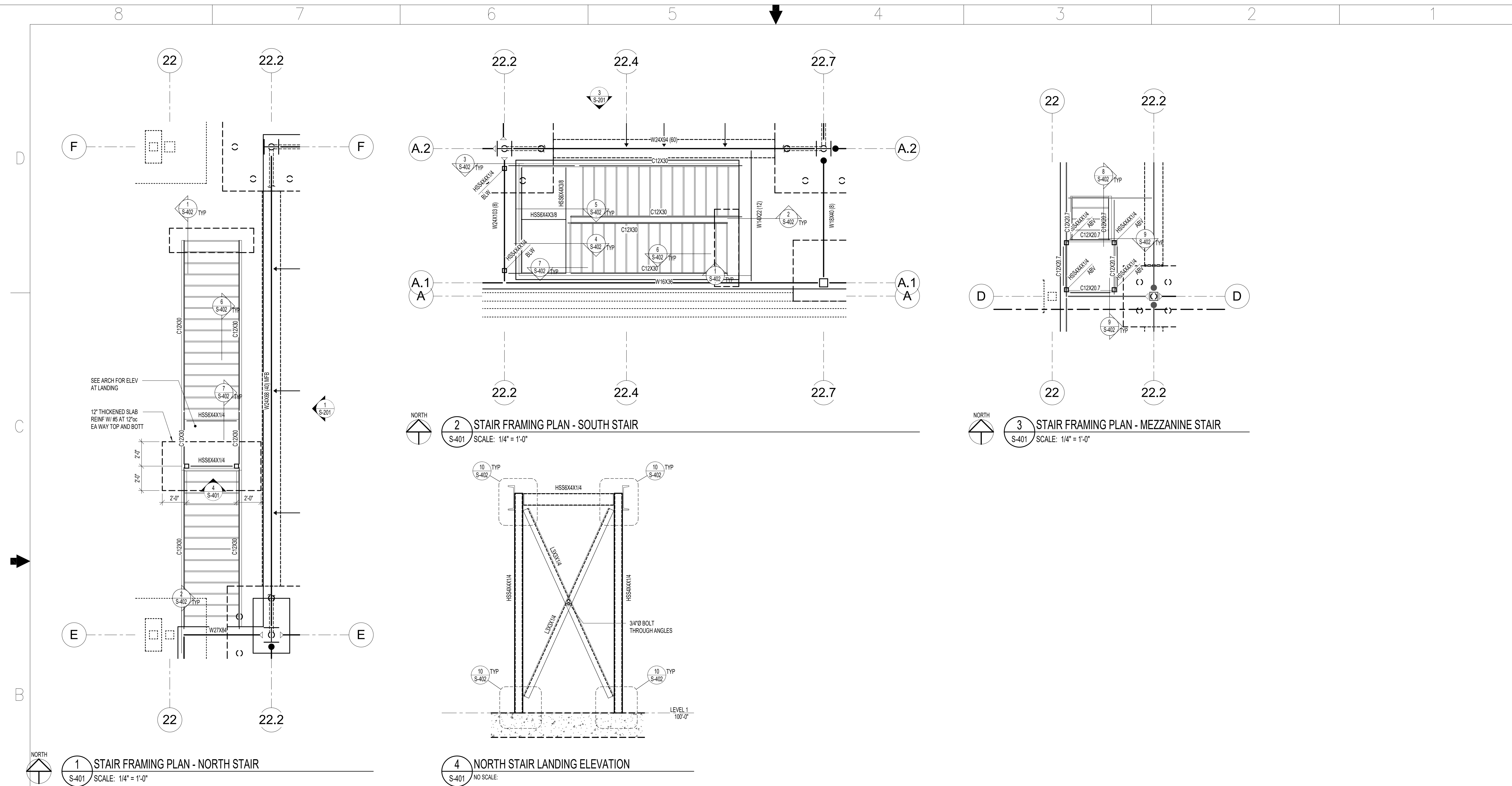
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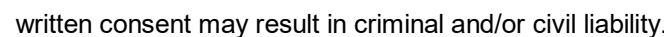
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Drawn Author	Project No. 22159







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GerouxArchitects@gmail.com

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Owner / Project Contact

Albany
Engineered
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Issued/Revisions

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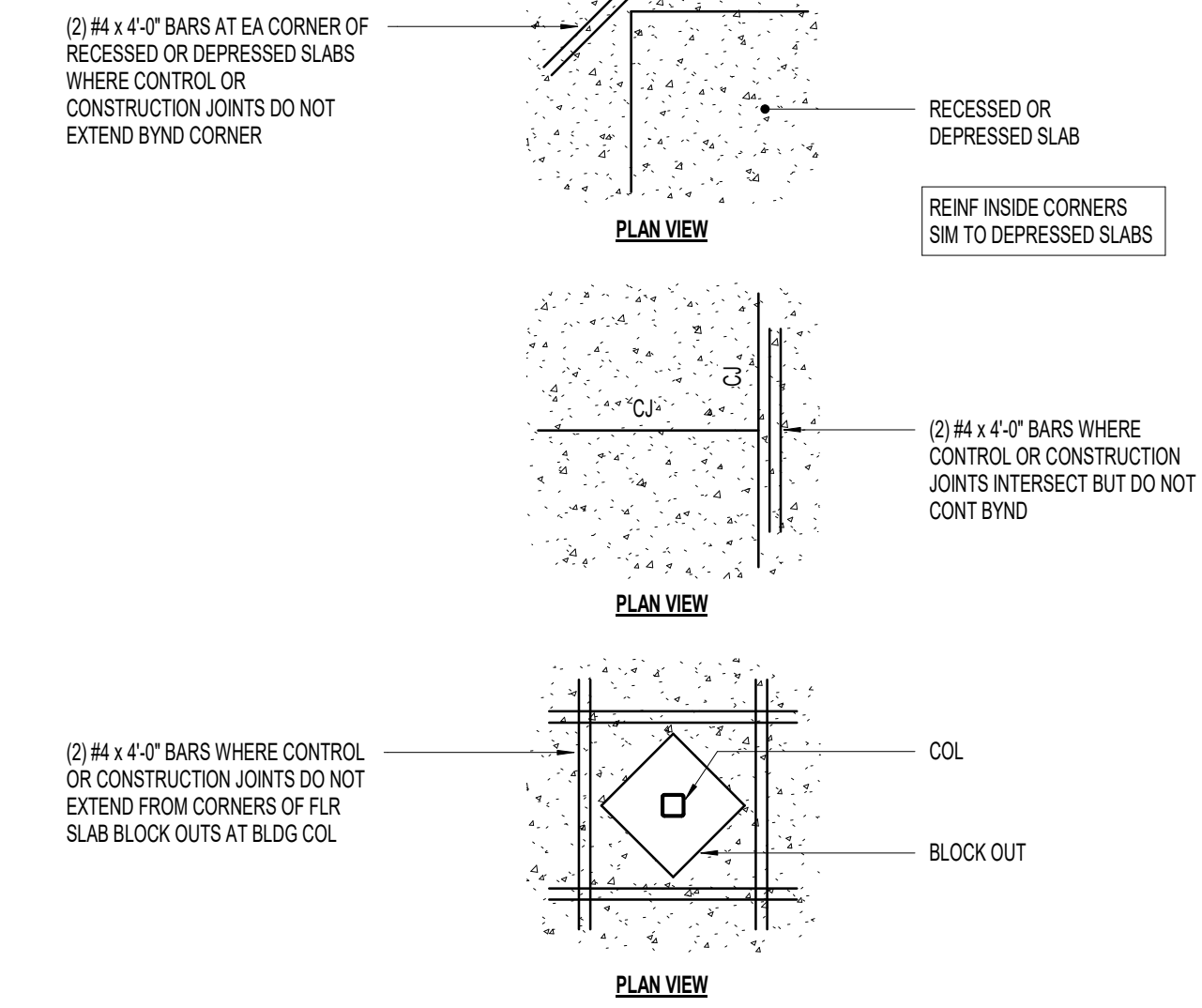
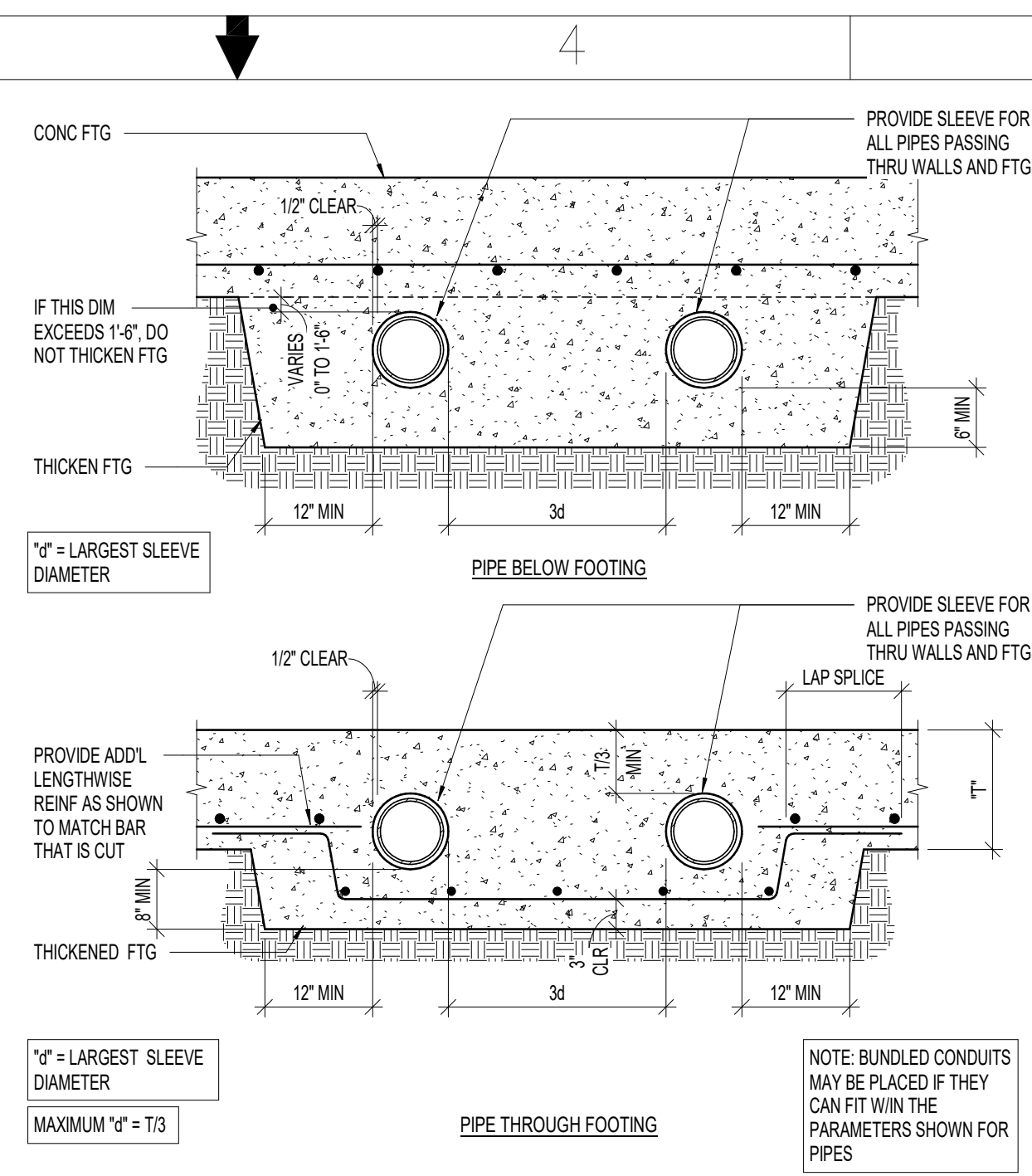
Project Name
ALBANY MEZZANINE

Sheet Title

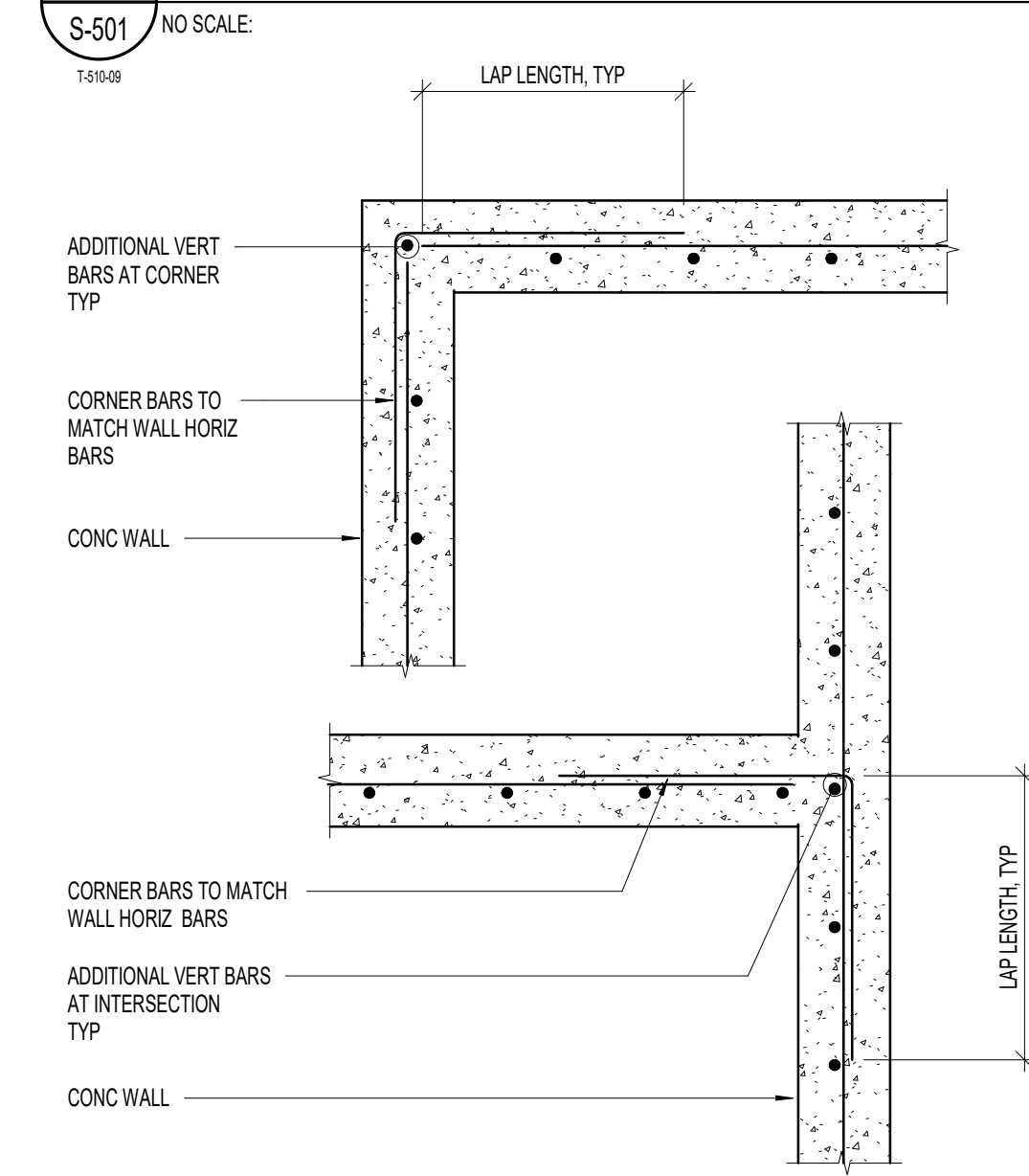
FOOTING AND FOUNDATION DETAILS

Scale	Date 2023.03.28
Drawn JDD	Project No. 22159

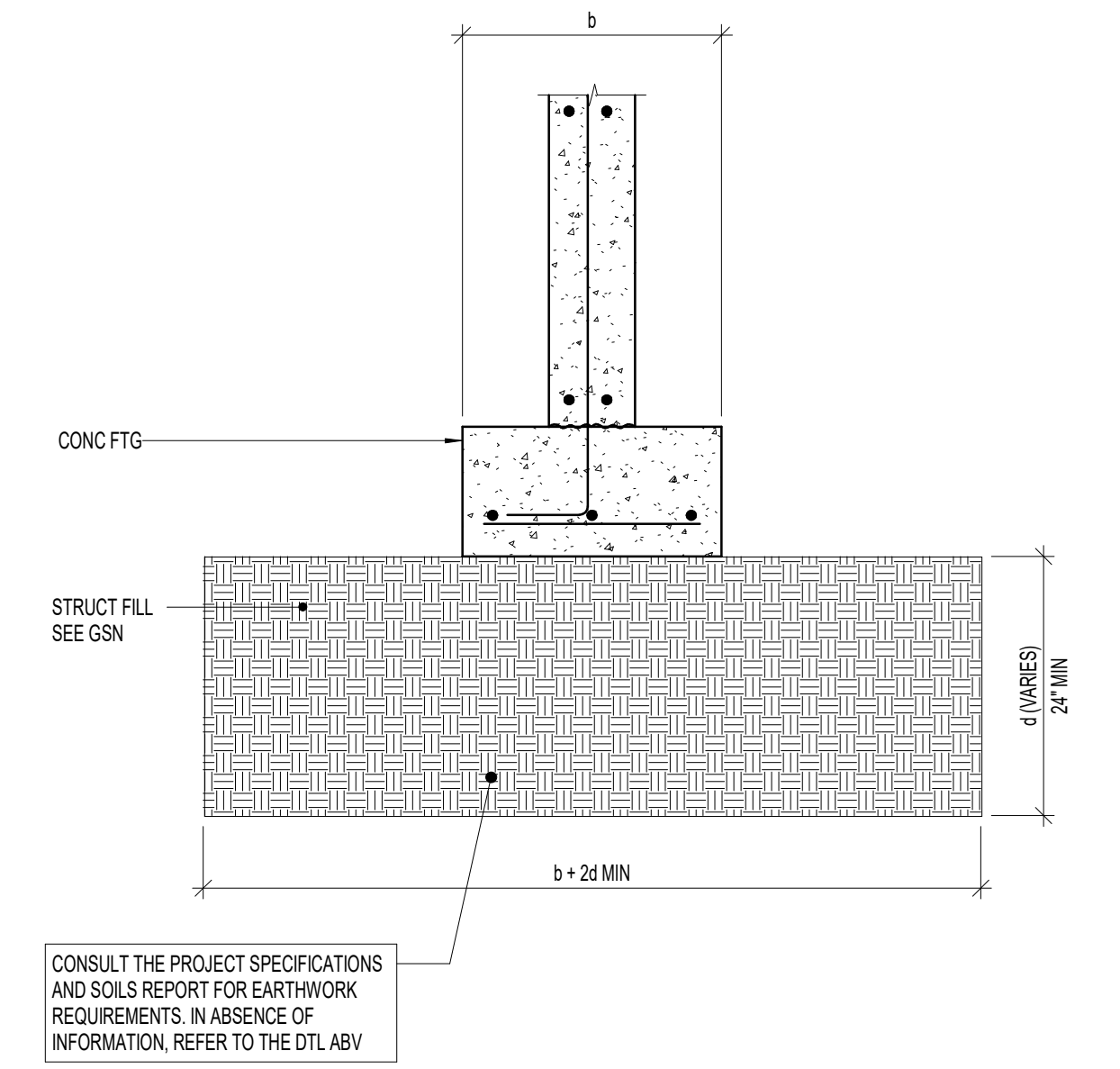
S-501



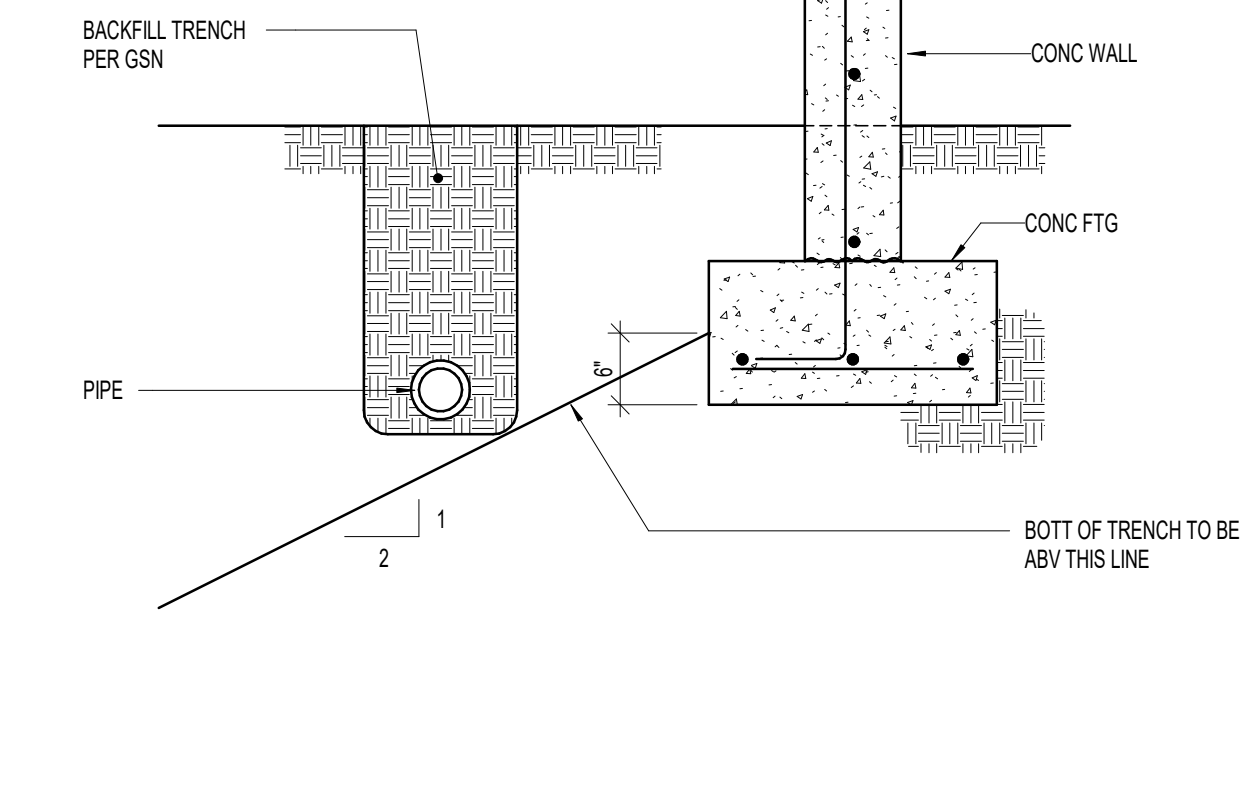
4 TYPICAL SLAB ON GRADE DISCONTINUITIES REQUIRING ADDITIONAL REINFORCING



7 TYPICAL CORNER BARS FOR SINGLE REINFORCED CONCRETE WALLS (PLAN VIEW)

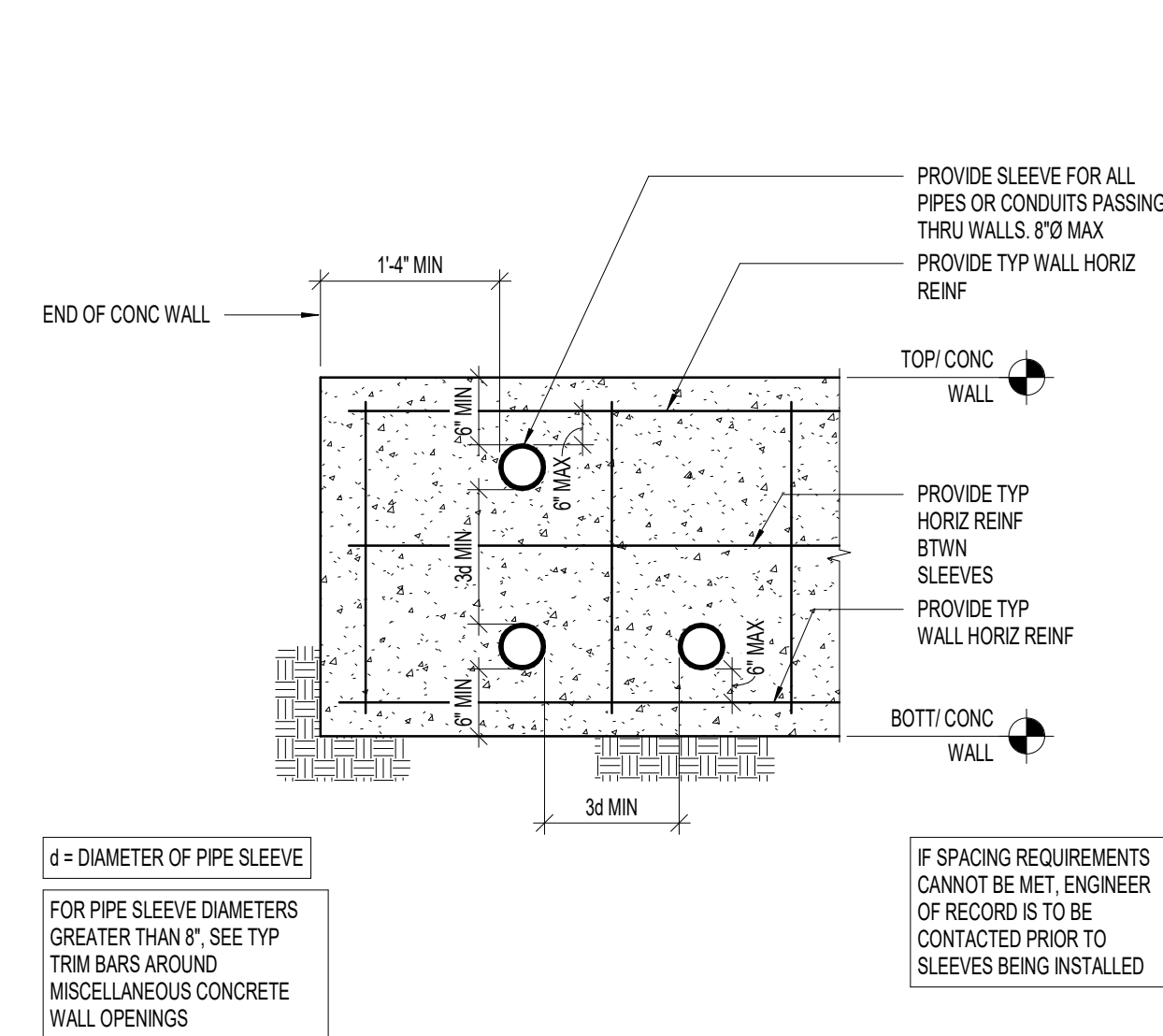


1 TYPICAL COMPACTED STRUCTURAL FILL

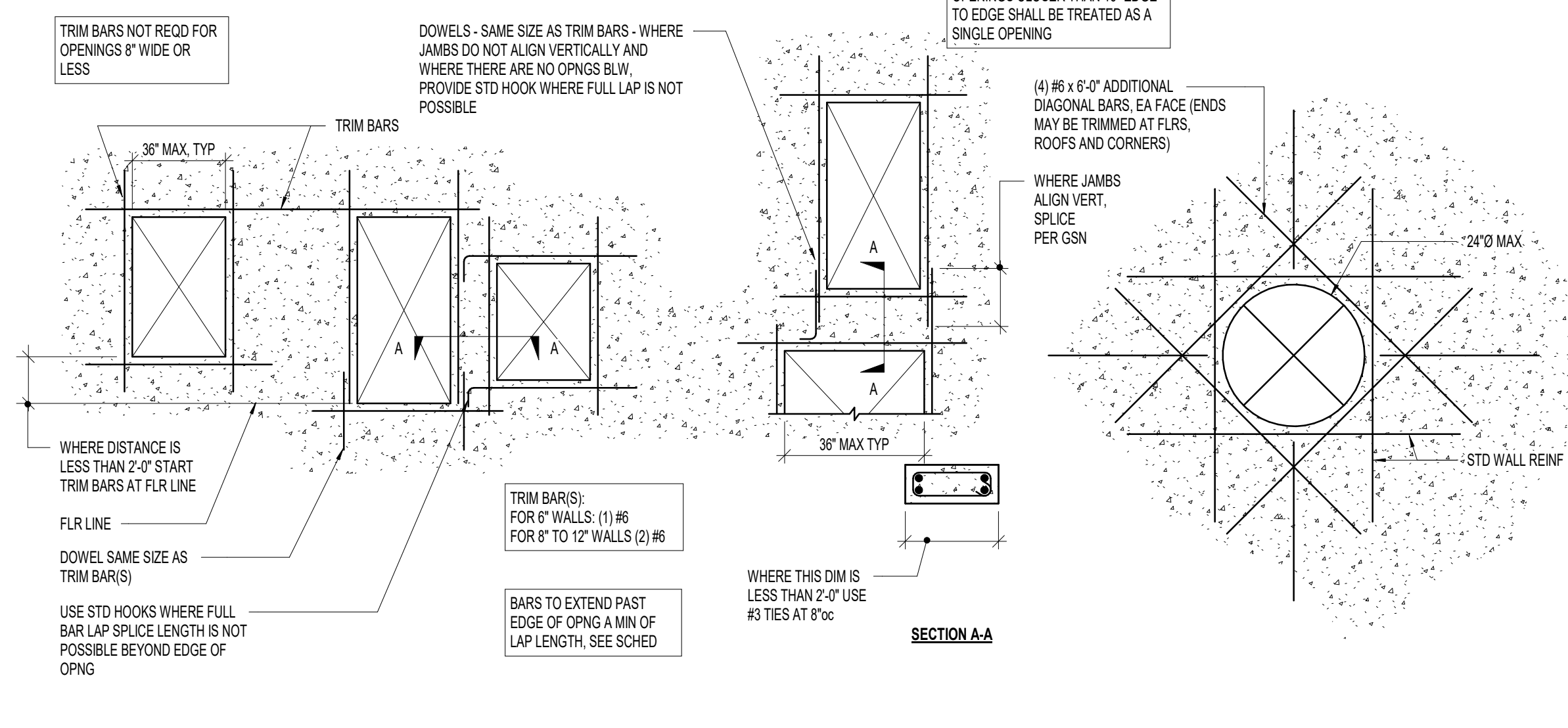


2 TYPICAL PIPE PARALLEL TO FOOTING

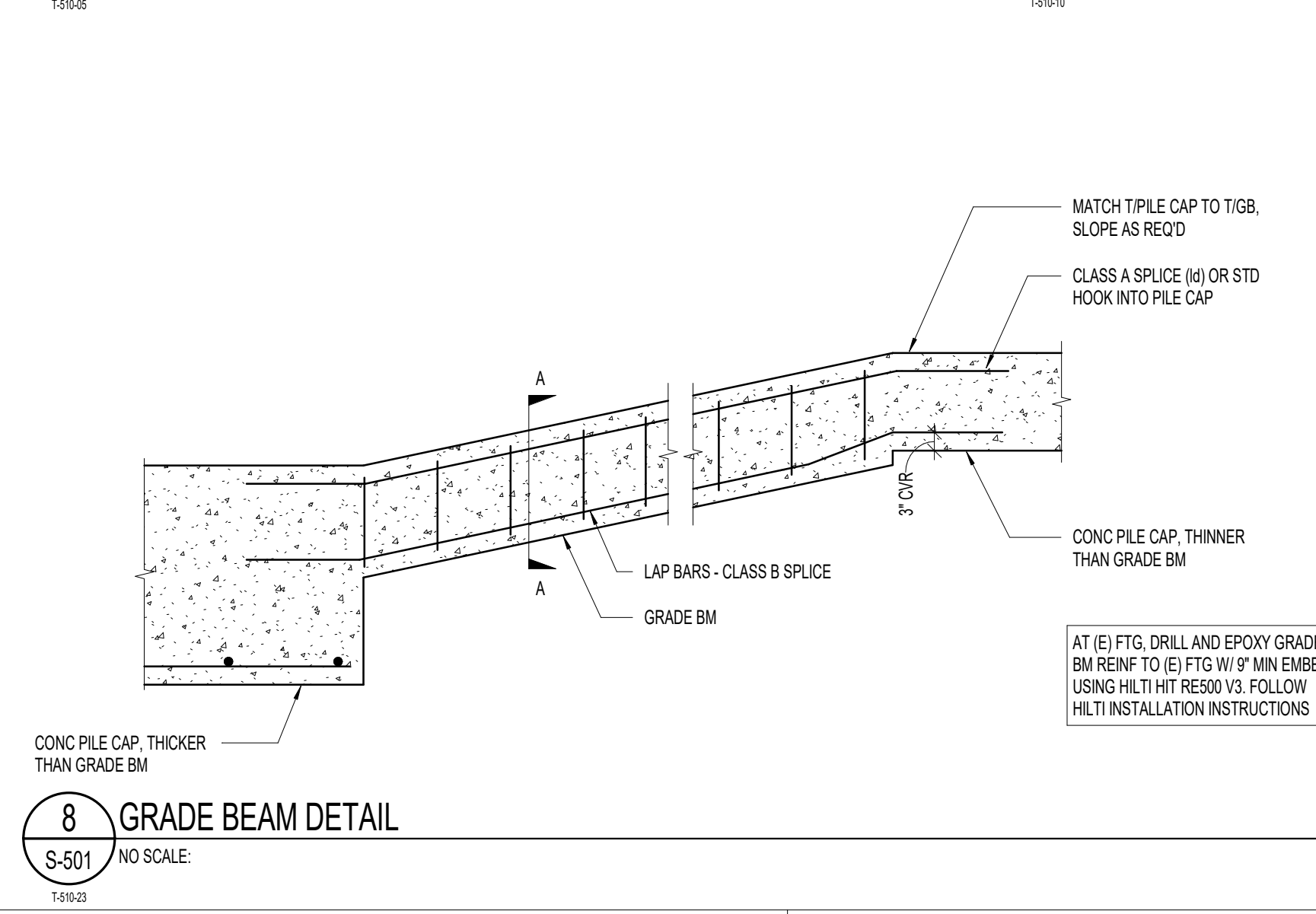
3 TYPICAL PIPE PERPENDICULAR TO FOOTING



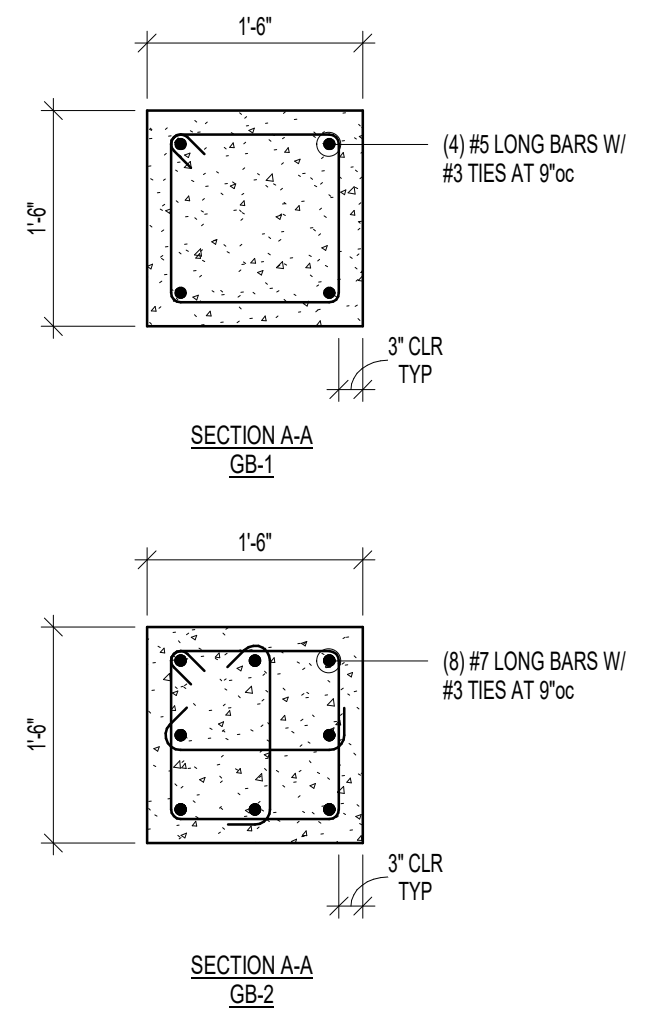
5 TYPICAL SMALL PIPE OR CONDUIT THROUGH CONCRETE WALL



6 TYPICAL TRIM BARS AROUND MISCELLANEOUS CONCRETE WALL OPENINGS UNLESS NOTED OTHERWISE



8 GRADE BEAM DETAIL



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2023.03.28
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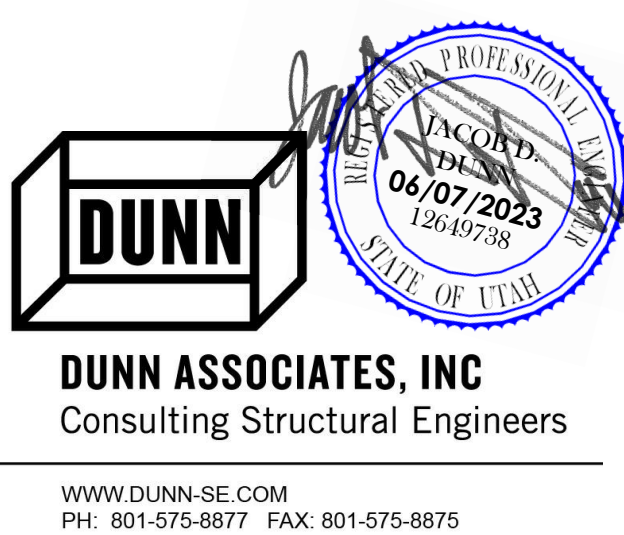
Project Name
ALBANY MEZZANINE

Sheet Title

FOOTING AND
FOUNDATION DETAILS

Scale	Date 2023.03.28
Drawn JDD	Project No. 22159

S-502



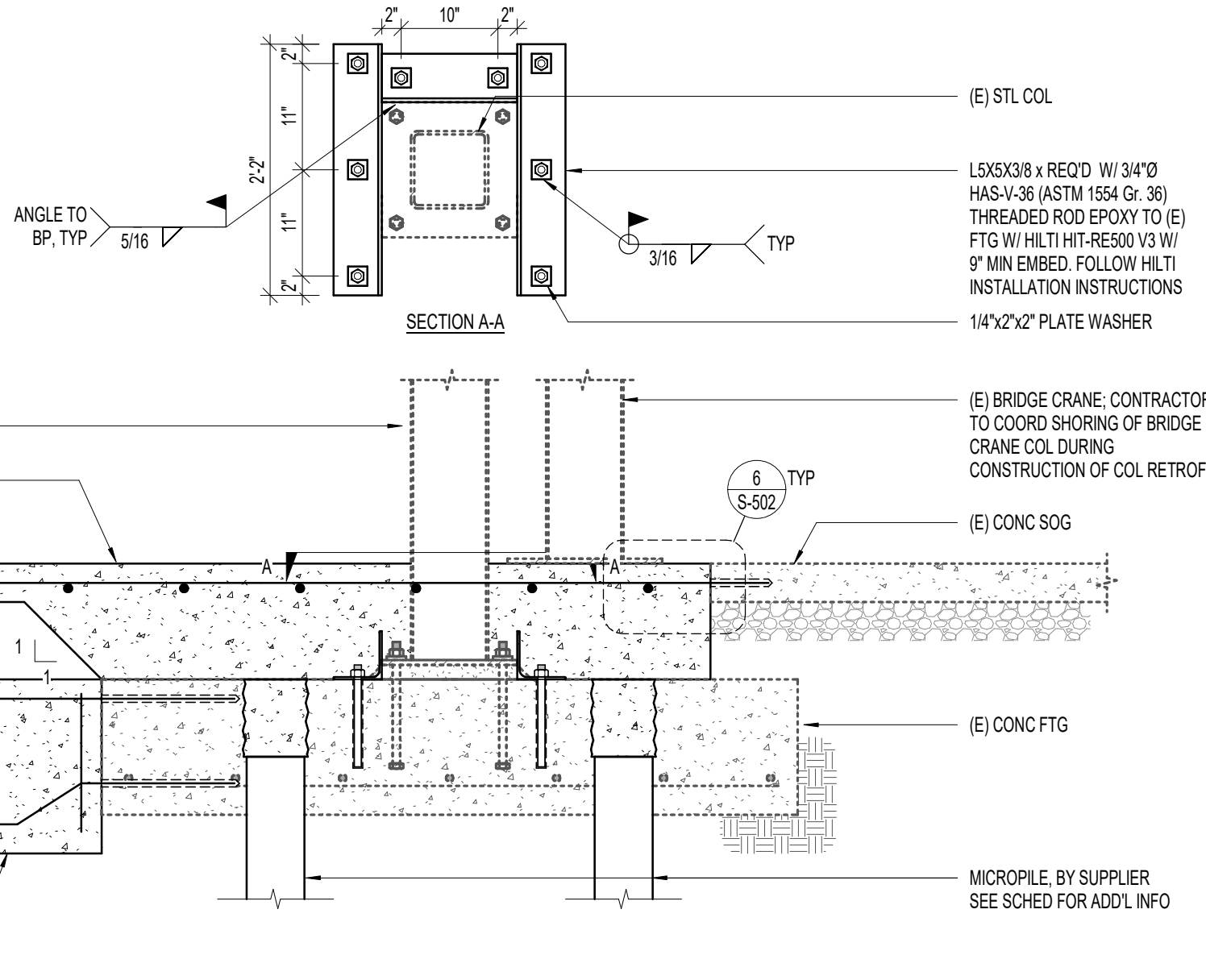
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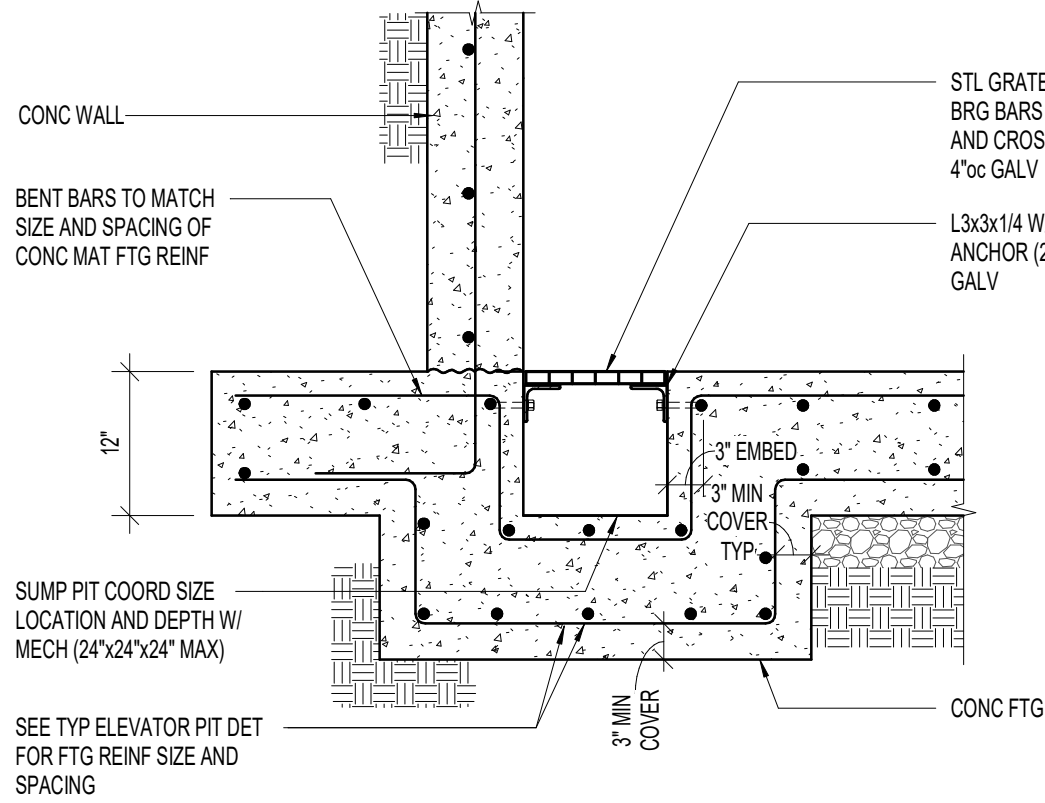
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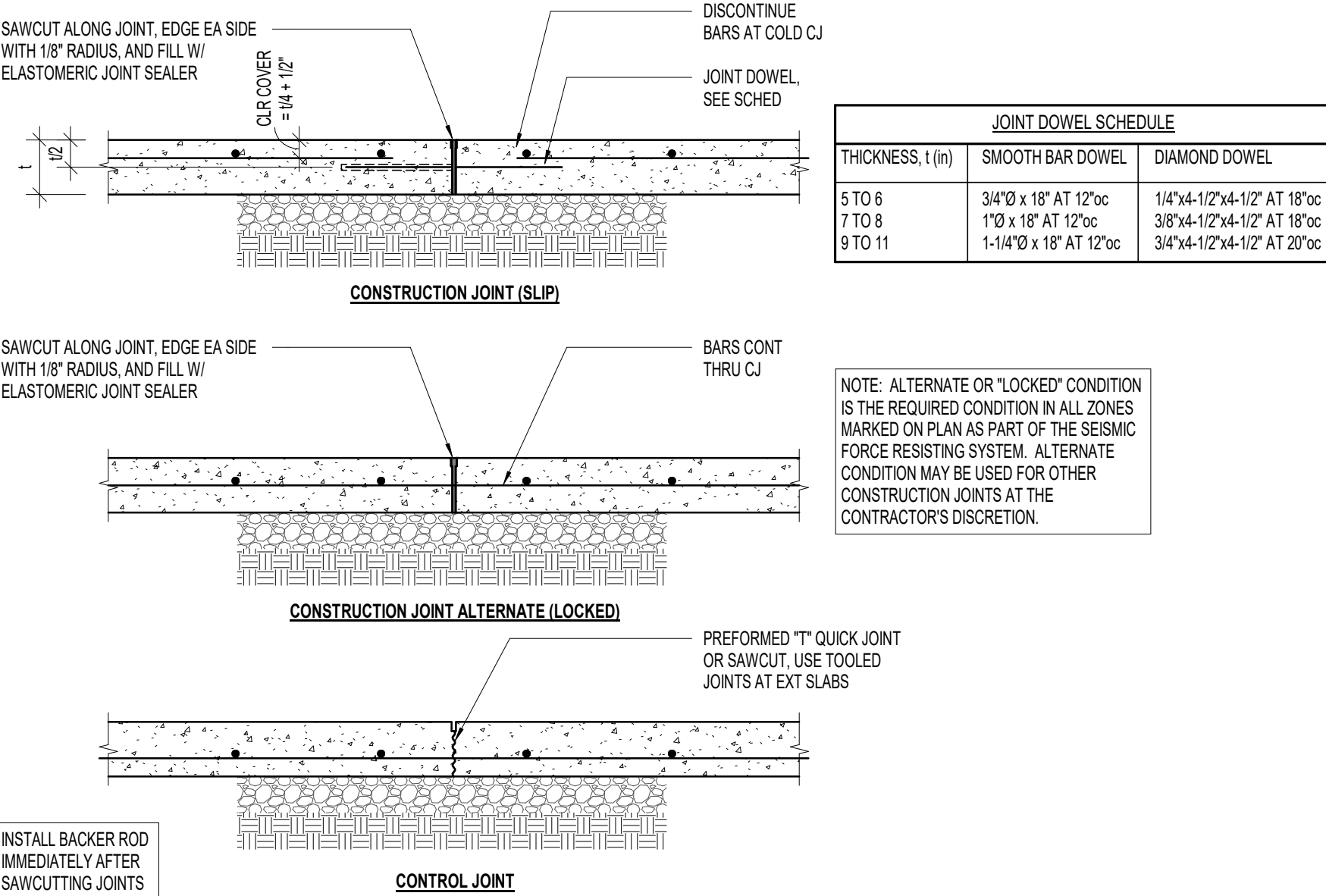
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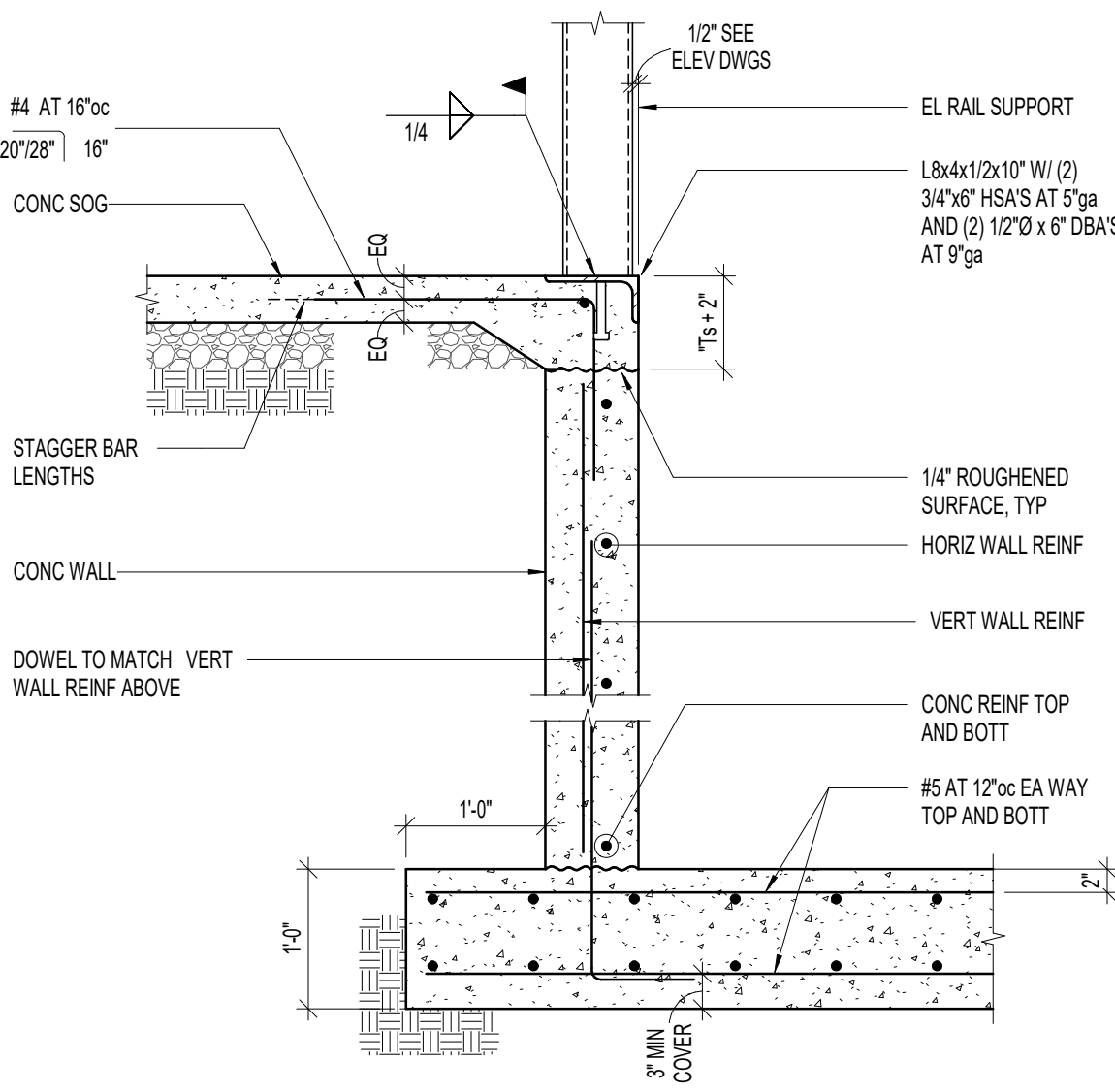
4 TYPICAL EXISTING TUBE STEEL COLUMN TO CONCRETE FOOTING
S-502 NO SCALE:
1-4/16/02



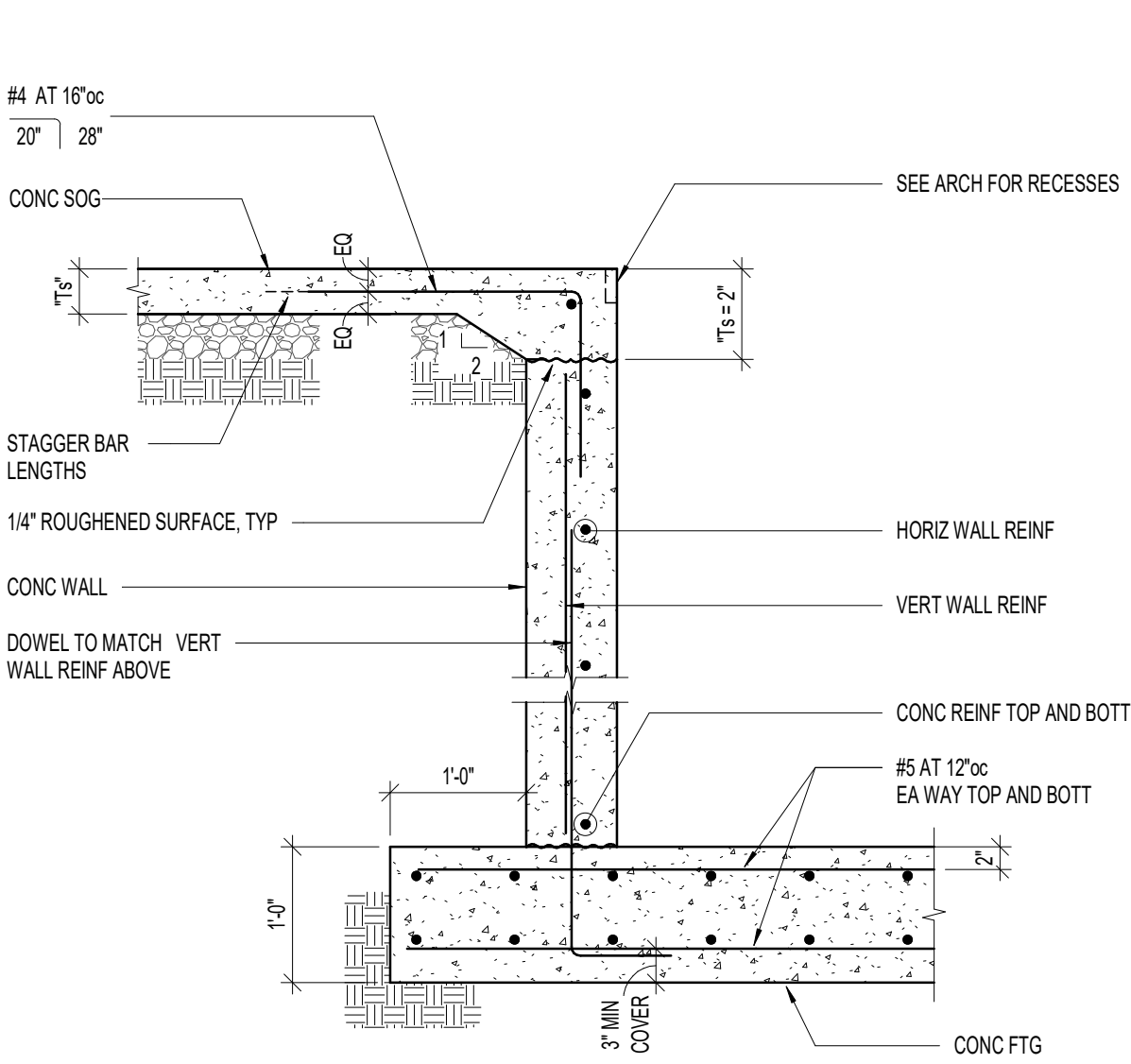
3 SUMP IN ELEVATOR PIT
S-502 NO SCALE:
1-4/16/04



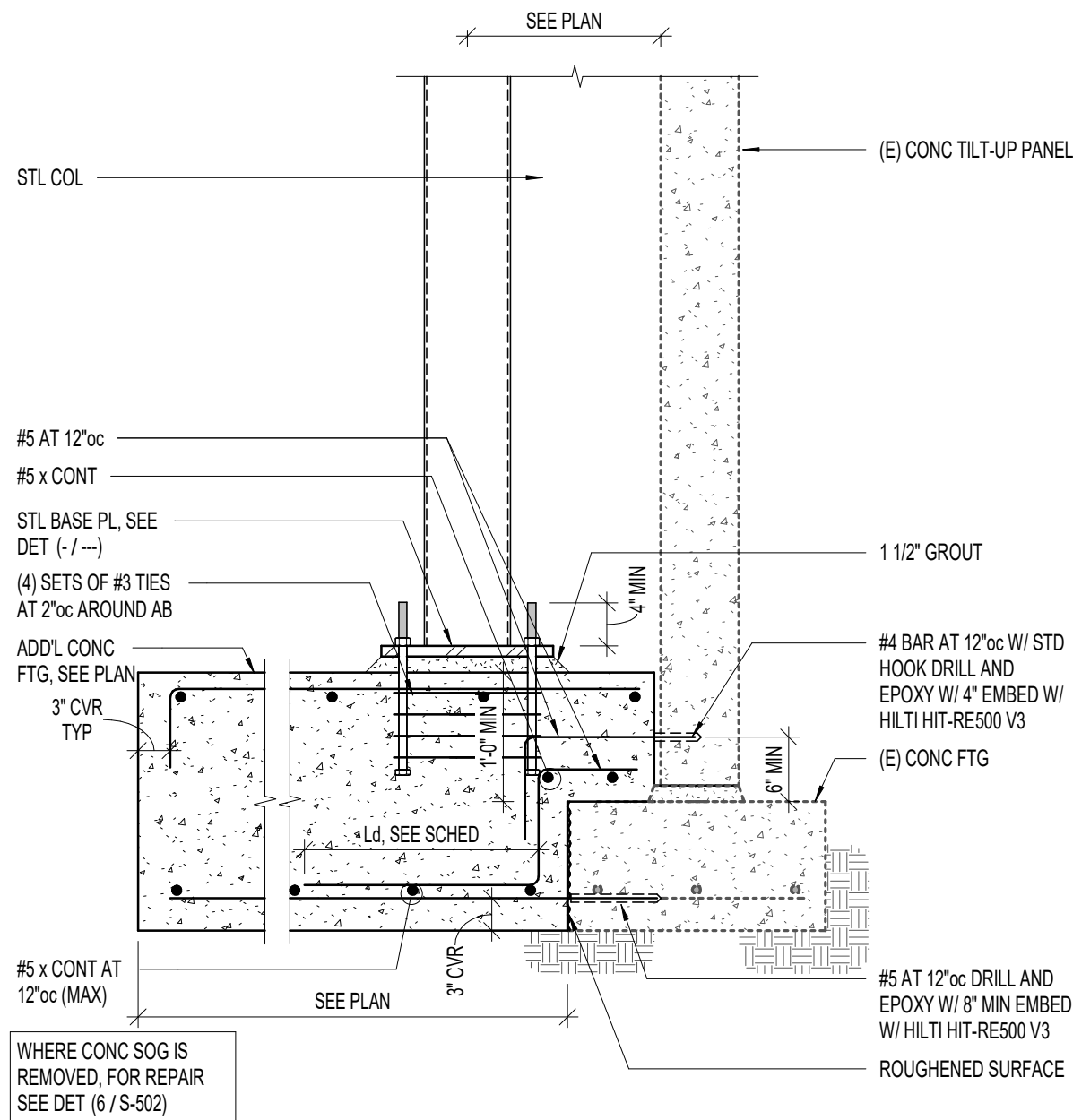
7 TYPICAL REINFORCED STRUCTURAL SLAB ON GRADE JOINTS
S-502 NO SCALE:
1-10/08



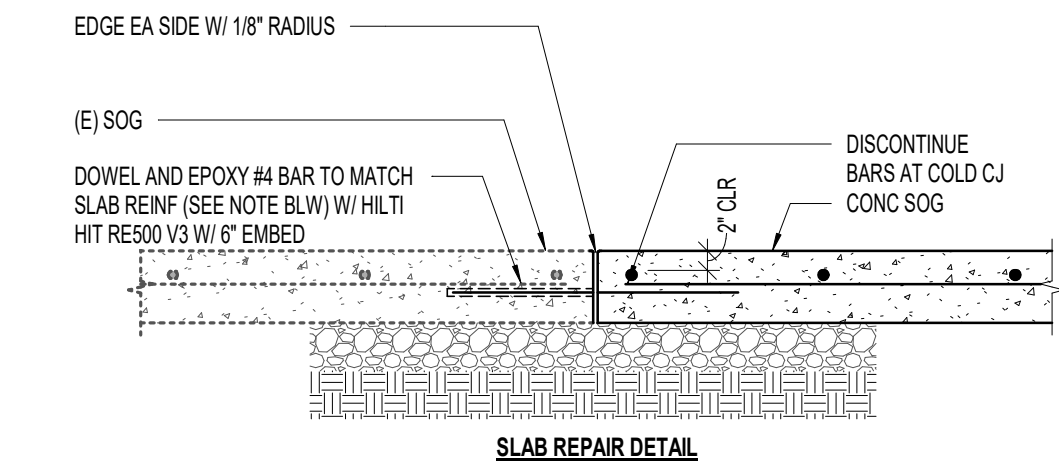
2 TYPICAL ELEVATOR PIT DETAIL
S-502 NO SCALE:
1-4/16/03



1 TYPICAL ELEVATOR PIT DETAIL
S-502 NO SCALE:
1-4/16/02



5 NEW TUBE STEEL COLUMN AT EXISTING WALL
S-502 NO SCALE:
1-10/08



6 TYPICAL SLAB ON GRADE REPAIR DETAIL
S-502 NO SCALE:
1-10/08

8 TYPICAL ELEVATOR PIT DETAIL AT CONCRETE GRADE BEAM
S-502 NO SCALE:
1-4/16/02

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No.	Description	Date

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Project Name
ALBANY MEZZANINE

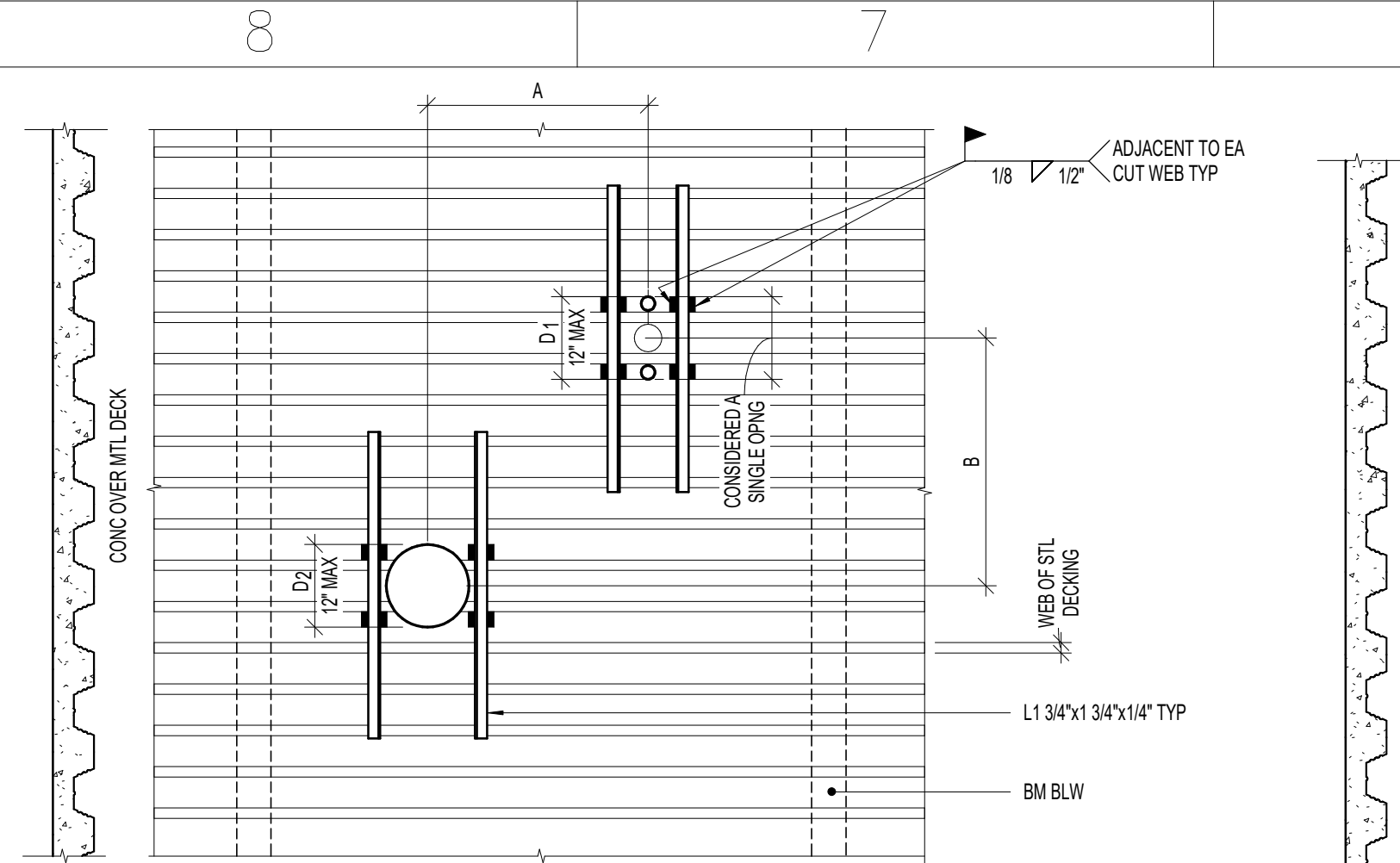
Sheet Title

FLOOR FRAMING
DETAILS

Scale	Date
JDD	2023.03.28
Project No.	22159

S-601

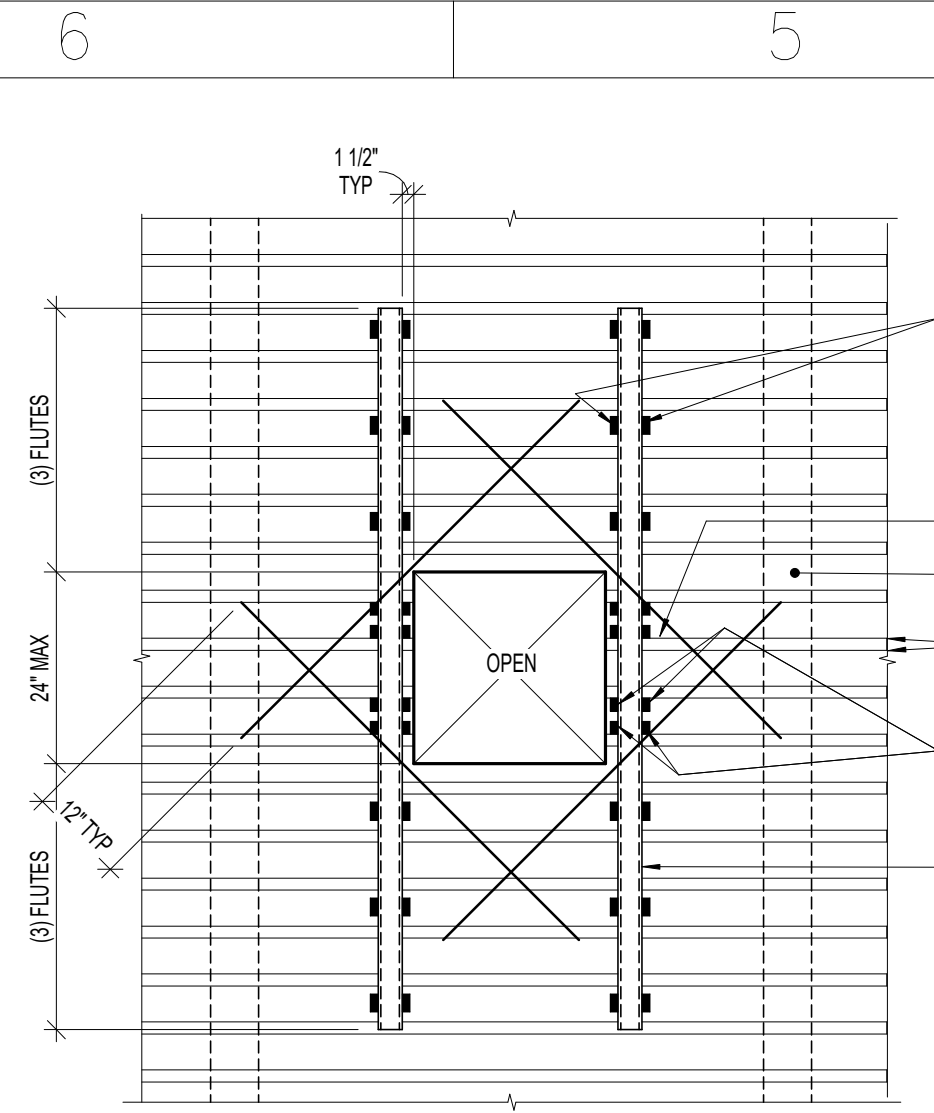
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- NOTES:
- THIS DETAIL APPLIES TO HOLES CUTTING NO MORE THAN (2) ADJACENT WEBS FOR 12" MODULE DECK.
 - OPENINGS SHALL BE BLOCKED OUT AND THE DECK LEFT INTACT. AFTER THE CONCRETE HAS CURED, THE DECK IN THE AREA OF THE OPENING CAN BE REMOVED.
 - ANGLES SHALL BE PLACED ON TOP OF THE DECK. ANGLES SHALL EXTEND 3 WEBS PAST THE DECK OPENING (TYP) IF DIMENSION A IS $>4D_1$, $4D_2$, OR 32" WHICHEVER IS LARGER, THERE IS NO RESTRICTION ON DIMENSION B IF DIMENSION B IS $>4D_1$, $4D_2$, OR 32" WHICHEVER IS LARGER, THERE IS NO RESTRICTION ON DIMENSION A IF DIMENSIONS A AND B ARE $<4D_1$, $4D_2$, OR 32" WHICHEVER IS LARGER, THE OPENING GROUP SHALL BE CONSIDERED AS A SINGLE HOLE, AND SHALL BE REINFORCED AS REQUIRED FOR LARGER OPENING. SEE TYPICAL MEDIUM OPENING IN FLOOR SLAB DETAIL.
 - INDIVIDUAL HOLES LESS THAN 6" IN DIAMETER AND CUTTING NO MORE THAN ONE WEB DO NOT REQUIRE REINFORCING.

1 TYPICAL SMALL OPENING IN FLOOR SLAB

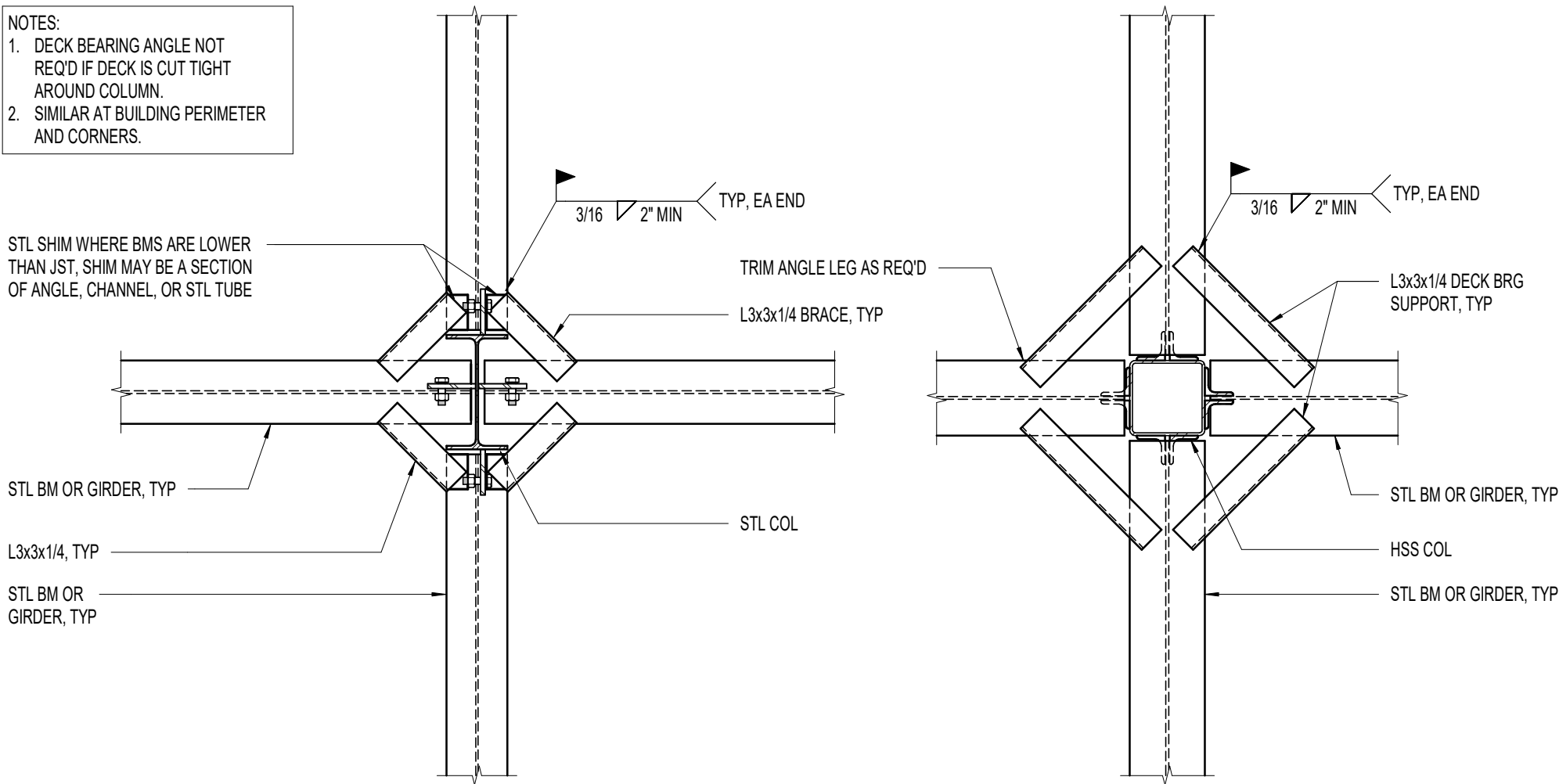
S-601 NO SCALE:
1:600-61



- NOTES:
- THIS DETAIL APPLIES TO OPENING UP TO 24".
 - TUBES SHALL BE PLACED ON TOP OF DECK.
 - THE DECK SHALL NOT BE CUT UNTIL CONCRETE HAS BEEN PLACED AND CURED. AT THE TIME OF POURING, SUITABLE SLEEVES OR BULKHEADS SHALL BE PLACED AROUND THE OPENING.
 - SEE FOLLOWING DETAIL IF OPENING IS GREATER THAN 24".

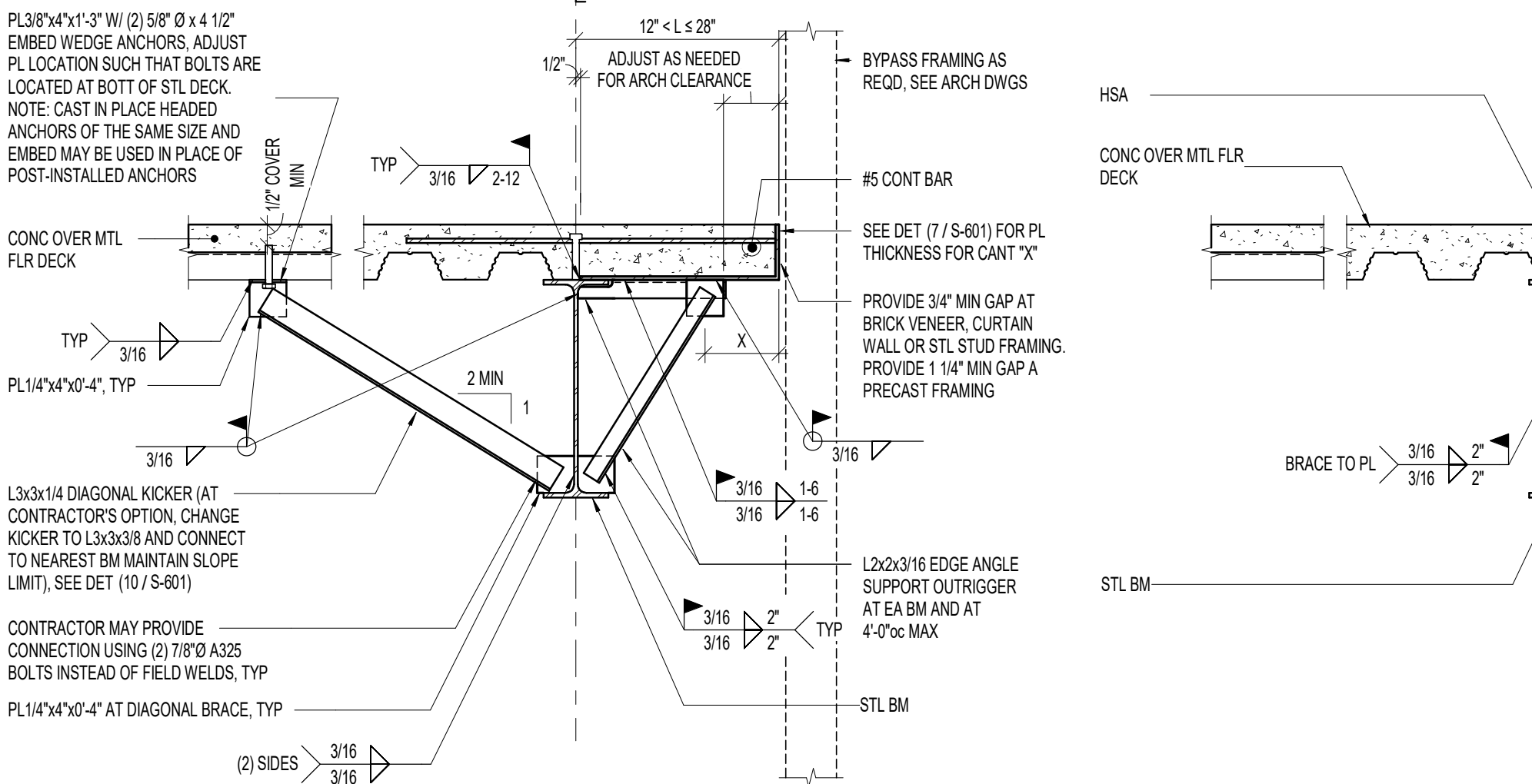
2 TYPICAL MEDIUM OPENING IN FLOOR SLAB

S-601 NO SCALE:
1:600-62



5 TYPICAL DECK BEARING ANGLES AT COLUMNS (WHERE REQUIRED)

S-601 NO SCALE:
1:600-64



9 TYPICAL CLOSURE PLATE (OVERHANG 12" < L ≤ 28")

S-601 NO SCALE:
1:600-68

10 TYPICAL KICKER CONNECTION DETAIL AT STEEL BEAM

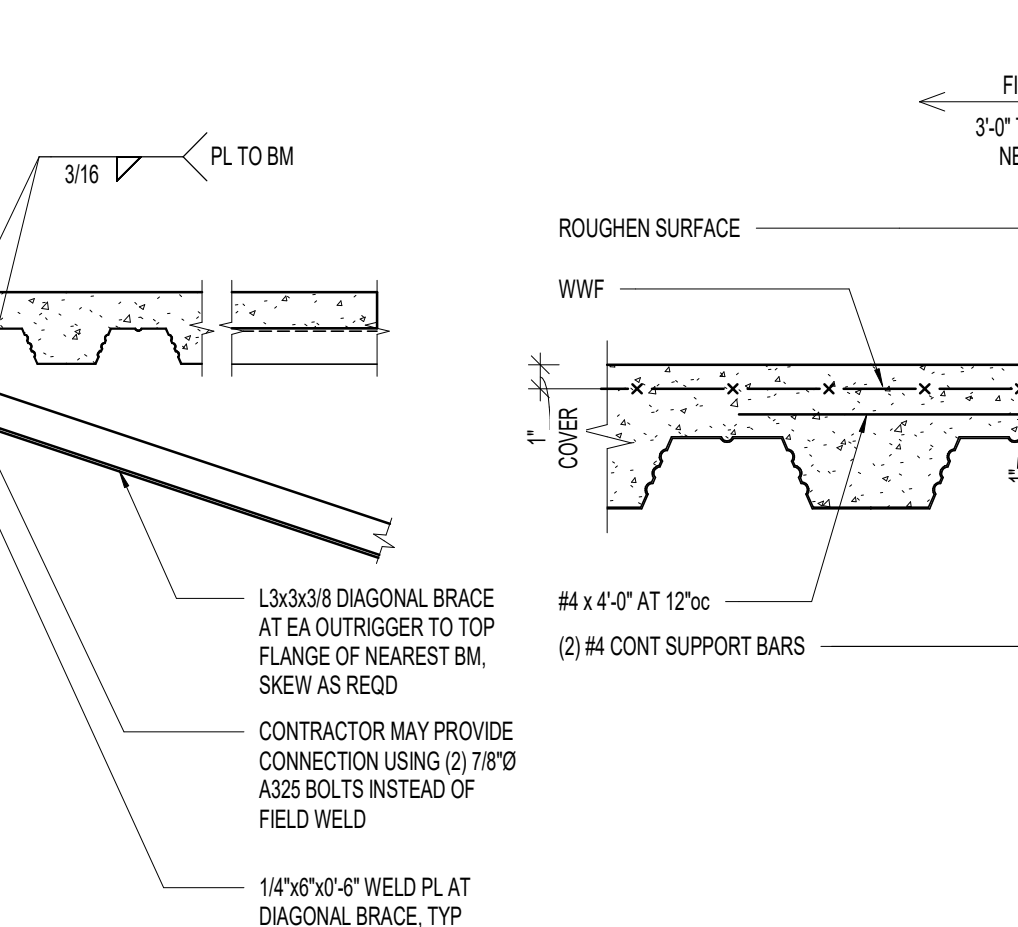
S-601 NO SCALE:
1:600-14

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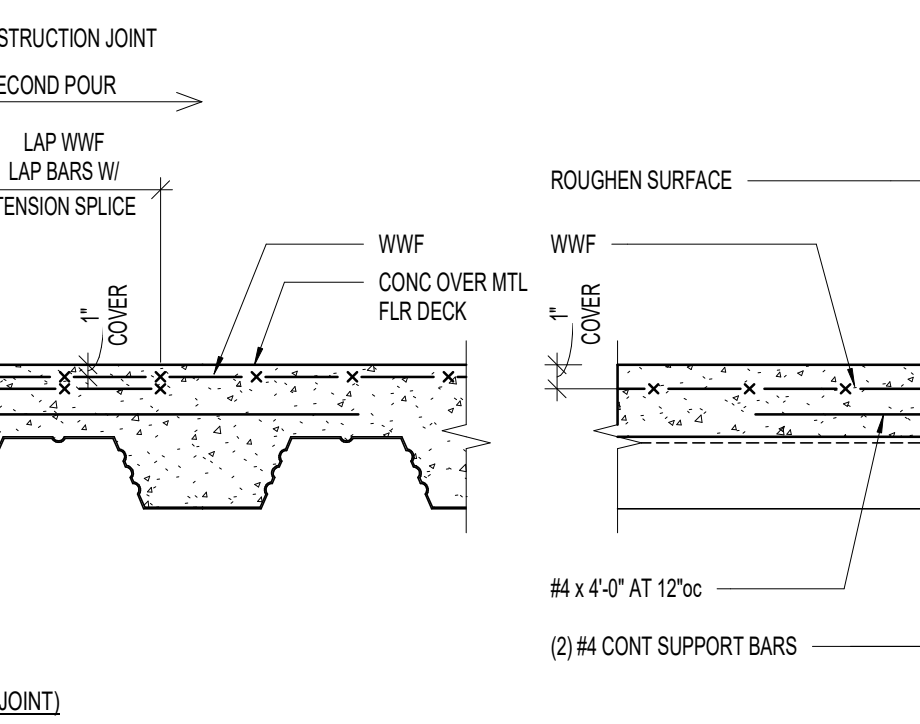
6 TYPICAL STEEL BEAM TO STEEL BEAM CONNECTION DETAILS

S-601 NO SCALE:
1:600-66



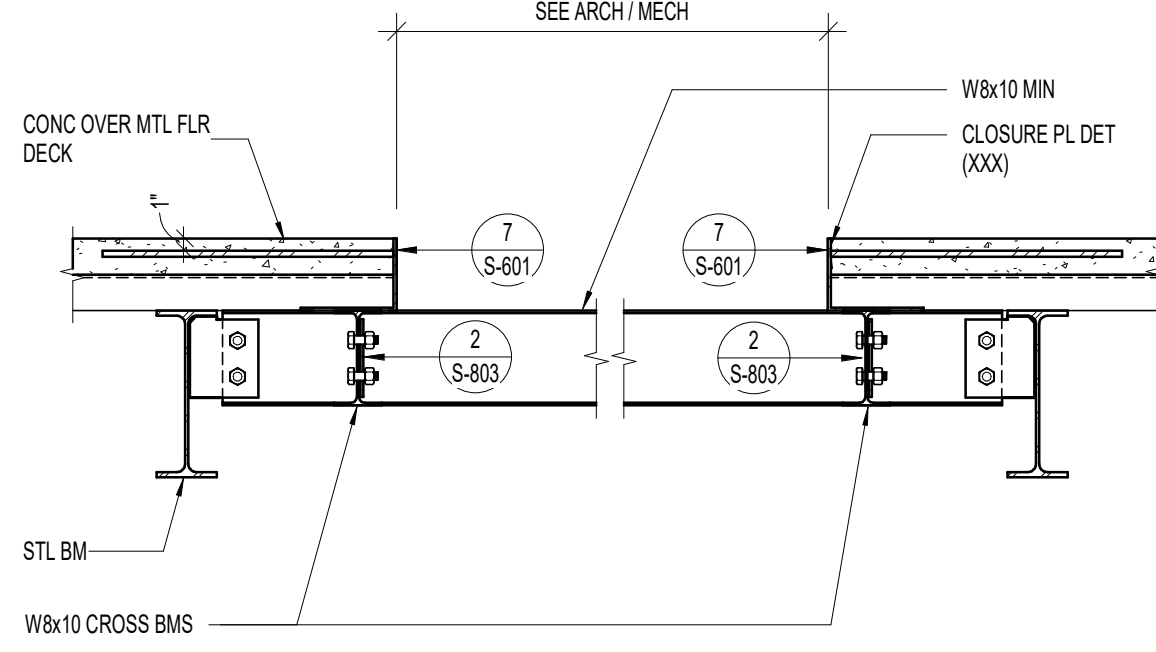
7 TYPICAL CLOSURE PLATE (OVERHANG L ≤ 12")

S-601 NO SCALE:
1:600-67



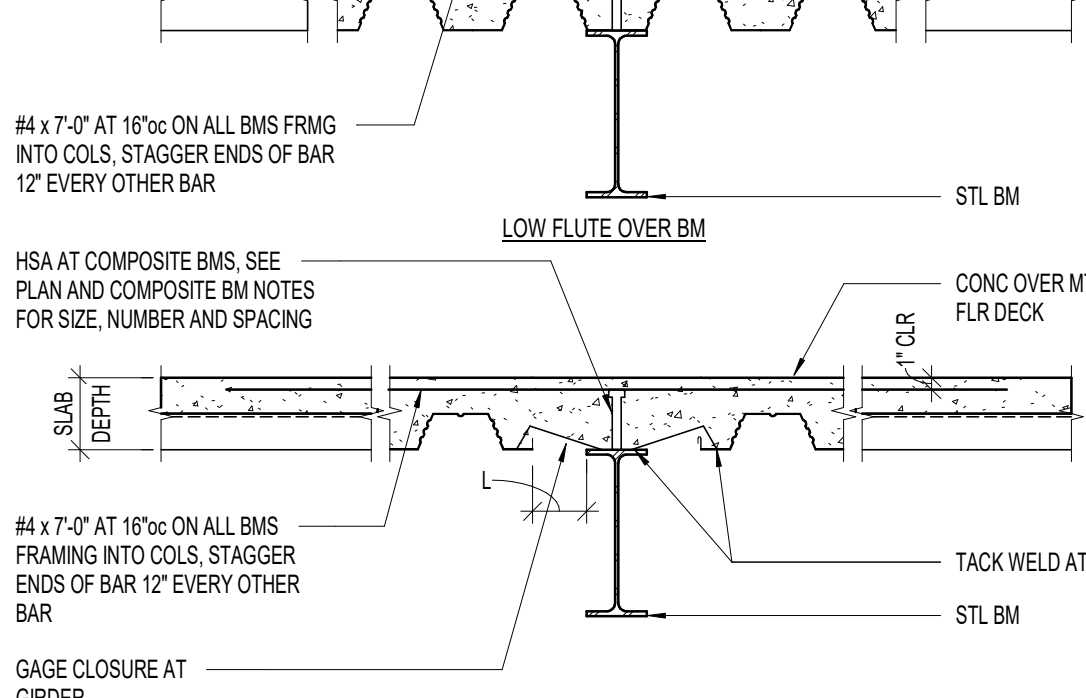
11 TYPICAL SLAB ON DECK CONTRUCTION JOINT DETAIL

S-601 NO SCALE:
1:600-62



3 MISCELLANEOUS FLOOR OPENING DETAIL

S-601 NO SCALE:
1:600-63

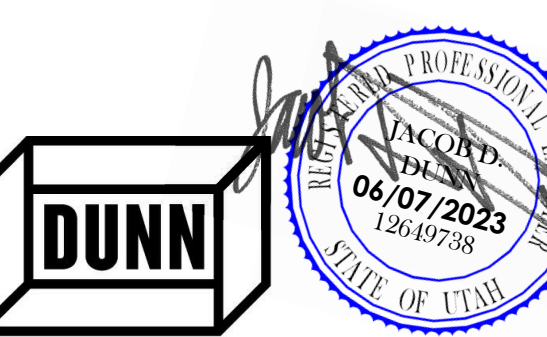


4 TYPICAL COMPOSITE BEAM DETAILS

S-601 NO SCALE:
1:600-66

RECOMMENDED GIRDER FILLER THICKNESS		
L*	METAL GAGE (TYPE)	
0" TO 5"	20	
5" TO 7 1/2"	16	
7 1/2" TO 9"	14	

* NOT TO EXCEED 12" ABV
GAGES ARE BASED ON
FLAT MATERIAL WITH NO
STRENGTH INCREASE
DUE TO ITS SHAPE.



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2023.03.28
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CONVENIENCE OF THE CONTRACTOR. VERIFY
DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL
DRAWINGS.



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Murray, Utah 84107

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1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

Issued/Revisions

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Project Name _____

Sheet Title

FLOOR FRAMING DETAILS

Scale	Date 2023.03.28
Drawn JDD	Project No. 22159

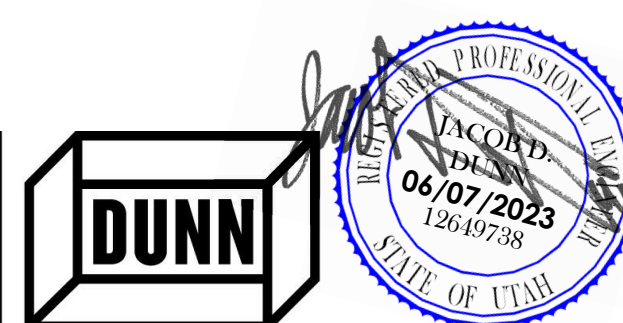
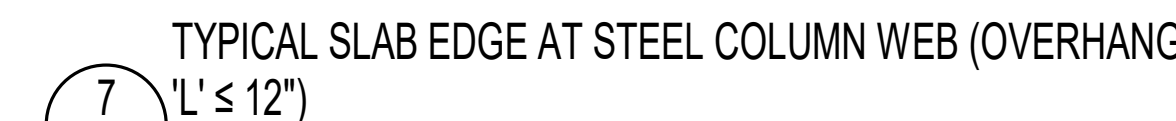
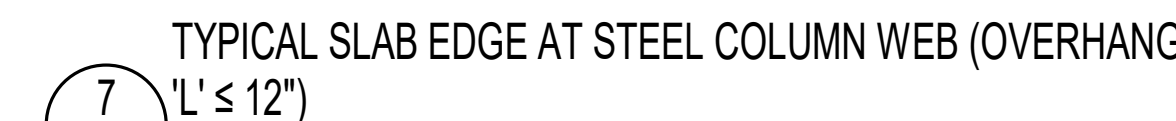
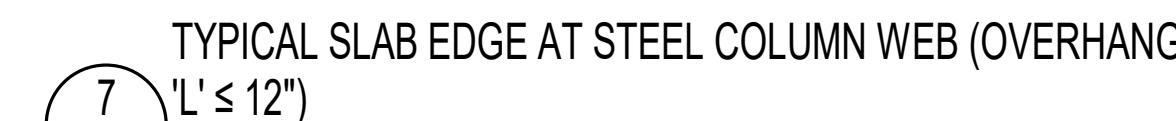
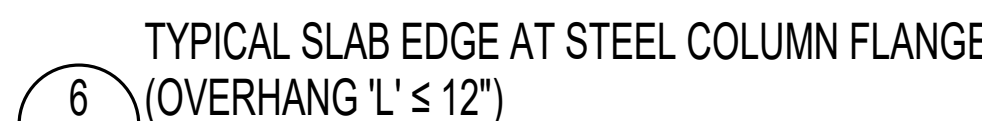
S-602



8 BEAM
S-602 NO SCALE

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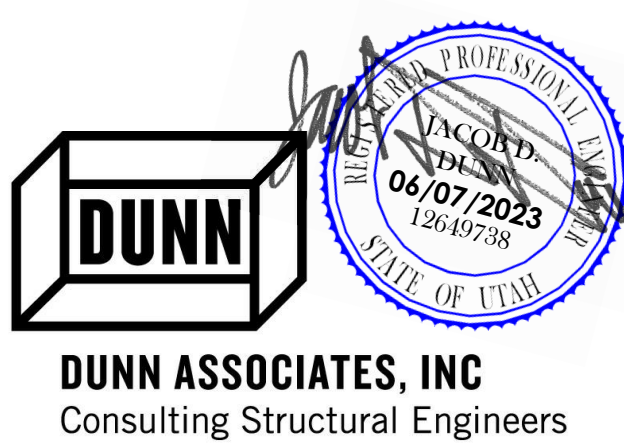
Project Name
ALBANY MEZZANINE

Sheet Title

ROOF FRAMING
DETAILS

Scale	Date 2023.03.28
Drawn Author	Project No. 22159

S-702



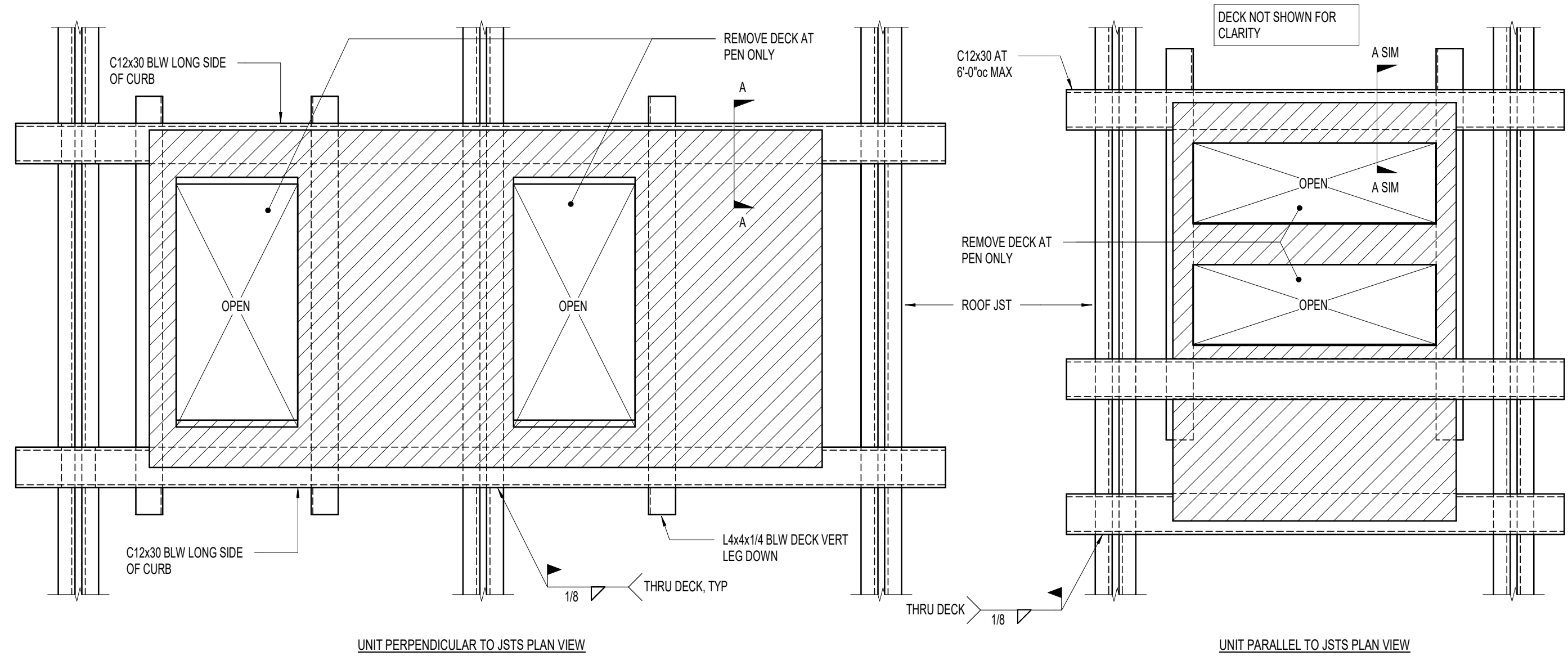
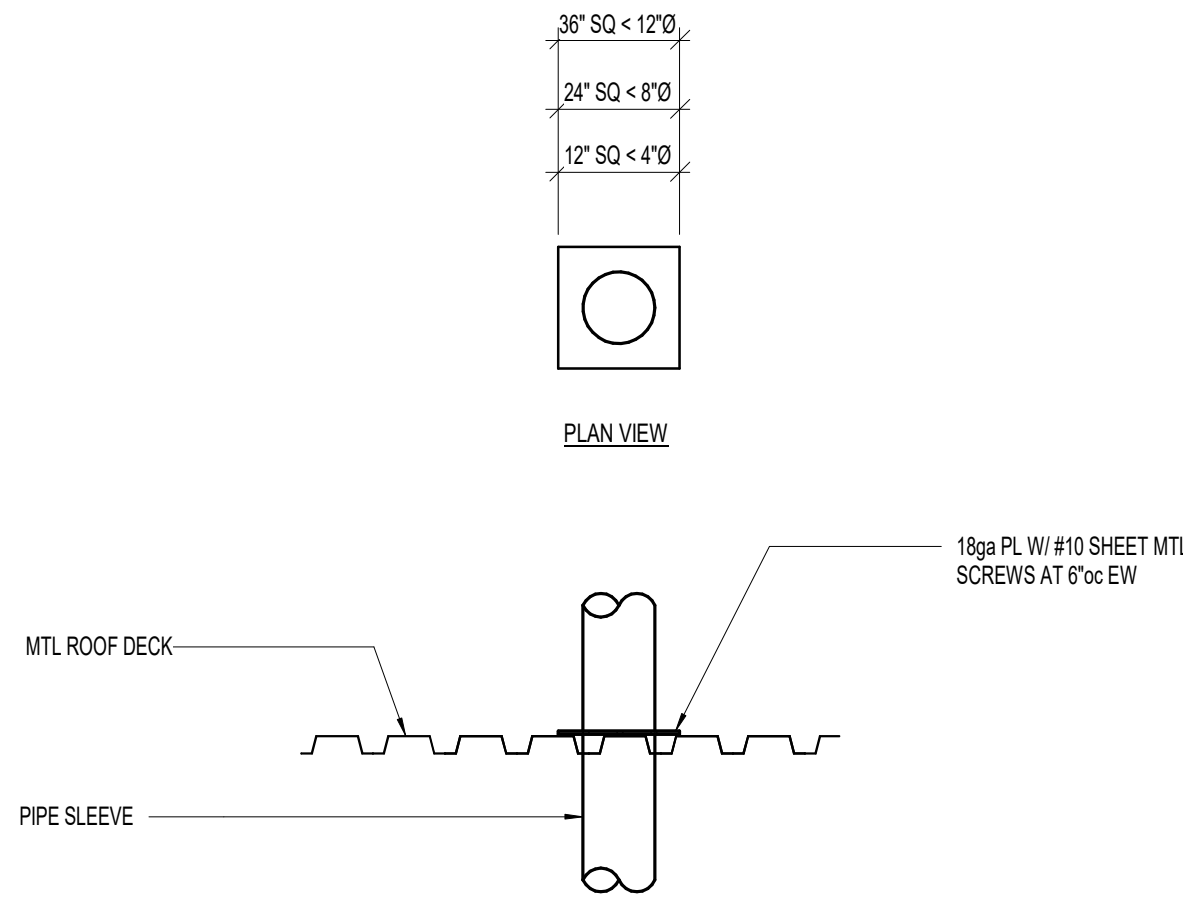
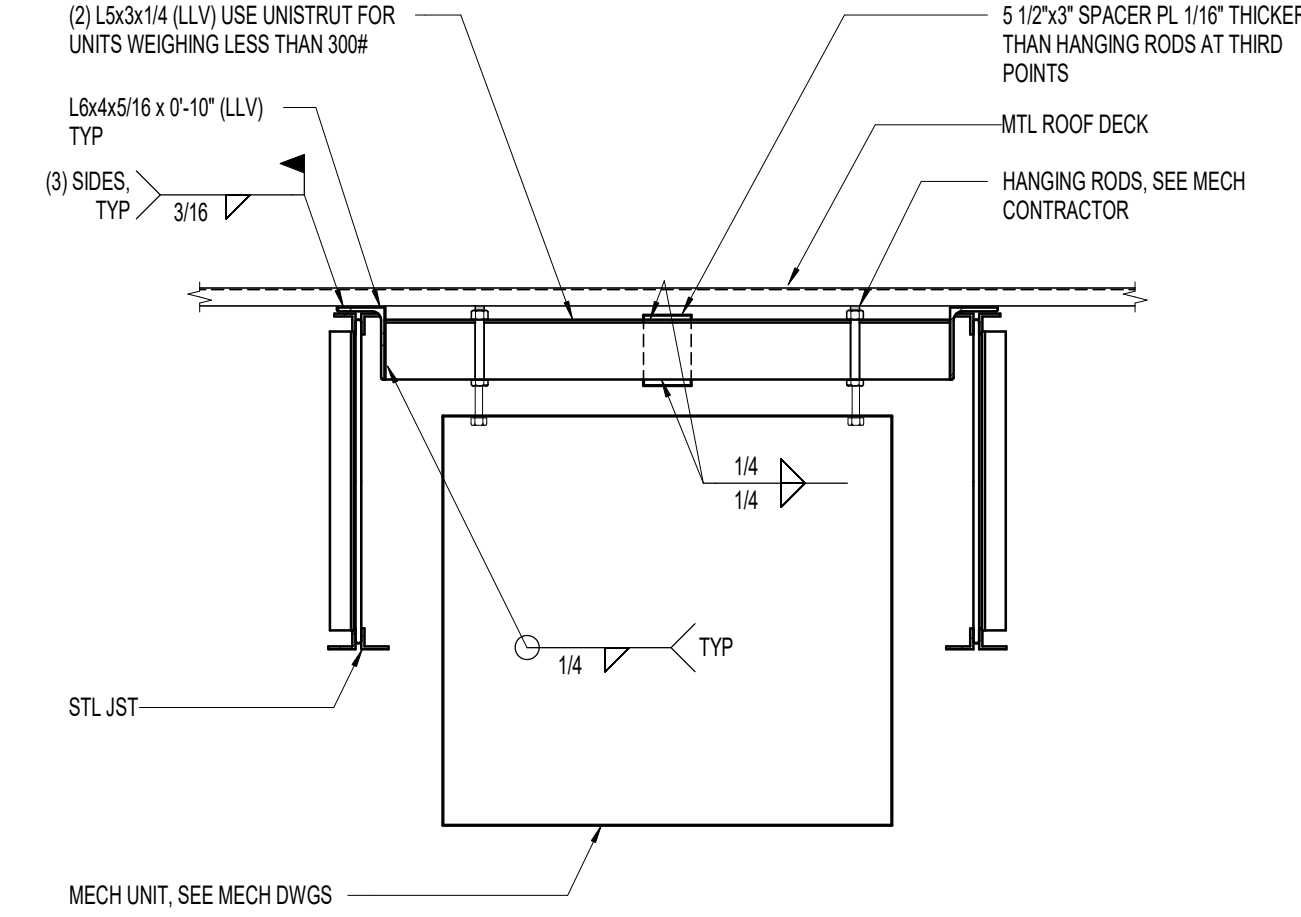
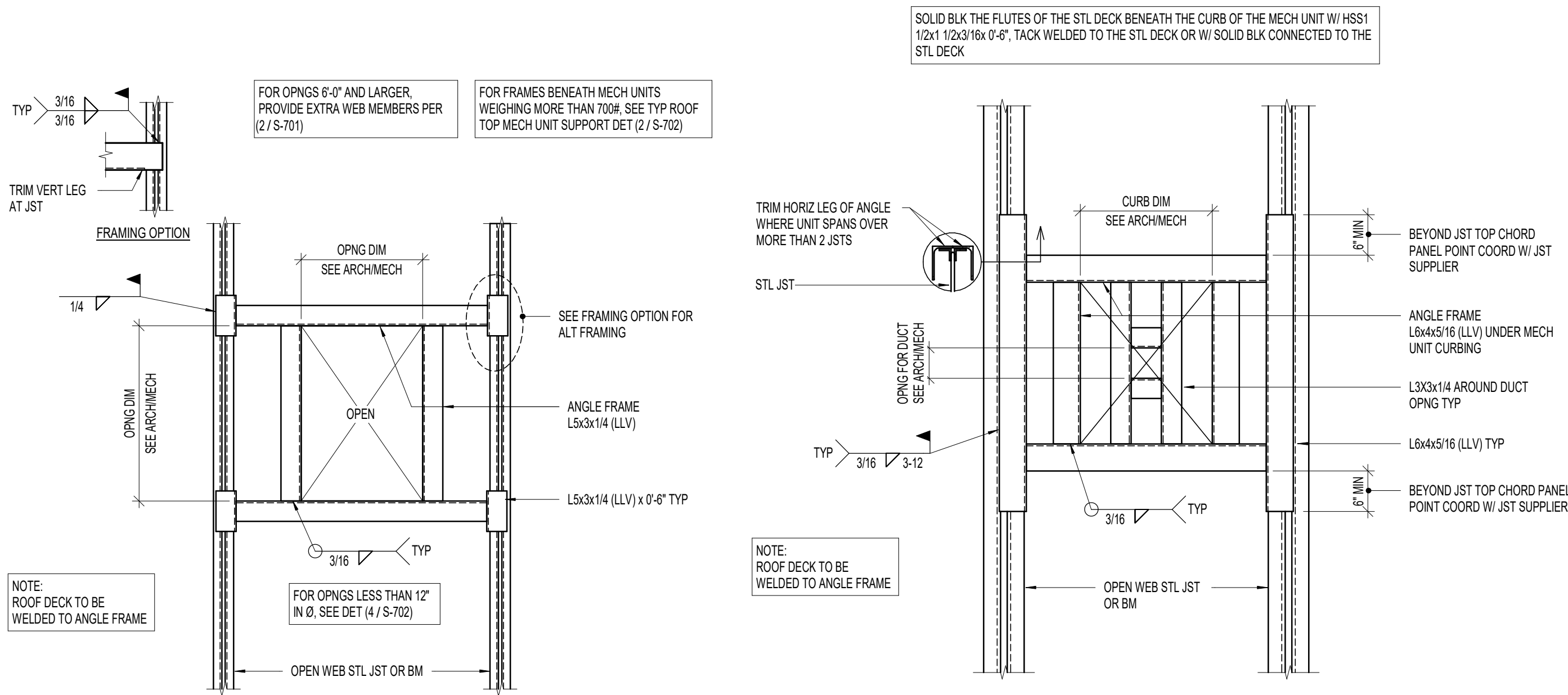
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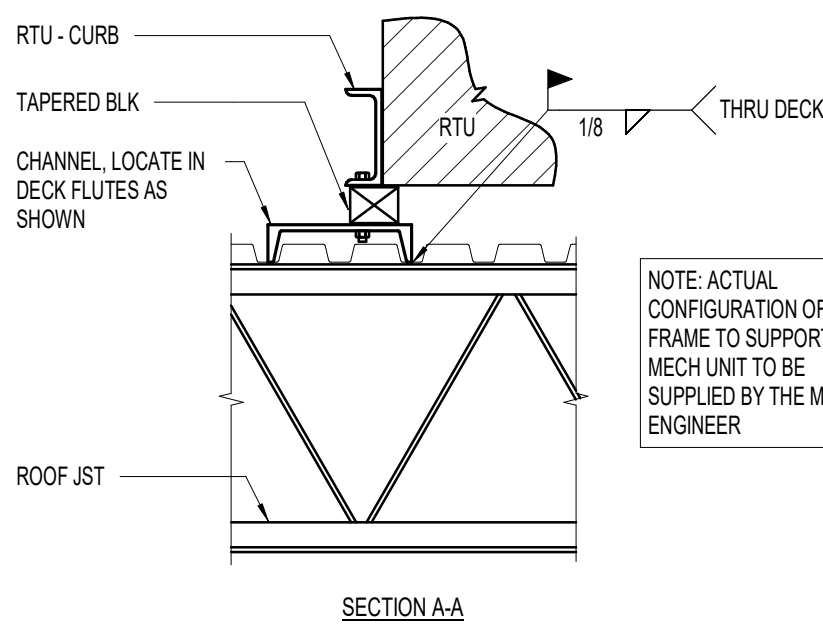
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- MECHANICAL UNIT SUPPORT NOTES**
1. INSTALL CHANNEL FROM TOP SIDE ABOVE DECK. LOCATE IN DECK FLUTES ADJUSTING SPACING IN 6" INCREMENTS TO SUPPORT LONG SIDE OF CURB.
 2. POSITION CURB OVER CHANNELS AND LOCATE REQUIRED DUCT PENETRATIONS THRU ROOF. REFER TO MECHANICAL PLANS FOR EXACT SIZES AND LOCATIONS.
 3. POSITION ANGLES BELOW DECK AND WELD TO CHANNELS THRU DECK FROM TOP SIDE. OMIT CROSS ANGLES IF EDGES OF PENETRATION IS WITHIN 6" OF A JOIST.
 4. CUT ROOF DECK ONLY AS REQUIRED FOR DUCT PENETRATIONS.



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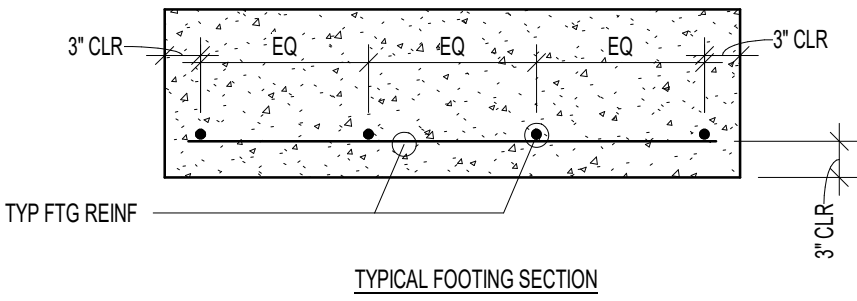
Sheet Title

SCHEDULES

Scale	Date 2023.03.28
Drawn JDD	Project No. 22159

S-801

CONCRETE FOOTING SCHEDULE												
MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				COMMENTS
				NO	SIZE	LENGTH	SPACING	NO	SIZE	LENGTH	SPACING	
FSS.0	5'-0"	5'-0"	12"	5	#5	4'-6"	EQ	5	#5	4'-6"	EQ	



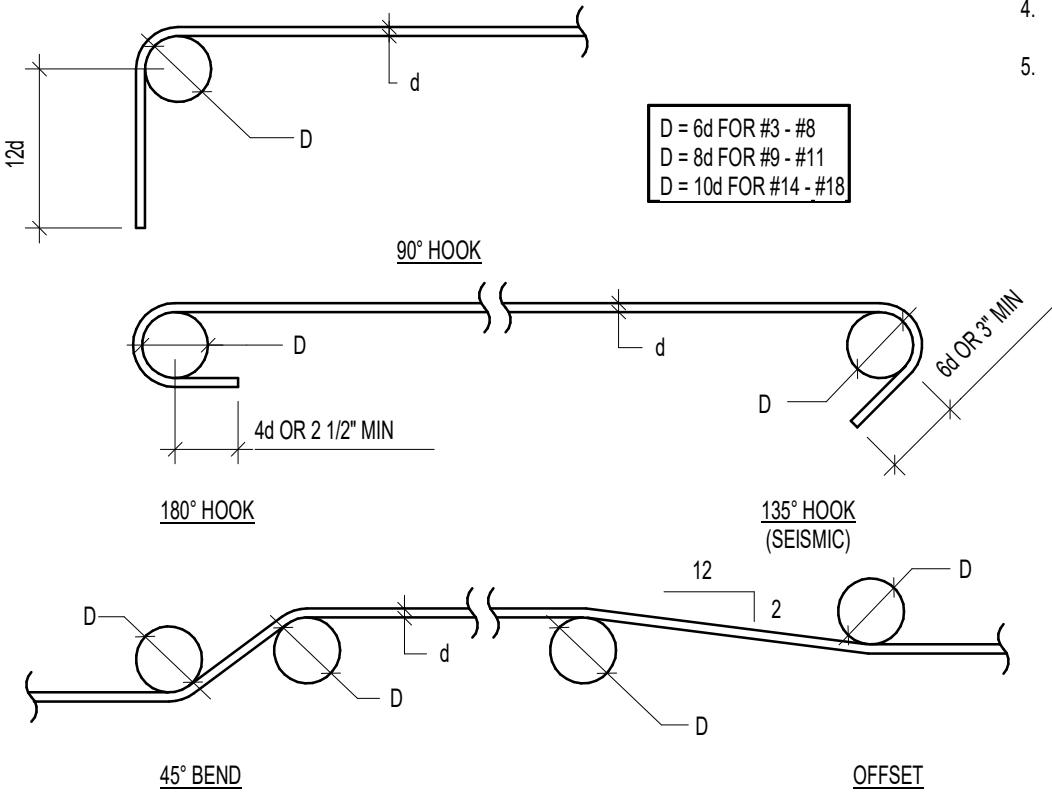
CONCRETE FOOTING NOTES:

1. PLACE ALL FOOTING REINFORCING IN BOTTOM OF FOOTING WITH 3" CLEAR CONCRETE COVER, UNLESS NOTED OTHERWISE.
2. TOP REINFORCING, WHERE SPECIFIED, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH FORMED, FOOTING WIDTH AND LENGTH SHALL BE 6" WIDER AND LONGER THAN SCHEDULED.
4. SEE GENERAL STRUCTURAL NOTES FOR ALL OTHER REQUIREMENTS.
5. NOT ALL FOOTINGS ARE USED. SEE FOUNDATION PLAN FOR FOOTING MARKS.
6. RUN CONTINUOUS BARS IN FC FOOTING THROUGH INTERSECTED FS FOOTINGS.
7. EXTEND CONTINUOUS FOOTINGS 1" OR BEYOND END OF WALL, EXCEPT AT INTERSECTING CORNERS OR UNO ON PLAN.
8. FOOTINGS MAY BE THICKER THAN THE SCHEDULED DEPTH IN AREAS SURROUNDING ANCHOR BOLTS OR HOLD DOWNS, SEE ANCHORAGE AND HOLD DOWN DETAILS.
9. IN FC FOOTINGS CROSSWISE BAR SHALL BE BELOW THE LENGTHWISE BAR

CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE																								
BAR SIZE	f _c = 3000 PSI				f _c = 3500 PSI				f _c = 4000 PSI				f _c = 4500 PSI				f _c = 5000 PSI				f _c = 6000 PSI			
	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP	
	CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS	
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B
#3	17"	22"	22"	28"	16"	21"	21"	26"	15"	19"	19"	25"	14"	18"	18"	23"	13"	17"	17"	22"	12"	16"	16"	20"
#4	22"	29"	29"	38"	21"	27"	27"	36"	19"	25"	25"	33"	18"	24"	24"	31"	17"	23"	23"	29"	16"	21"	21"	27"
#5	28"	36"	36"	47"	26"	34"	34"	44"	24"	31"	31"	41"	23"	30"	30"	38"	22"	28"	28"	36"	20"	26"	26"	33"
#6	33"	43"	43"	56"	31"	40"	40"	52"	29"	37"	37"	49"	27"	35"	35"	46"	26"	34"	34"	44"	24"	31"	31"	40"
#7	48"	63"	63"	81"	45"	59"	59"	75"	42"	54"	54"	71"	40"	51"	51"	67"	38"	49"	49"	63"	34"	45"	45"	58"
#8	55"	72"	72"	93"	51"	67"	67"	82"	48"	62"	62"	81"	45"	59"	59"	76"	43"	56"	56"	72"	39"	51"	51"	66"
#9	62"	81"	81"	105"	58"	75"	75"	98"	54"	70"	70"	91"	51"	66"	66"	86"	48"	63"	63"	81"	44"	57"	57"	74"
#10	70"	91"	91"	118"	65"	85"	85"	110"	61"	79"	79"	102"	57"	74"	74"	96"	54"	71"	71"	92"	50"	64"	64"	84"
#11	78"	101"	101"	131"	73"	94"	94"	122"	67"	87"	87"	114"	64"	82"	82"	107"	60"	78"	78"	102"	55"	71"	71"	93"

NOTES:

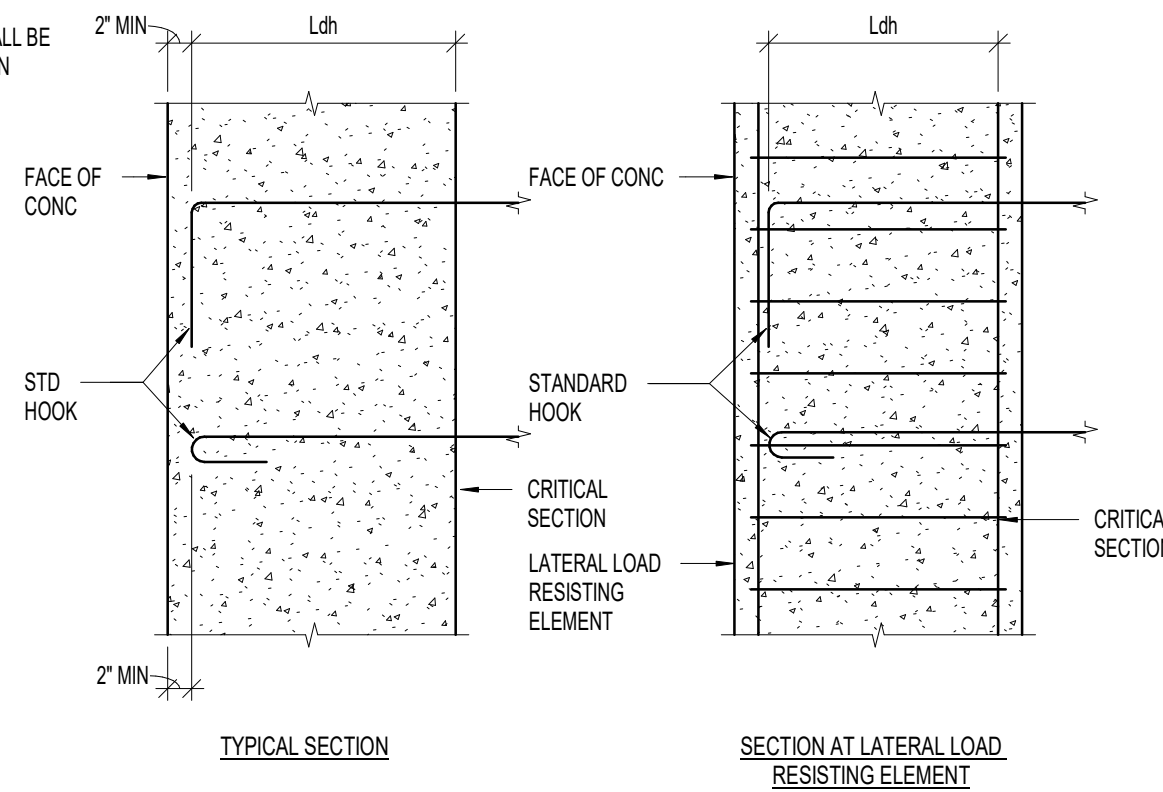
1. THIS SCHEDULE SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE.
2. HORIZONTAL BARS ARE CLASSIFIED AS TOP BARS WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BARS.
3. CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS NOTED OTHERWISE.
4. TIES AND STIRRUPS SHALL NOT BE SPLICED.
5. FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
6. FOR ALL EPOXY COATED BARS, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5 FOR BARS WITH CLEAR COVER LESS THAN 3 BAR DIAMETERS OR CLEAR SPACING LESS THAN 6 BAR DIAMETERS, OTHERWISE MULTIPLY BY 1.2.
7. LAP LENGTHS SHALL BE MULTIPLIED BY 1.25 AT SHEARWALL BOUNDARY ELEMENTS.
8. DEVELOPMENT LENGTH L_d IS EQUAL TO CLASS 'A' SPLICE.
9. IF REINFORCING HAS CLEAR COVER LESS THAN ONE BAR DIAMETER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
10. IF REINFORCING IS NOT ENCLOSED IN TIES OR STIRRUPS AND IS SPACED TIGHTER THAN 2 BAR DIAMETERS ON CENTER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
11. LAP LENGTHS SHALL BE MULTIPLIED BY 1.25 FOR GRADE 75 REBAR.
12. WHERE BARS OF DIFFERENT SIZES ARE LAPPED, THE SPLICE LENGTH SHALL BE THE LARGER OF 'L_d' OF THE LARGER BARS AND THE SPLICE LENGTH OF THE SMALLER BAR.



HOOKED BAR DEVELOPMENT LENGTHS, L _{dh}						
BAR SIZE	f _c = 3000 PSI	f _c = 4000 PSI	f _c = 4500 PSI	f _c = 5000 PSI	f _c = 6000 PSI	
#3	9"	8"	7"	7"	6"	
#4	11"	10"	9"	9"	8"	
#5	14"	12"	12"	11"	10"	
#6	17"	15"	14"	13"	12"	
#7	20"	17"	16"	15"	14"	
#8	22"	19"	18"	17"	16"	
#9	25"	22"	21"	20"	18"	
#10	28"	25"	23"	22"	20"	
#11	31"	27"	26"	24"	22"	

NOTES:

1. FOR GRADE 75 REBAR, MULTIPLY LENGTHS BY 1.25.
2. FOR LIGHTWEIGHT CONCRETE, MULTIPLY LENGTHS BY 1.3.
3. FOR EPOXY COATED REINFORCEMENT, MULTIPLY LENGTHS BY 1.2.
4. FOR HOOKS WITH 2.5" MINIMUM SIDE COVER PERPENDICULAR TO PLANE OF HOOK, MULTIPLY LENGTHS BY 0.7.
5. FOR LATERAL LOAD RESISTING ELEMENTS, CRITICAL SECTIONS SHALL BE TAKEN AS THE FACE OF TIE / HOOP AT CONFINED CORES OF COLUMN JOINTS OR SHEAR WALL BOUNDARY ZONE.



1 CONCRETE FOOTING SCHEDULE
S-801 NO SCALE:

2 CONCRETE REINFORCING BAR LAP SCHEDULES AND DIAGRAMS
S-801 NO SCALE:

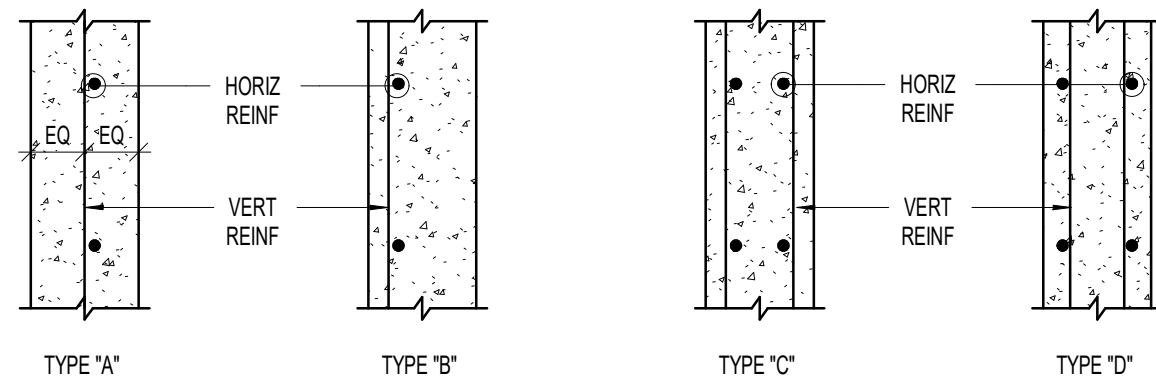
CONCRETE WALL SCHEDULE						
MARK	THICKNESS	REINFORCING			WALL TYPE	COMMENTS
		VERTICAL	HORIZONTAL	TOP AND BOTTOM		
CW-08	8"	#4 AT 18"oc	#4 AT 12"oc	(1) #5	A	

CONCRETE WALL NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR COVER AND OTHER REQUIREMENTS NOT NOTED IN SCHEDULE.
2. CONCRETE WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:

THICKNESS	VERTICAL REINFORCING	HORIZONTAL REINFORCING
6"	#4 BARS AT 18"oc	#4 BARS AT 18"oc
8"	#4 BARS AT 18"oc	#4 BARS AT 12"oc
10"	#4 BARS AT 16"oc	#5 BARS AT 15"oc
12"	#4 BARS AT 18"oc EA FACE	#4 BARS AT 16"oc EA FACE
3. PLACE STEEL IN THE CENTER OF THE WALL (EXCEPT TYPE 'B' AND RETAINING WALLS). WALLS THICKER THAN 10" SHALL HAVE TWO CURTAINS OF REINFORCEMENT (PLACED NEAR EA FACE OF THE WALL), UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.

WALL REINFORCEMENT PLACEMENT TYPES:



3 CONCRETE WALL SCHEDULE
S-801 NO SCALE:

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Project Name _____

Sheet Title

SCHEDULES

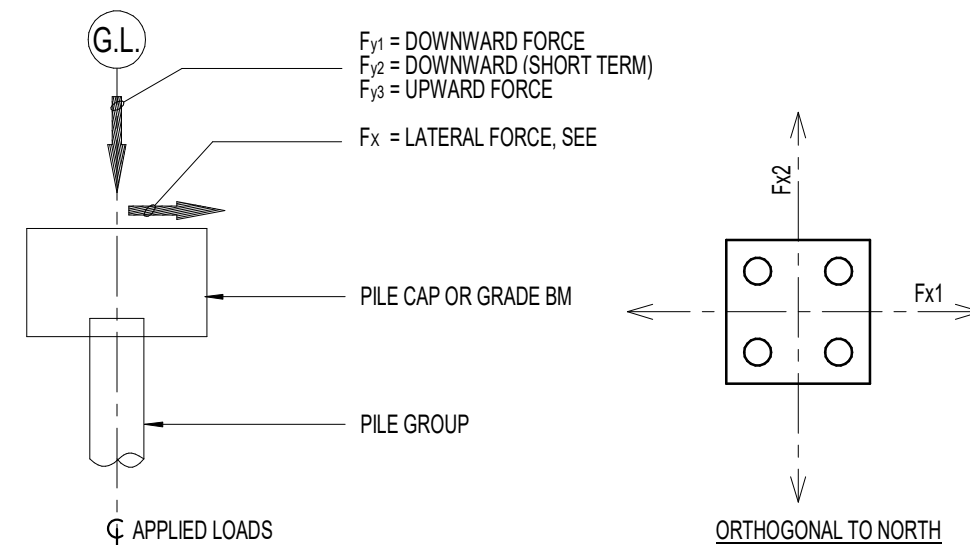
Scale	Date
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Drawn	Project No.
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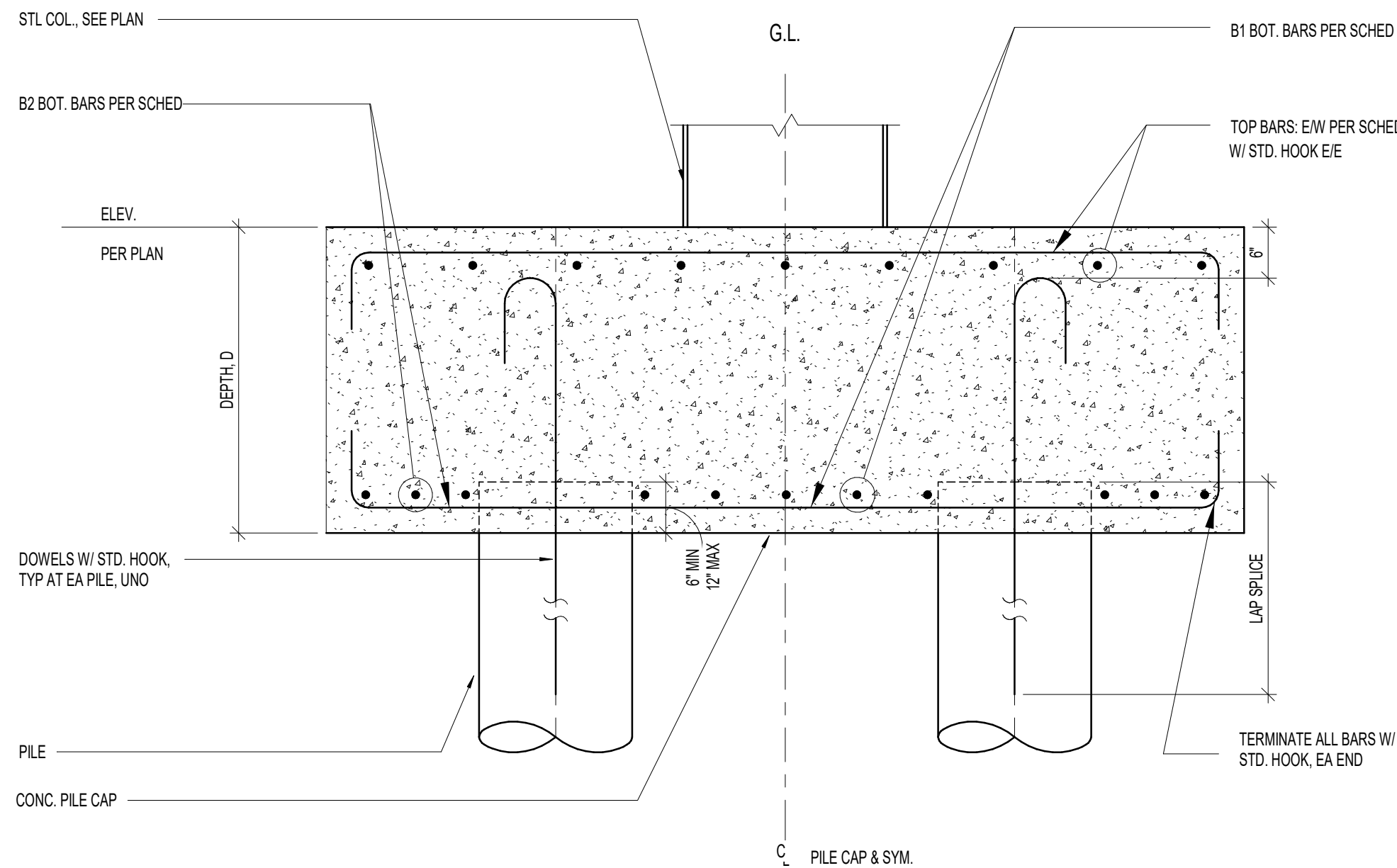
Sheet No. 22159

S-802

1. PILES SHALL BE GROUTED MICRO-PILES. THE PILES SHALL BE DESIGNED IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE (IBC) BY AN ENGINEER LICENSED IN THE STATE OF UTAH AND RETAINED BY THE OWNER.
2. THE CONTRACTOR SHALL SUBMIT PILE SHOP DRAWINGS AND STRUCTURAL CALCULATIONS STAMPED AND SIGNED BY THE CONTRACTOR'S ENGINEER. SHOP DRAWINGS WILL BE REVIEWED BY THE ARCHITECT, STRUCTURAL ENGINEER, AND GEOTECHNICAL ENGINEER AND ARE SUBJECT TO THE APPROVAL OF THE BUILDING DEPARTMENT.
3. PILES SHALL BE DESIGNED TO PROVIDE ADEQUATE RESISTANCE TO THE APPLIED ALLOWABLE STRESS DESIGN (ASD) LOADS LISTED IN THE TABLE FOR EACH PILE GROUP. REFERENCE PROJECT GEOTECHNICAL REPORT.
4. PILES SHALL BE DESIGNED TO PROVIDE STRENGTH ADEQUATE TO RESIST THE INTERNAL FORCES GENERATED BY THE APPLIED STRENGTH DESIGN LOADS LISTED IN THE TABLE FOR EACH PILE GROUP. THESE FORCES SHALL BE CONSIDERED IN COMBINATION WITH THE LATERAL LOADS. (SEE NOTE R8). SEE DETAIL (B33802) FOR THE DIRECTION OF THE APPLIED LOAD.
5. PILES SHALL BE DESIGNED TO PROVIDE ADEQUATE RESISTANCE AND STRENGTH TO RESIST APPLIED LATERAL LOAD LISTED IN THE TABLE FOR EACH PILE GROUP. THE APPLIED LATERAL LOAD IS REPORTED AT THE ALLOWABLE STRESS DESIGN (ASD) FORCE LEVEL. SEE DETAIL (B33802) FOR THE DIRECTION OF THE APPLIED LOAD.
6. PILES SHALL BE DESIGNED AS PINNED HEAD OR FIXED HEAD AS REQUIRED TO MEET DEFLECTION LIMIT. LATERAL DEFLECTION SHALL BE LIMITED TO "1" UNDER ALLOWABLE STRESS DESIGN LEVEL LOADS.
7. THE NUMBER OF PILES SHOWN IN EACH GROUP ON PLAN CAN BE ADJUSTED BY THE CONTRACTOR PROVIDED THAT THE ADEQUACY OF THE MODIFIED DESIGN IS DEMONSTRATED. THE MODIFIED DESIGN SHALL USE ONE OF THE GROUP CONFIGURATIONS SHOWN AND SHALL MAINTAIN A MINIMUM PILE SPACING OF 3 PILE DIAMETERS.
8. PILE INSTALLATION SHALL BE PERFORMED UNDER THE OBSERVATION OF THE GEOTECHNICAL ENGINEER.
9. PILE DESIGNER TO COORD PILE HEAD DETAILING WITH ANCHOR BOLT SETTING TO AVOID CONFLICTS. PILE EMBEDMENT SHALL BE ENCASED AS REQ'D AT THICKENED REGION AT MOMENT FRAME COLUMN (MFC) PILE CAPS AS REQUIRED.

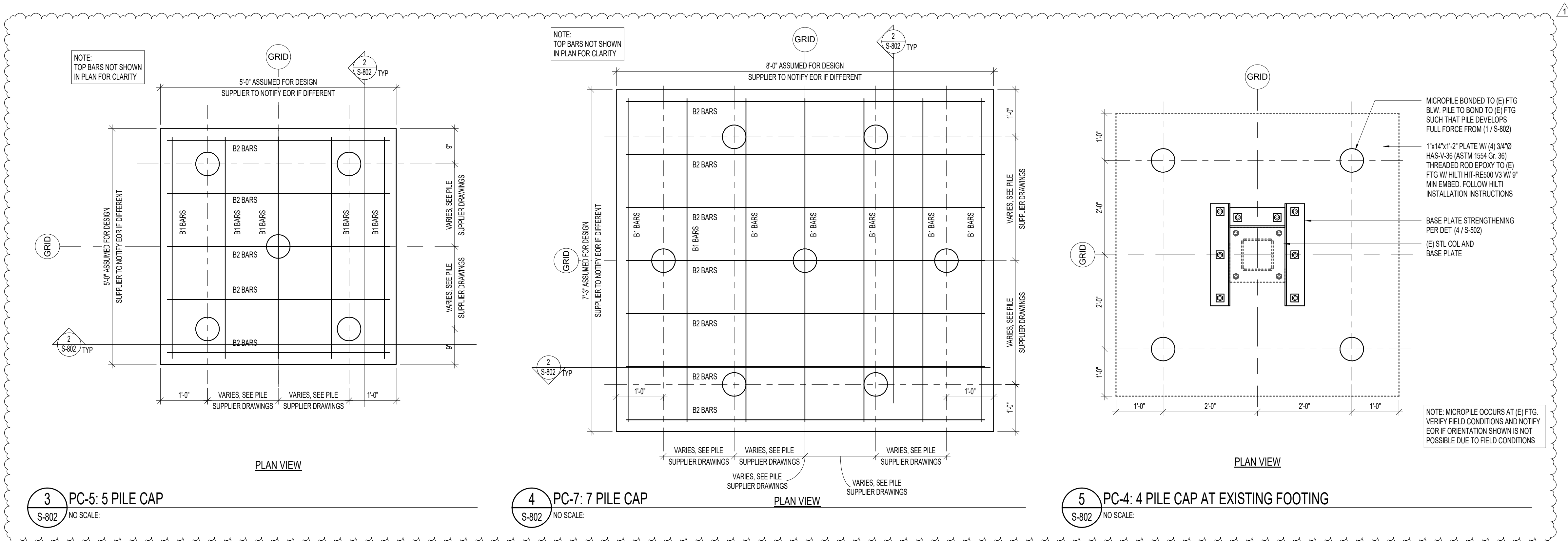


PILE CAP FOR 6" MICROPILES (ASSUMED)							
MARK	LENGTH (L)	WIDTH (B)	DEPTH (D)	BOT. BARS (B1)	BOT. BARS (B2)	TOP BARS	DETAIL
PC-5	DET	DET	2'-6"	(5) #8	(5) #8	(5) #8 EW	(3 / S-802)
PC-7	DET	DET	3'-0"	(7) #8	(7) #8	(7) #8 EW	(4 / S-802)



1 PILE GROUP LOAD SCHEDULE

2 PILE CAP SCHEDULE
S-802 NO SCALE:



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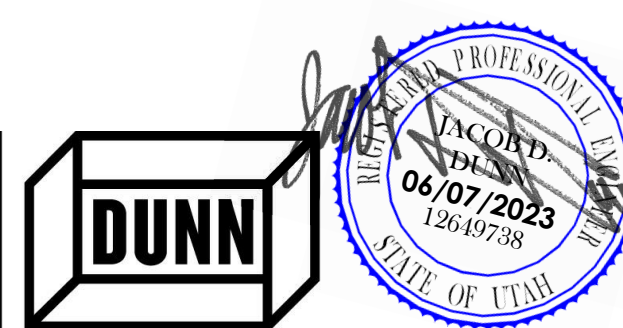
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Project Name
ALBANY MEZZANINE

Sheet Title

SCHEDULES

Scale	Date 2023.03.28
Drawn JDD	Project No. 22159

S-803

STEEL COLUMN NOTES:

- ALL ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 55 (AT CONTRATOR'S OPTION, GRADE 55 ANCHORS MAY BE SUBSTITUTED WITH GRADE 105 ANCHORS), UNLESS NOTED OTHERWISE. ALL COLUMNS SHALL BE INSTALLED WITH HEADED (OR DOUBLE NUT) ANCHOR BOLTS. PROJECT ANCHOR BOLTS 4" MINIMUM ABOVE THE TOP OF THE BASE PLATE.
- ANCHOR BOLTS SHALL NOT BE WELDED (INCLUDING TACK WELDS).
- IF DESIRED SPlice LOCATIONS DIFFER FROM THOSE LEVELS SHOWN ON PLAN, NOTIFY STRUCTURAL ENGINEER PRIOR TO FABRICATION. WRITTEN APPROVAL REQUIRED.
- ALL CAP PLATE BOLTS SHALL BE 3/4" Ø BOLTS, TYPICAL UNLESS NOTED OTHERWISE.
- ALL CAP PLATES TO BE 3/4" THICK, UNO.
- SEE GENERAL STRUCTURAL NOTES FOR OTHER REQUIREMENTS.
- ERECTION AIDS TO BE REMOVED AFTER COLUMN SPLICING.
- FOR HSS14x14 AND HSS16x16 COLUMNS, ANCHOR BOLTS SHOULD BE 1" DIAMETER WITH 1/2" EMBEDS W/ 3/8"x2" PLATE WASHERS. FOR HSS 12x12 COLUMNS OR SMALLER, ANCHOR BOLTS SHOULD BE 3/4" Ø WITH 8" EMBEDS WITH 1/4"x2" PLATE WASHERS. FOR HSS10x10 COLUMNS OR SMALLER HSS COLUMNS, ASTM F1554 GRADE 36 RODS MAY BE SUBSTITUTED.
- OVERSIZED HOLES MAY BE USED IN BASE PLATES PROVIDED THEY DO NOT EXCEED THE FOLLOWING SIZES:
3/4" Ø BOLT ≤ 1 1/16"
1" Ø BOLT ≤ 1 1/2"
1 1/4" Ø BOLT ≤ 1 7/8"
1 1/2" Ø BOLT ≤ 2 1/4"
11. HOLES IN PLATE WASHERS SHOULD BE 1/16" GREATER THAN BOLT DIMENSION.
- NON-SHRINK GROUT UNDER BASE PLATES SHALL BE 1/2" THICK UNO.
- COLUMN LOCATIONS SHOWN ON SCHEDULE ARE APPROXIMATE. PLEASE SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS.
- DO NOT WELD IN WIDE FLANGE "K" ZONES. WELDS ARE NOT REQUIRED AT "K" ZONES.
- WELDING OF PLATE WASHERS TO BASE PLATES MAY BE OMITTED AT SINGLE STORY COLUMNS AND HSS 8x8 COLUMNS OR SMALLER.

(A) = PLATE WASHER THICKNESS - 1/16" (MAX WELD SIZE 5/16")

(B) = 2" FOR HSS16x16 OTHERWISE 1".

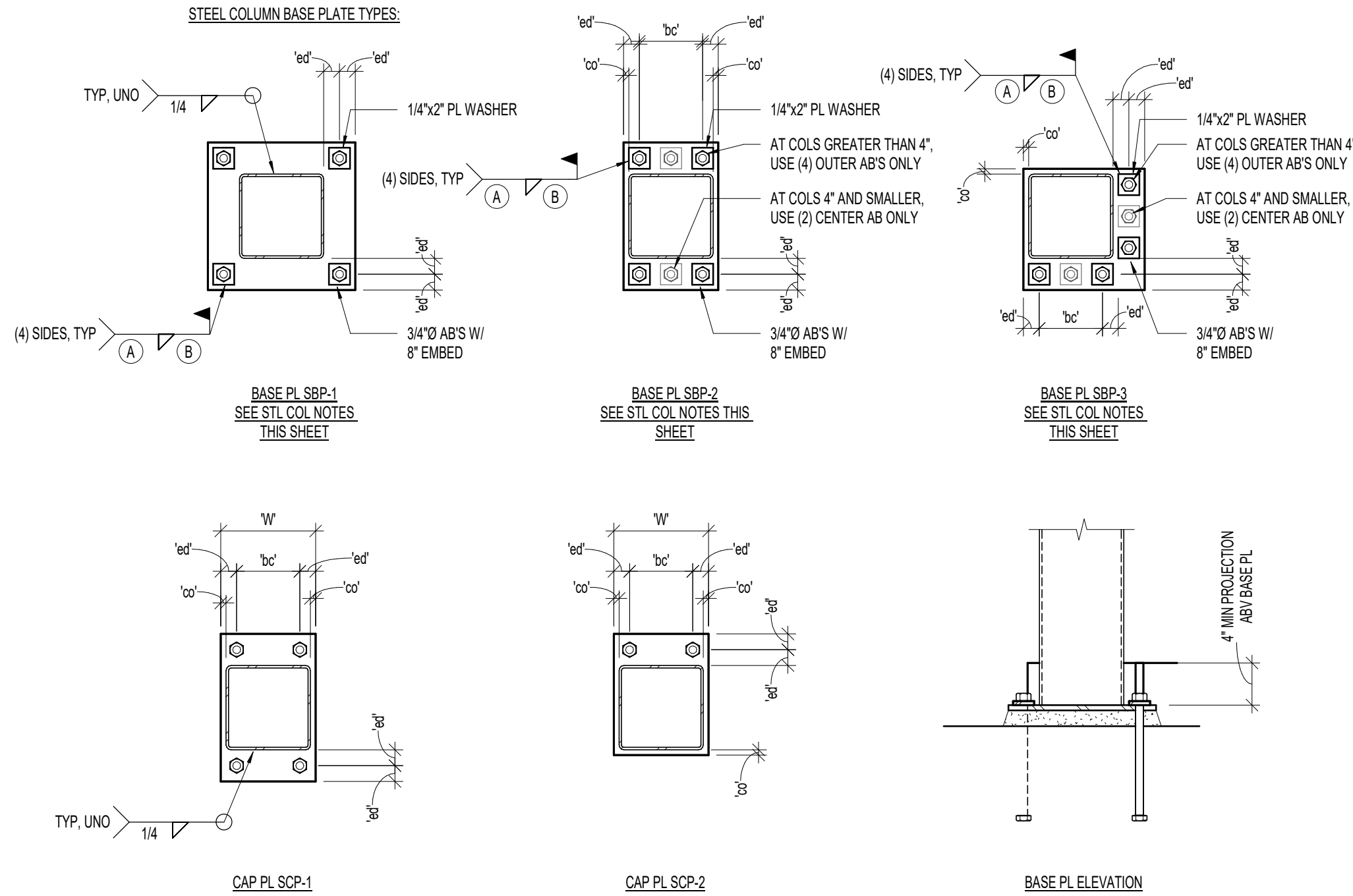
BASE PL LEGEND:

'a' = 1/2" MINIMUM
'b' = 2" MINIMUM
'c' = 3" MINIMUM

CAP PL LEGEND:

'a' = 1/2" MINIMUM
'b' = 1 1/2" MINIMUM
'w' = BEAM OR GIRDER GAGE + 3"
'w' = BEAM OR GIRDER GAGE + 3"
OR
BEAM OR GIRDER FLANGE WIDTH + 1"
OR
COLUMN DEPTH + 1"
WHICHEVER IS GREATER
'H' = BEAM OR GIRDER GAGE + 3"
OR
BEAM OR GIRDER FLANGE WIDTH + 1"
OR
COLUMN WIDTH + 1"
WHICHEVER IS GREATER

STEEL COLUMN BASE PLATE TYPES:



1 STEEL COLUMN SCHEDULE FOR SEISMIC DESIGN CATEGORIES C THRU F

S-803 NO SCALE:

1/8/21/22

A-325N BOLT SCHEDULE

MAXIMUM BEAM SIZE IN EACH BEAM DEPTH GROUP	A-325N BOLTS	
	No. PER BEAM	SIZE
W8	2	7/8" Ø
W10	2	7/8" Ø
W12	3	7/8" Ø
W14	3	7/8" Ø
W16	4	7/8" Ø
W18	5	7/8" Ø
W21	6	7/8" Ø
W24	6	7/8" Ø
W27	7	7/8" Ø
W30	8	7/8" Ø
W33	9	7/8" Ø
W36	10	7/8" Ø
W40	11	7/8" Ø
W44	12	7/8" Ø

1 CLIP ANGLES: 1.5x3 1/2" THICKNESS SHALL BE EQUAL TO ONE HALF THE BEAM WEB THICKNESS PLUS 1/16" (1/4" MIN). FOR TWO ROWS OF BOLTS OR SKEWED CONNECTIONS, USE BENT PLATES. WHERE COLUMN WIDTH IS SMALLER THAN THE CONNECTING CLIP ANGLES, ANGLE LEGS SHALL BE REDUCED TO MATCH WIDTH OF COLUMN.

2 BEAM WEB CONNECTION PLATE THICKNESS EQUALS 3/8" MINIMUM THICK FOR W18 BEAMS OR SMALLER 1/2" MINIMUM THICK FOR W21 BEAMS OR LARGER 3/4" MINIMUM THICK FOR BEAMS WITH WEB GREATER THAN 1" THICK

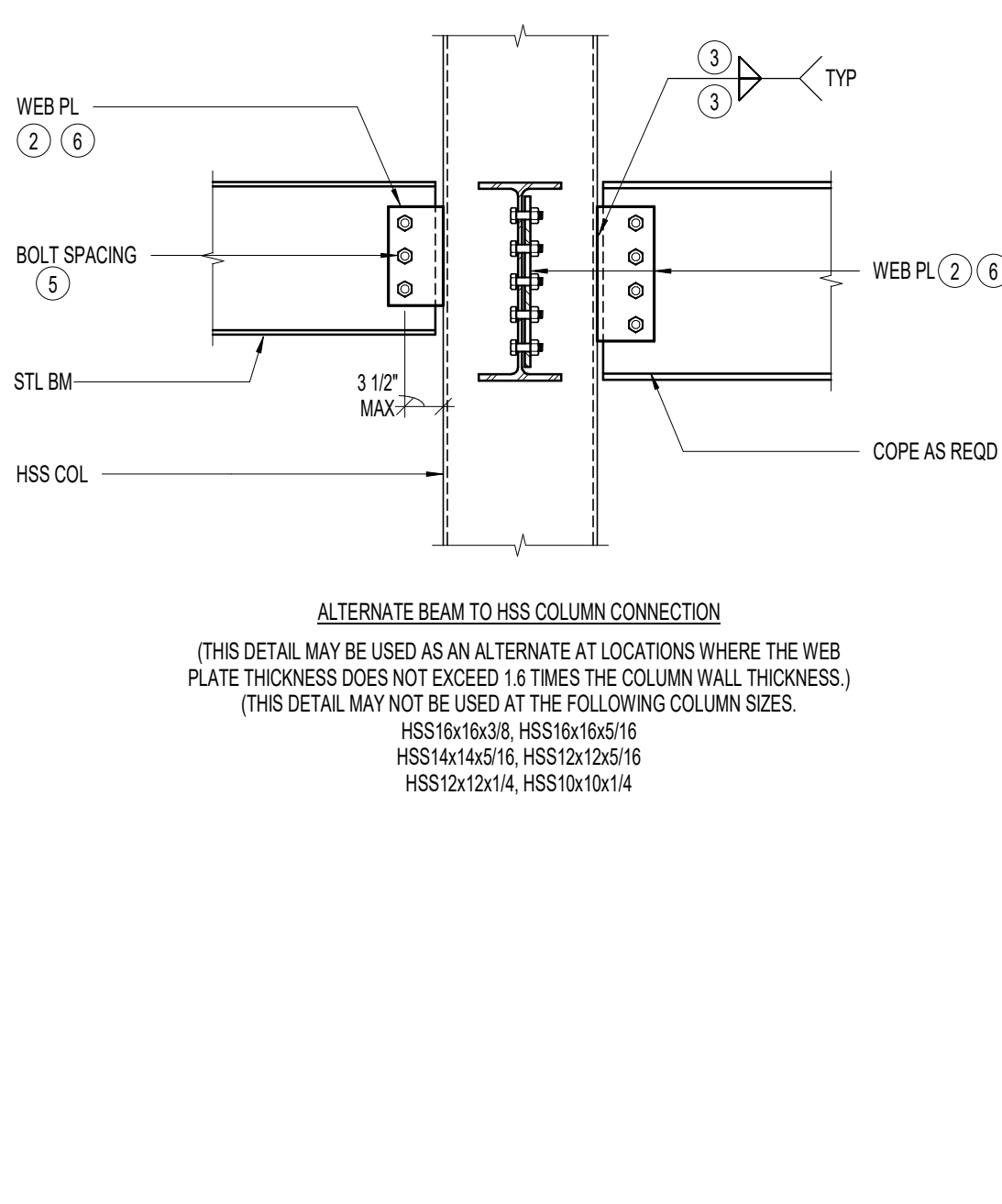
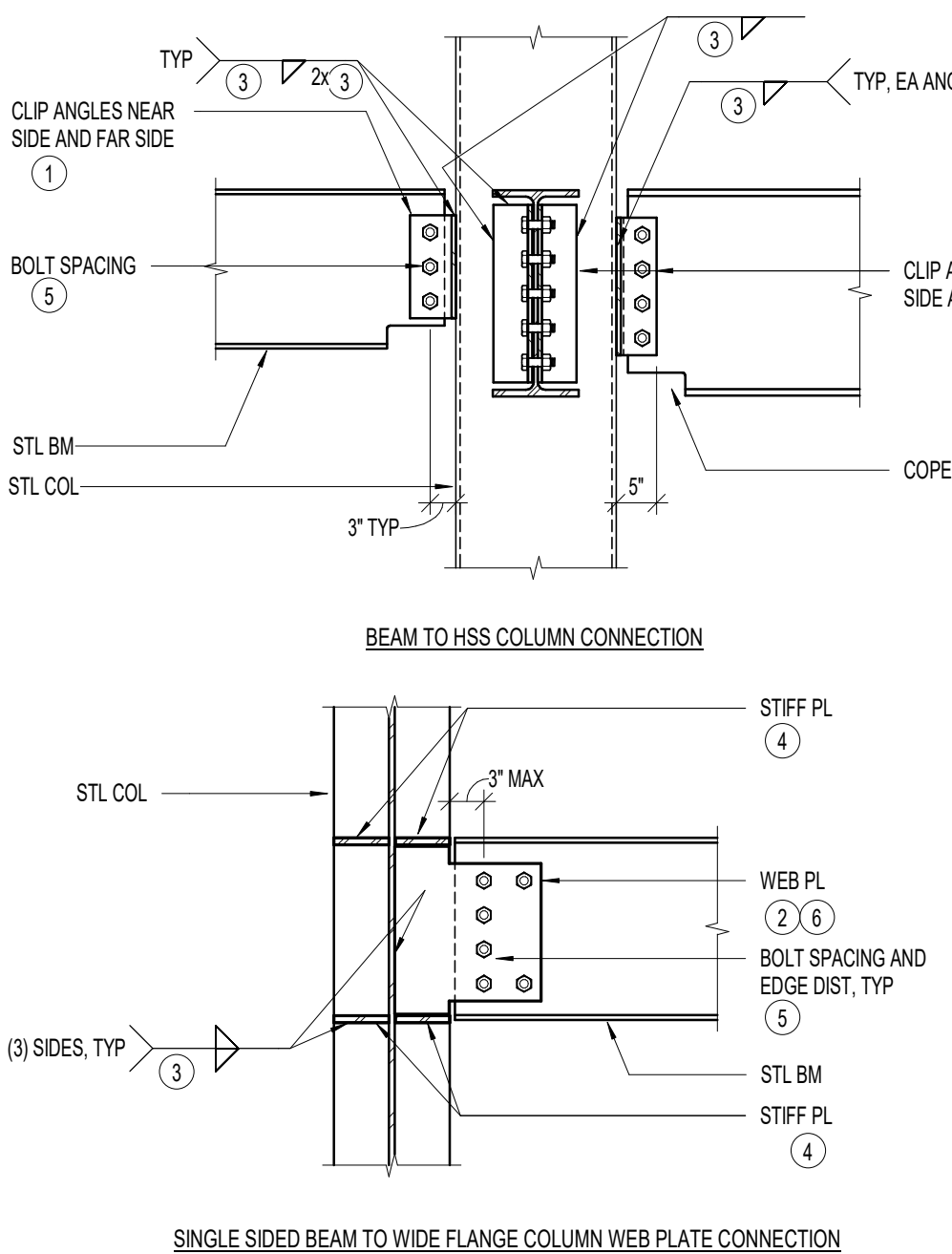
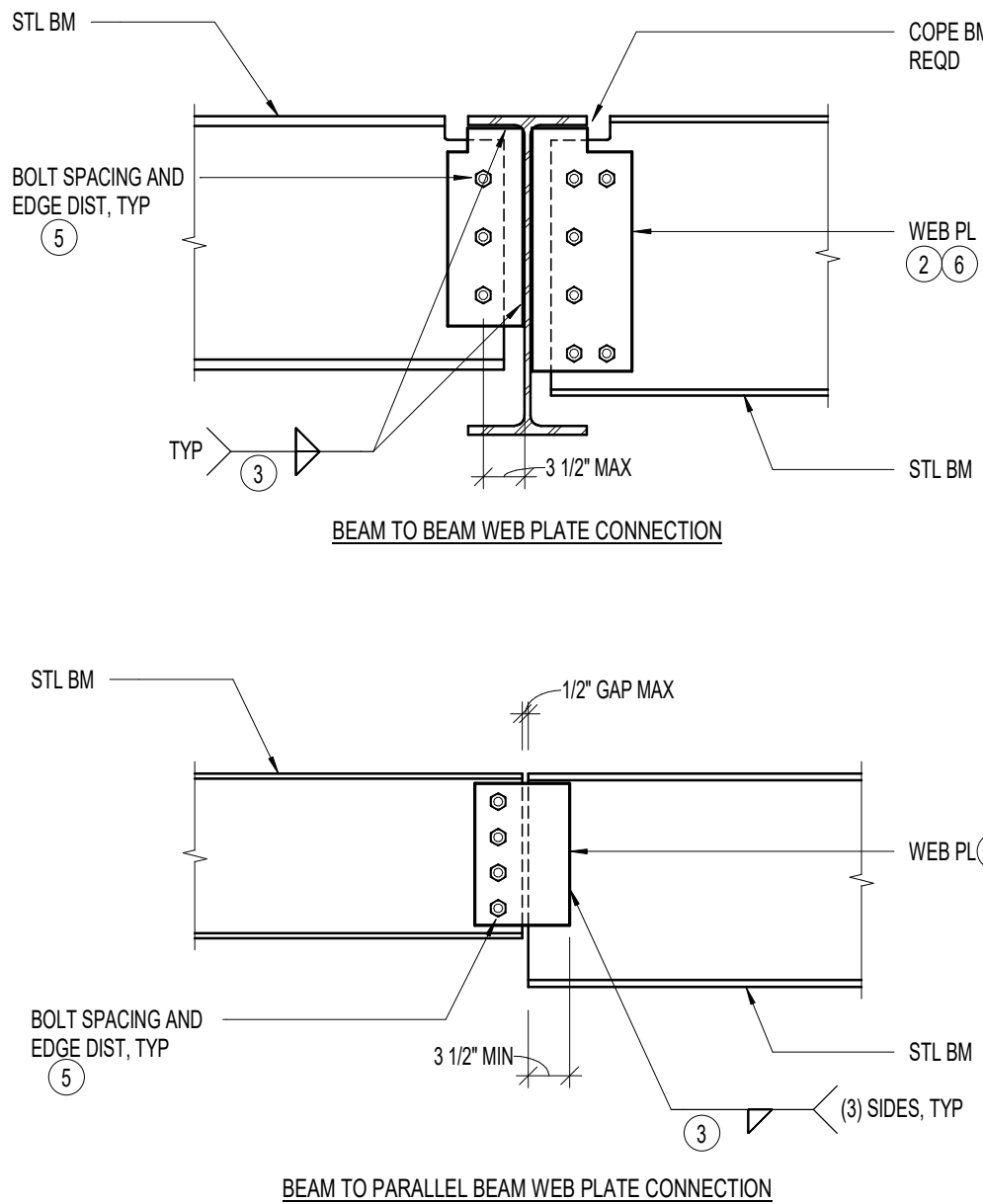
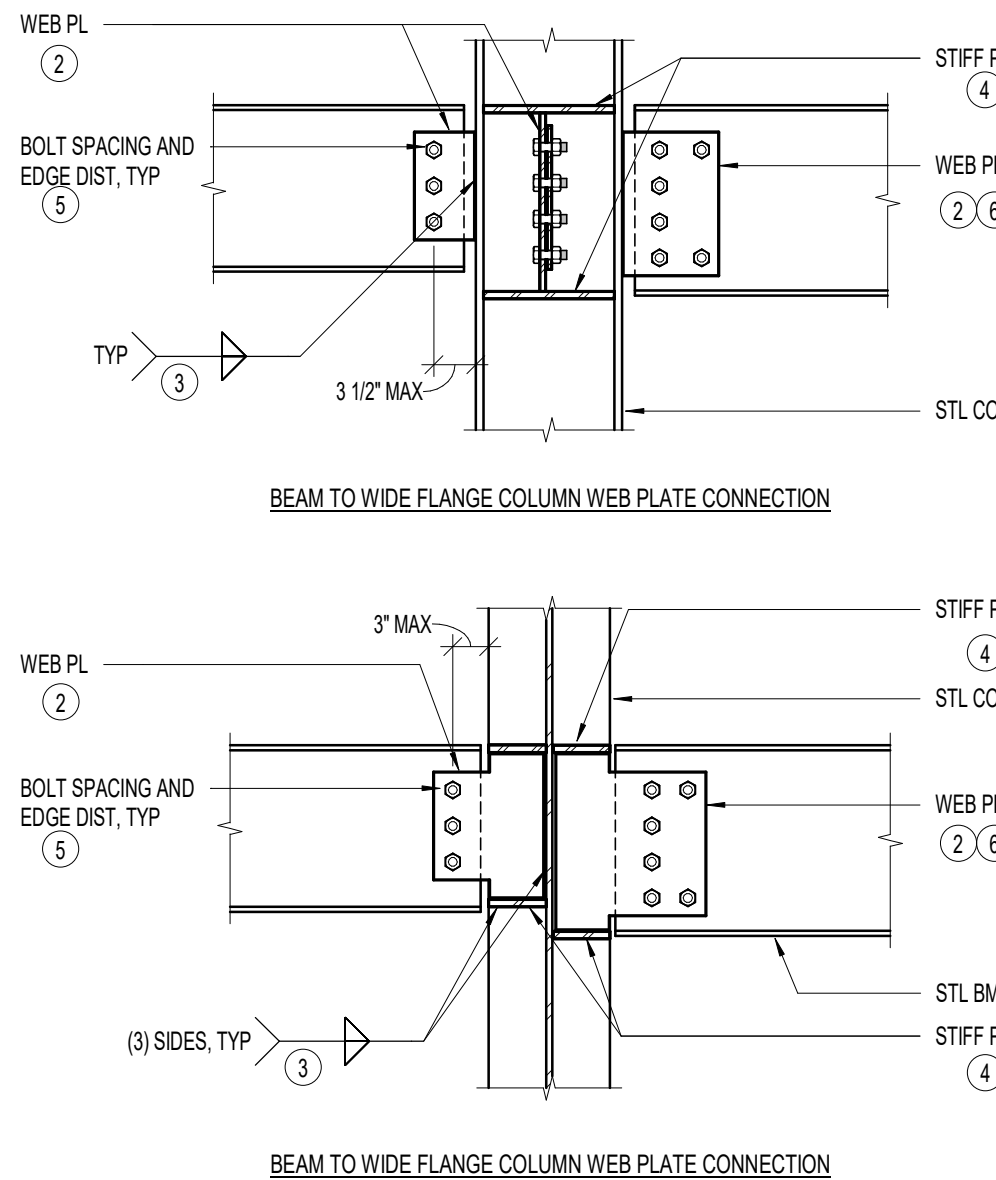
3 FILLET WELDS SHALL BE AS FOLLOWS:
1/4" FOR 3/8" PLATES
5/16" FOR 1/2" PLATES
7/16" FOR 3/4" PLATES

4 THICKNESS EQUALS BEAM FLANGE THICKNESS OF BEAM FRAMING INTO COLUMN WEB (3/8" MINIMUM).

5 BOLT EDGE DISTANCE SHALL BE 1 1/2" MINIMUM AT ALL EDGES. BOLT SPACING SHALL BE AT 3". BOLT SPACING MAY BE REDUCED TO 3x THE BOLT DIAMETER IF IT IS REQUIRED FOR A SINGLE ROW OF BOLTS. A SINGLE ROW OF BOLTS IS PREFERRED.

6 WHEN MORE THAN ONE COLUMN OF BOLTS IS NEEDED, THE FIRST COLUMN SHALL BE COMPLETE WITH THE REMAINDER OF THE BOLTS PLACED IN THE SECOND COLUMN.

7 1/2" PLATE THICKNESS + 5/16"



2 TYPICAL BOLTED WEB PLATE CONNECTIONS WITH BOLT SCHEDULE (SINGLE SHEAR)

S-803 NO SCALE:

1/8/21/22

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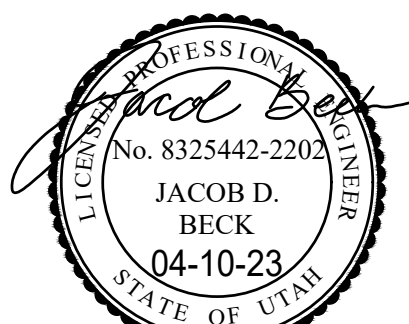
**MECHANICAL
SYMBOLS & LEGEND**

Scale: N/A Date: 04.10.2023

Drawn: GxA Project No: 23020

Sheet No.

M001



LEGEND OF MECHANICAL SYMBOLS AND ABBREVIATIONS

MECHANICAL

	POSITIVE PRESSURE DUCT - RISE
	POSITIVE PRESSURE DUCT - DROP
	NEGATIVE PRESSURE DUCT - RISE
	NEGATIVE PRESSURE DUCT - DROP
	ROUND DUCT - RISE
	ROUND DUCT - DROP
	UNDER FLOOR DUCT
	TURNING VANES
	FRESH AIR LOUVER
	RELIEF AIR OR EXHAUST AIR LOUVER
	CEILING SUPPLY DIFFUSER
	CEILING RETURN REGISTER
	CEILING EXHAUST REGISTER (BALANCE TO MATCH SUPPLY IF RETURN CFM IS NOT SHOWN)
	SIDEWALL SUPPLY REGISTER
	SIDEWALL EXHAUST OR RETURN REGISTER
	CEILING SUPPLY DIFFUSER WITH FLEXIBLE DUCT
	CEILING AIR GRILLE WITH FLEXIBLE DUCT
	CEILING RETURN AIR GRILLE W/ SOUND BOOT
	LINEAR DIFFUSER WITH PLENUM AND FLEXIBLE DUCT CONNECTION, NO. OF SLOTS & SIZE OF SLOT ON TOP, ACTIVE LENGTH AND CFM ON BOTTOM
	FLEXIBLE DUCT CONNECTION
	FLEXIBLE DUCT
	FAN
	FLAT OVAL DUCT WITH NET INSIDE DIMENSIONS SHOWN IN INCHES.
	RECTANGULAR DUCT WITH NET INSIDE DIMENSIONS SHOWN IN INCHES.
	ROUND DUCT WITH NET INSIDE DIMENSIONS SHOWN IN INCHES.
	INCLINED RISE
	INCLINED DROP
	RW=1. ROUND DUCT SIMILAR TO RECTANGULAR
	RECTANGULAR TO RECTANGULAR OR ROUND TO ROUND DUCT TRANSFORMATION MAXIMUM 15° INCLUDED ANGLE EXCEPT WHERE SHOWN OTHERWISE.
	RECTANGULAR TO ROUND DUCT TRANSFORMATION
	BRANCH DUCT SPLIT WITH 6" WIDTH AND MIN. R=WIDTH OF BRANCH DUCT DOWNSTREAM. ELBOW TURNING VANE OPTIONAL.
	TAP ENTRY AREA EQUALS 150% OF BRANCH AREA
	HIGH EFFICIENCY FITTING
	MANUAL VOLUME DAMPER
	FIRE DAMPER IN DUCT, W/ ACCESS PANEL RECD.
	COMBINATION FIRE/SMOKE DAMPER W/ ACCESS PANEL
	SMOKE DAMPER W/ ACCESS PANEL
	BACK DRAFT DAMPER
	ATC DAMPER
	ACCESS PANEL IN DUCT OR PLENUM
	HEATING OR COOLING COIL IN DUCT
	SINGLE DUCT AIR TERMINAL BOX VARIABLE OR CONSTANT VOLUME. MIN. 1-1/2" TERMINAL INLET SIZE STRAIGHT DUCT AT TERMINAL INLET.
	4-WAY BLOW PATTERN
	3-WAY BLOW PATTERN
	2-WAY BLOW PATTERN
	2-WAY BLOW PATTERN
	1-WAY BLOW PATTERN
	DUCT SMOKE DETECTOR
	UNIT HEATER

PLUMBING

	FLOOR SINK
	FLOOR DRAIN
	FLOOR CLEAN-OUT OR CLEAN-OUT TO GRADE
	ROOF DRAIN
	DOWNSPOUT NOZZLE
	ARROW INDICATES DIRECTION OF FLOW IN PIPE
	CHECK VALVE
	PRESSURE REDUCING, EXTERNAL PRESSURE VALVE
	PRESSURE REDUCING, SELF CONTAINED VALVE
	ATC VALVE - 2 WAY
	ATC VALVE - 3 WAY
	SOLENOID VALVE
	GATE VALVE
	GATE VALVE - NON RISING STEM
	GLOBE VALVE
	TEMPERATURE AND PRESSURE TEST PORT
	PRESSURE SWITCH
	GAS COCK
	CALIBRATED BALANCING VALVE WITH GPM INDICATED
	REDUCED PRESSURE BACKFLOW PREVENTOR W/ DRAIN PAN
	BRANCH - BOTTOM CONNECTION
	BRANCH - TOP CONNECTION
	BRANCH - SIDE CONNECTION
	RISE OR DROP
	RISER - DOWN (ELBOW)
	RISER - DOWN (ELBOW)
	VENT THRU ROOF
	WATER HAMMER ARRESTOR
	INLINE PUMP
	INLINE PUMP
	CLEAN-OUT
	RELIEF VALVE
	ANGLE VALVE
	FLOW METER
	UNION
	BALANCING COCK
	SHUT-OFF COCK FOR USE WITH PRESSURE GAUGE
	FLEXIBLE EXPANSION JOINT
	THERMOMETER - TEMP RANGE AS INDICATED
	PRESSURE GAUGE WITH SHUT-OFF COCK
	PRESSURE GAUGE WITH PIGTAIL
	LATERAL STRAINER WITH BLOW-OFF VALVE. PROVIDE HOSE END WITH CAP WHERE DISCHARGE IS NOT PIPED TO DRAIN
	BALL VALVE (PIPE SIZES 2" AND SMALLER) BUTTERFLY VALVE (PIPE SIZES 2-1/2" AND LARGER)
	MOTOR OPERATED BUTTERFLY VALVE
	VALVE IN RISE
	AIR VENT-MANUAL
	AIR VENT-AUTO
	FLOW SWITCH
	REDUCER
	CONCENTRIC REDUCER
	ECCENTRIC REDUCER

PLUMBING CONT.

	THERMOSTATIC MIXING VALVE
	HOSE BIBB
	PIPE CAP
	SWITCH
	SENSOR
	THERMOSTAT
	NIGHT THERMOSTAT
	FILL PORT
	DRAIN PAN AND P-TRAP
	FIXTURE FROM LEVEL ABOVE
	FLOW METER ORIFICE
	FLANGE
	90° ELBOW
	STEAM TRAP, F&T-FLOAT & THERMOSTATIC 45° ELBOW
	B-BUCKET, T-THERMOSTATIC
	LEADER INDICATES DOWNWARD SLOPE
	DEMOLITION
	ALIGNMENT GUIDE
	ANCHOR
	LUBRICATED PLUG COCK

SYMBOLS

	PLUMBING FIXTURES
	POINT OF CONNECTION
	SECTION TAG - TOP FIGURE IS SECTION NO. BOTTOM FIGURE IS SHEET NO.
	DETAIL TAG - TOP FIGURE IS DETAIL NO. BOTTOM FIGURE IS SHEET NO.
	EQUIPMENT IDENTIFICATION
	KEYED NOTE IDENTIFICATION

FIRE

	HOSE VALVE
	NRS GATE VALVE WITH SUPERVISION
	FLOW SWITCH
	FIRE RISER
	SPRINKLER HEAD
	FIRE SPRINKLER WATER

LINETYPES

	ACID VENT
	ACID WASTE
	BOILER BLOW DOWN
	BOILER FEED WATER
	BRINE
	CARBON DIOXIDE
	COMPRESSED AIR
	CHEMICAL FEED
	CHILLED WATER SUPPLY
	CHILLED WATER RETURN
	CONDENSER WATER SUPPLY
	CONDENSER WATER RETURN
	DOMESTIC COLD WATER (DCW)
	DOMESTIC HOT WATER (DHW)
	DOMESTIC HOT WATER RETURN (DHW)
	DEIONIZED WATER SUPPLY
	DEIONIZED WATER RETURN
	EXISTING PIPING
	EXISTING PIPING TO BE REMOVED
	GLYCOL HEAT RECOVERY PIPING
	GLYCOL PIPING SOLUTION
	FUEL OIL RETURN
	FUEL OIL SUPPLY
	FUEL OIL VENT
	NATURAL GAS
	HOT GAS
	HELICOPTER FUEL RETURN
	HELICOPTER FUEL SUPPLY
	HIGH PRESSURE DOMESTIC WATER
	HIGH PRESSURE CONDENSATE
	HIGH PRESSURE STEAM
	HEATING HOT WATER RETURN
	HEATING HOT WATER SUPPLY
	INSTRUMENT AIR
	INSTRUMENT AIR AT PRESSURE INDICATED
	LAB AIR
	LAB VACUUM
	LOW PRESSURE CONDENSATE
	LIQUIFIED PETROLEUM GAS
	LOW PRESSURE STEAM
	MEDICAL AIR
	MEDICAL AIR AT PRESSURE INDICATED
	MEDIUM PRESSURE CONDENSATE
	MEDIUM PRESSURE STEAM
	MAKE UP WATER
	MEDICAL VACUUM
	NITROGEN
	NITROUS OXIDE
	MEDICAL OXYGEN
	MEDICAL OXYGEN AT PRESSURE INDICATED
	PUMPED CONDENSATE

LINETYPES CONT.

	REVERSE OSMOSIS WATER SUPPLY
	REVERSE OSMOSIS WATER RETURN
	ROOF DRAIN
	ROOF DRAIN OVERFLOW
	REFRIGERANT LIQUID
	REFRIGERANT SUCTION
	SEWER (BELOW GRADE)
	SEWER (ABOVE GRADE)
	SOFT DOMESTIC WATER (SW)
	VACUUM
	VENT (SEWER)

MECH. GENERAL NOTES

- DO NOT ROUTE DUCTS AND PIPES ABOVE ELECTRICAL PANELS. ALL ELECTRICAL PANELS MUST HAVE CLEAR ACCESS SPACE IN FRONT OF PANEL 4'-0" DEEP AND 6'-0" HIGH. DO NOT ROUTE DUCTS AND PIPES IN ELECTRICAL ROOMS, EXCEPT DUCTS AND PIPES SERVING THE ROOM.
- ALL DUCT DIMENSIONS ARE INSIDE FREE AREA DIMENSIONS. ADJUST SHEET METAL DIMENSION FOR LINED DUCT.
- IF CONTRACTOR ENCOUNTERS MATERIAL WHICH MAY CONTAIN ASBESTOS IMMEDIATELY STOP WORK IN THIS AREA AND NOTIFY THE OWNER.
- STEEL ROOF DECK SHALL NOT BE USED TO SUPPORT LOADS FROM PIPING, DUCTWORK OR EQUIPMENT, UNLESS NOTED OTHERWISE. HANGER LOADS LESS THAN 50 LBS. MAY BE HUNG FROM THE STEEL ROOF DECK IN CASES WHEN HANGING FROM THE STEEL ROOF DECK CANNOT BE AVOIDED; THE ATTACHMENT METHOD MUST DISTRIBUTE THE LOAD ACROSS THE DECK AS APPROVED BY THE STRUCTURAL ENGINEER.

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Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

Issued/Revisions

No.	Description	Date
1	PERMIT SUBMISSION	03/30/23
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Project Name
ALBANY MEZZANINE

Sheet Title
MECHANICAL
FLOOR PLANS

Scale
1/8" = 1'-0"

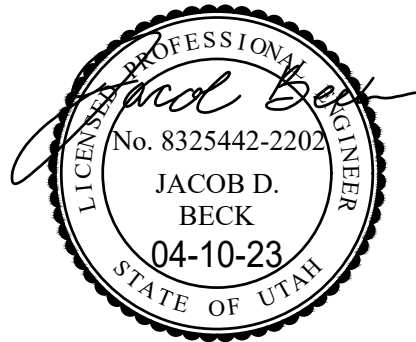
Date
04.10.2023

Drawn
GxA

Project No.
23020

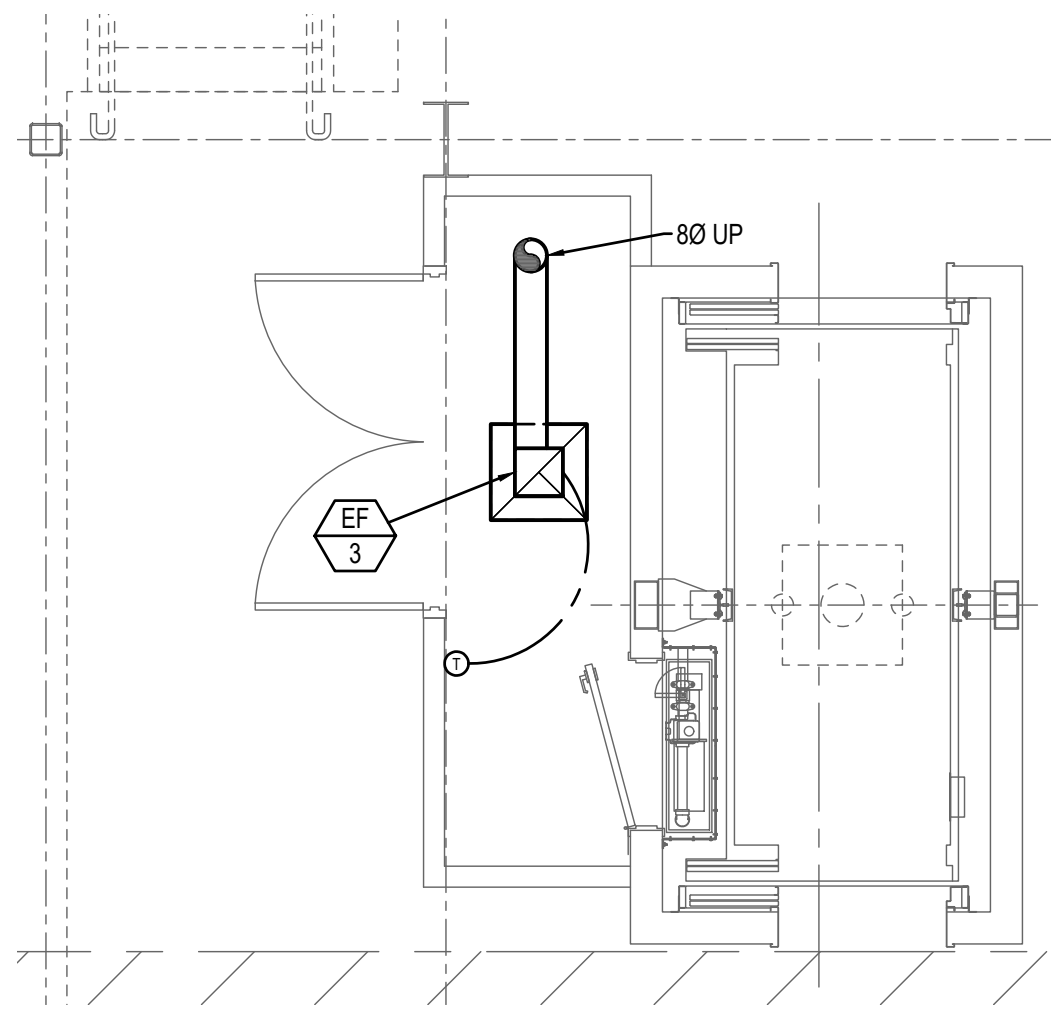
Sheet No.

M101

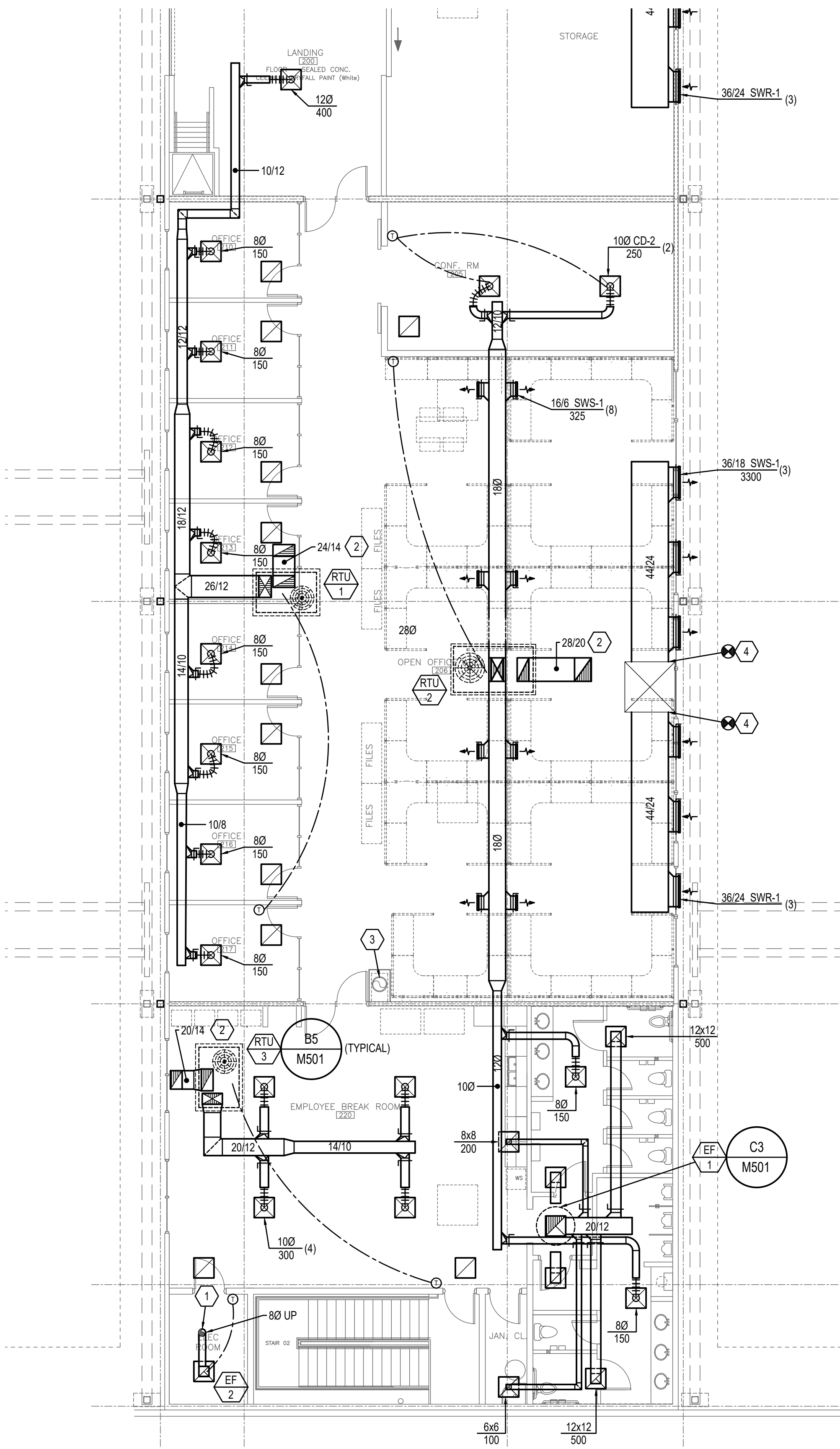


KEYED NOTES

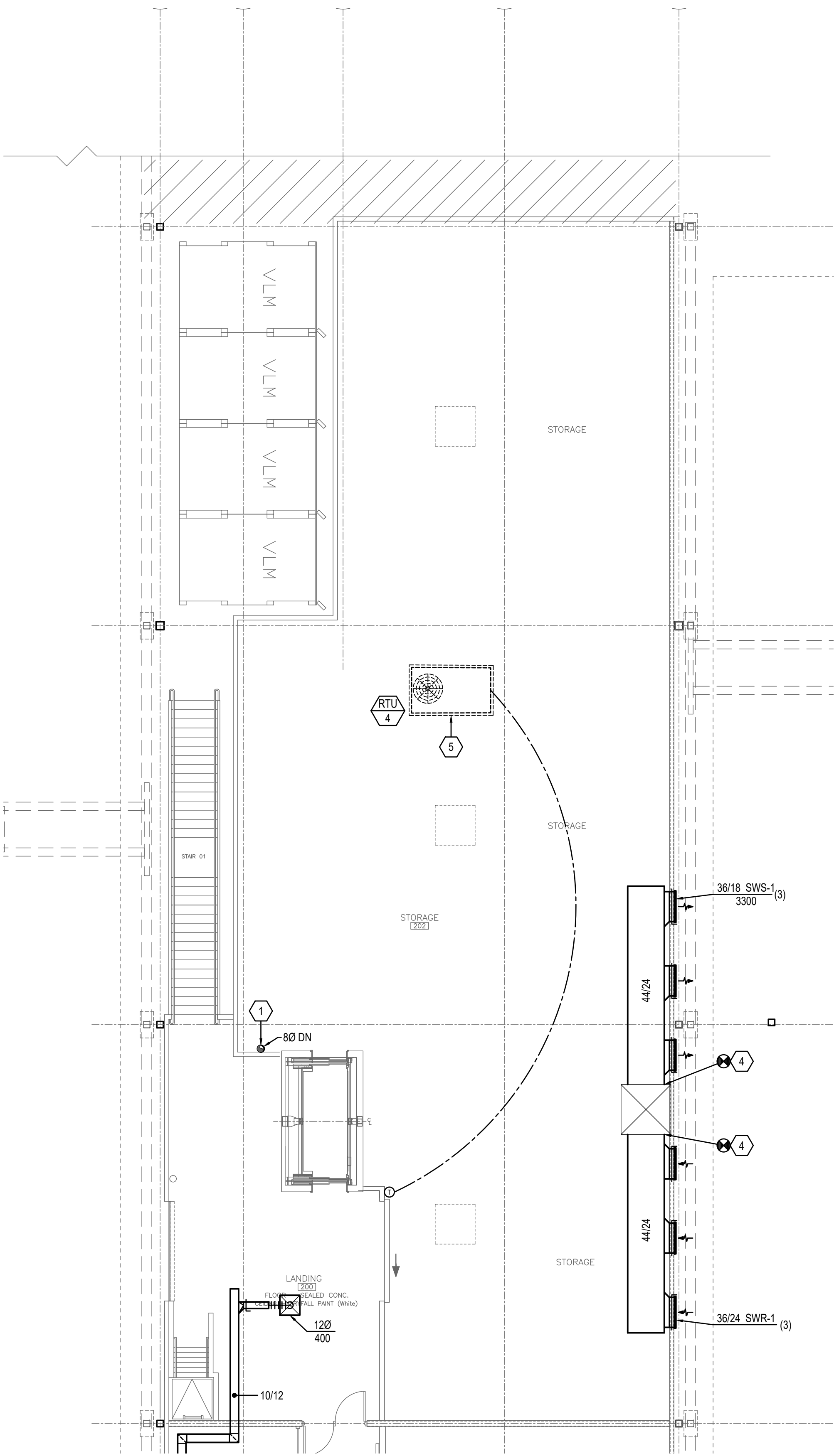
1. EXTEND EXHAUST DUCT THROUGH ROOF AND TERMINATE WITH ROOF CAP.
2. RETURN AIR DUCT TO BE OPEN ON TOP.
3. STAINLESS STEEL DUCTWORK FROM OVEN LOCATED ON LEVEL BELOW. OWNER PROVIDED, CONTRACTOR INSTALLED. COORDINATE WITH ARCHITECTURAL DRAWINGS.
4. CONTRACTOR TO DEMOLISH EXISTING CONCENTRIC DIFFUSER. PROVIDE NEW DUCTWORK AND CONNECT TO EXISTING SUPPLY AND RETURN MAINS. FIELD VERIFY EXISTING CONDITIONS.
5. PROVIDE A CONCENTRIC DUCT FITTING WITH NEW ROOFTOP UNIT. SEE MECHANICAL SCHEDULES.



1 FIRST LEVEL - ELEVATOR ROOM
SCALE: 1/4" = 1'-0"



2 MEZZANINE LEVEL - MECHANICAL FLOOR PLAN
SCALE: 1/8" = 1'-0"



3 MEZZANINE LEVEL - MECHANICAL FLOOR PLAN
SCALE: 1/8" = 1'-0"

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Project Name
ALBANY MEZZANINE

Sheet Title
**MECHANICAL
DETAILS**

Scale
N/A

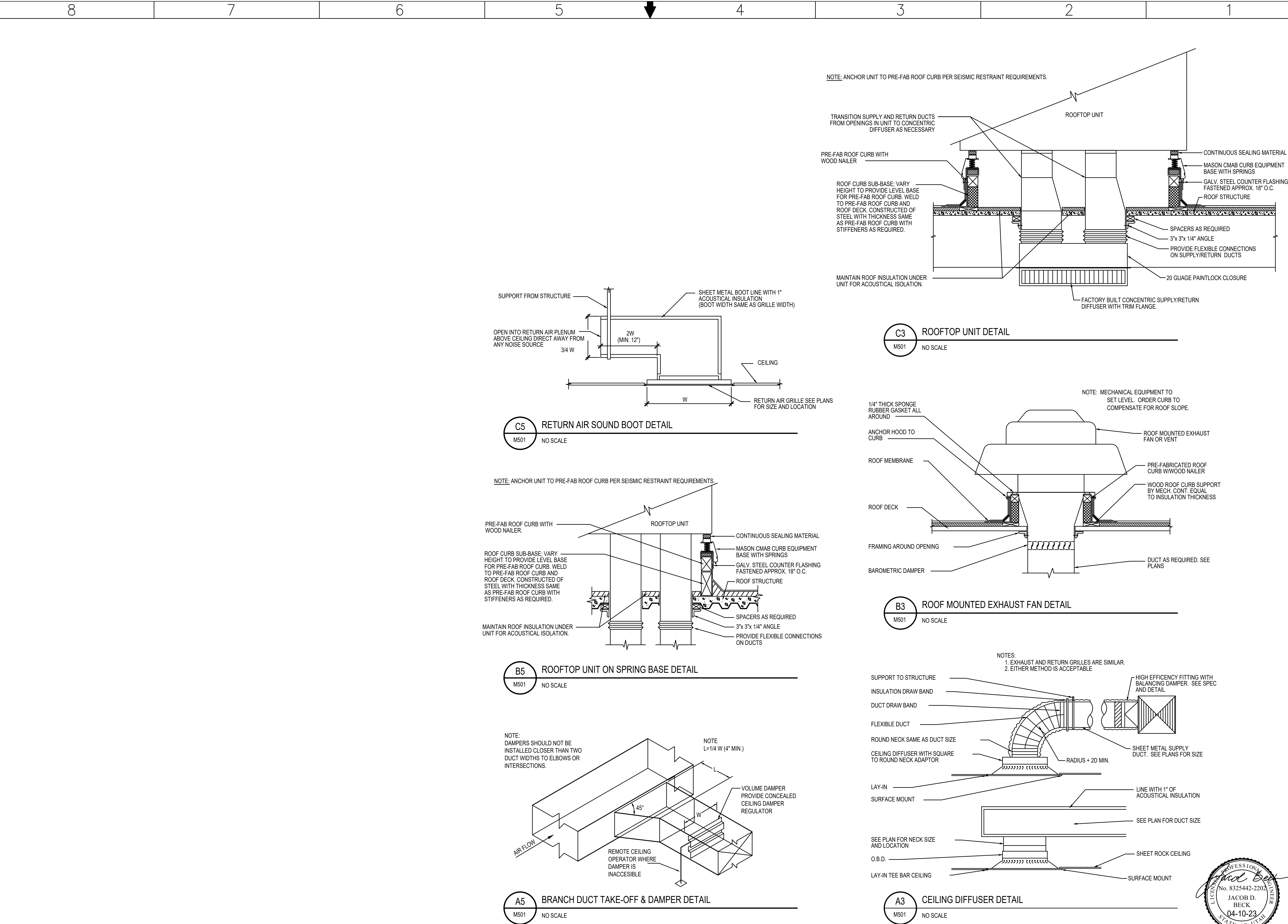
Date
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GxA

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23020

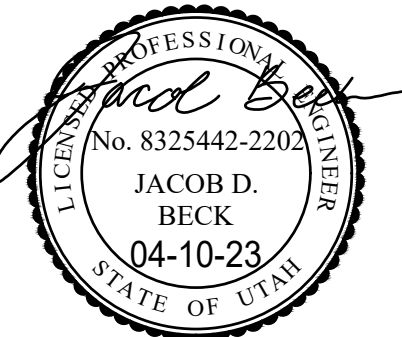
Sheet No.

M501



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Project Name
ALBANY MEZZANINE

Sheet Title
PLUMBING FLOOR PLANS

Scale
1/4" = 1'-0"

Date
04.10.2023

Drawn
GxA

Project No.
23020

Sheet No.

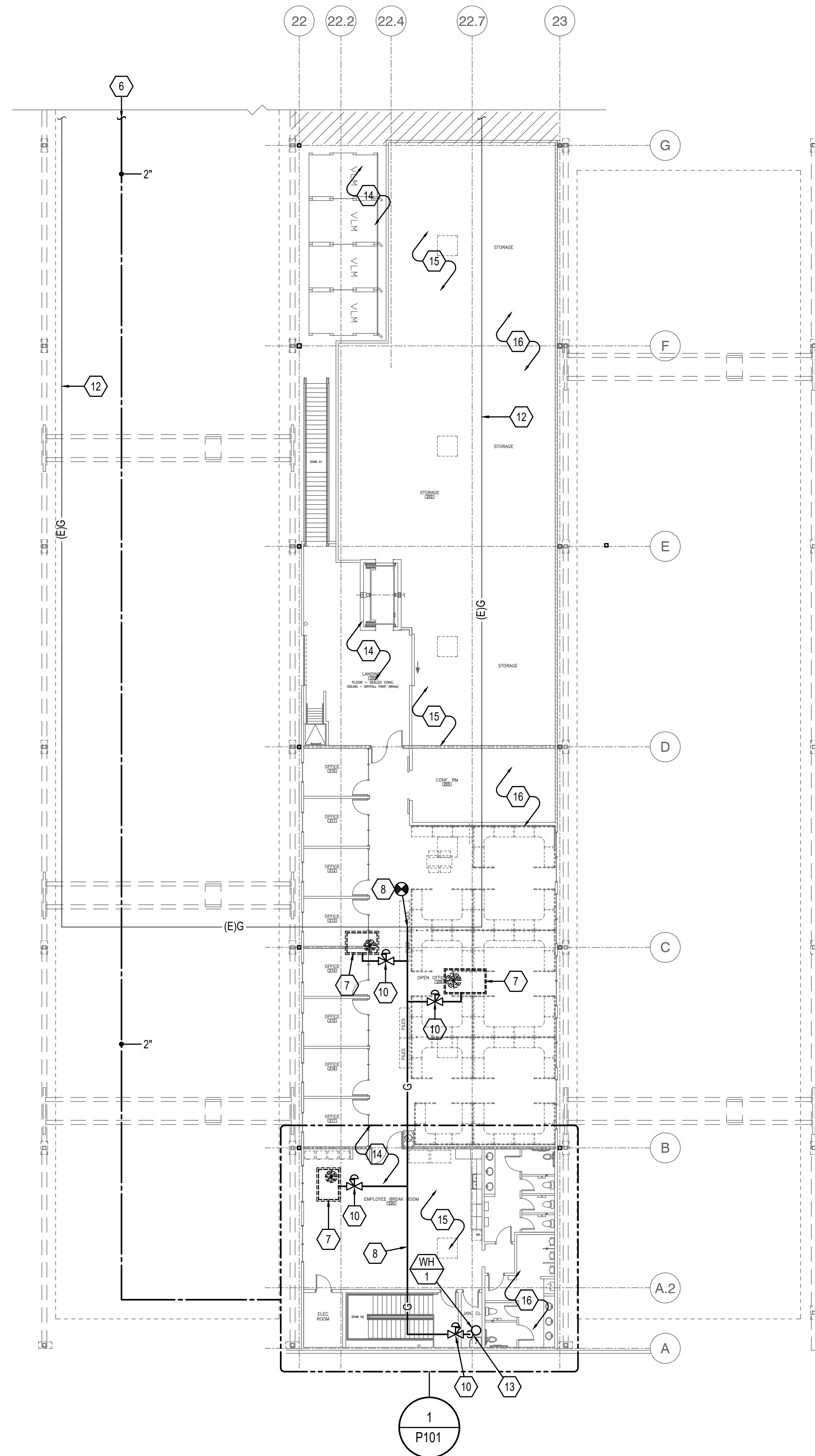
P101

KEYED NOTES

14. EXTEND EXISTING FIRE SPRINKLER SYSTEM TO SERVE BELOW NEW MEZZANINE. CONTRACTOR SHALL FIELD VERIFY NEAREST MAIN OF ADEQUATE SIZE AND PROVIDE HYDRAULIC CALCULATIONS. MATCH THE CURRENT WAREHOUSE HAZARD CLASSIFICATION.
15. FOR NEW MEZZANINE LEVEL, THE FIRE SPRINKLER CONTRACTOR SHALL FIELD VERIFY THE LOCATION OF THE EXISTING FIRE SPRINKLERS. ADD/REPOSITION EXISTING SPRINKLER LOCATION WITH A NEW SPRINKLER HEAD AS NECESSARY FOR THE REMODELED SPACE, INCLUDING NEW FLOOR PLAN CEILING PLAN AND CEILING HEIGHT ADJUSTMENTS. MODIFY SPRINKLER PIPING AS REQUIRED, TYPICAL. REFER TO THE ARCHITECTURAL SHEETS FOR COMPLETE SCOPE OF THE PROJECT. ALL SPRINKLERS IN THE REMODELED AREA SHALL BE REPLACED WITH QUICK RESPONSE TYPE. REPLACEMENT OF SPRINKLERS SHALL EXTEND TO ALL WALLS OR SOFFIT BREAKS.
16. FIRE SPRINKLERS SHALL BE INSTALLED TO MEET NFPA 13-2016 REQUIREMENTS, TYPICAL.

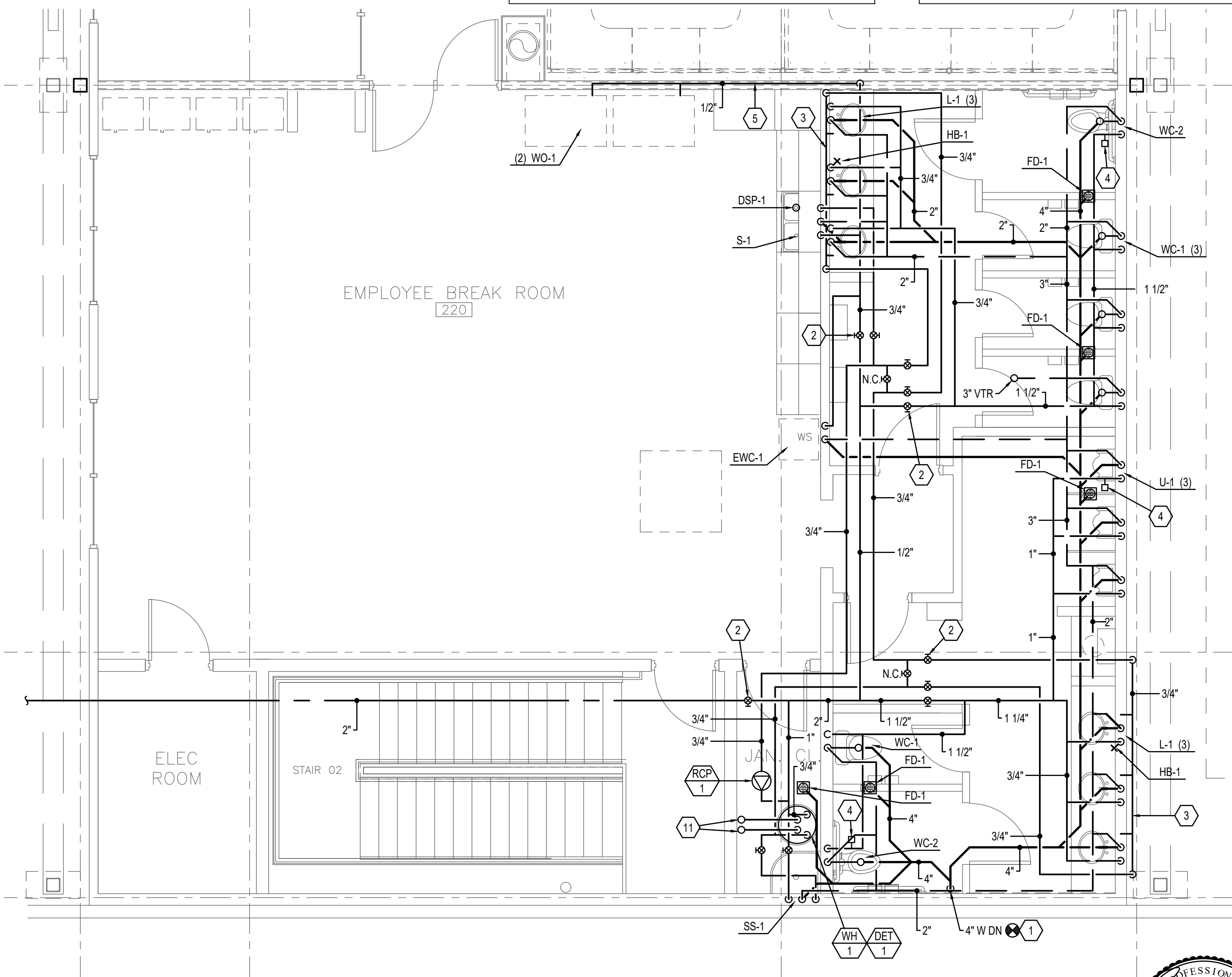
KEYED NOTES

1. EXTEND 4" WASTE PIPING DOWN TO LEVEL BELOW AND CONNECT TO EXISTING 4" WASTE PIPE STUB-OUT. FIELD VERIFY EXISTING CONDITIONS.
2. PROVIDE AND INSTALL ISOLATION VALVE IN ACCESSIBLE LOCATION. TYPICAL OF ALL.
3. EXTEND DHW MAIN LOOP INSIDE WALL WITHIN 18" OF LAVATORY HOT WATER SUPPLY CONNECTION.
4. PROVIDE AND INSTALL WATER HAMMER ARRESTOR IN ACCESSIBLE LOCATION.
5. EXTEND 1/2" COW LINE INSIDE WALL TO WATER OUTLETS SERVING REFRIGERATORS.
6. EXTEND NEW 2" COW MAIN LINE APPROXIMATELY 120 FEET AND CONNECT TO EXISTING MAINS. FIELD VERIFY EXISTING CONDITIONS.
7. MECHANICAL EQUIPMENT LOCATED ON ROOF. SEE MECHANICAL DRAWINGS.
8. NEW NATURAL GAS PIPING TO BE LOCATED ON ROOF. SEE GAS PIPE SCHEMATIC FOR GAS LOAD, PIPE SIZES, ETC.
10. PROVIDE 2 PSI TO 4 OZ. GAS PRESSURE REGULATOR TO BE LOCATED ON ROOF. PROVIDE GAS SHUT-OFF VALVE AND DIRT LEG AT POINT OF CONNECTION.
11. EXTEND WATER HEATER COMBUSTION AIR AND FLUE PIPES UP THROUGH ROOF AND TERMINATE WITH CONCENTRIC VENT FITTINGS. INSTALL PER MANUFACTURER'S REQUIREMENTS.
12. EXISTING GAS PIPING MAINS LOCATED ON ROOF. FIELD VERIFY EXISTING CONDITIONS.
13. DROP GAS PIPING TO MEZZANINE LEVEL AND CONNECT TO GAS FIRED WATER HEATER. PROVIDE GAS SHUT-OFF VALVE AND DIRT LEG AT POINT OF CONNECTION.



2 MEZZANINE LEVEL - PLUMBING FLOOR PLAN

SCALE: 1/16" = 1'-0"

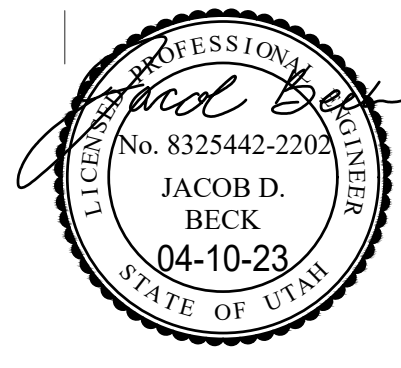


1 ENLARGED PLUMBING FLOOR PLAN

SCALE: 1/4" = 1'-0"

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Project Name
ALBANY MEZZANINE

Sheet Title
PLUMBING
DETAILS &
SCHEDULES

Scale
N/A

Date
04.10.2023

Drawn
GxA

Project No.
23020

Sheet No.

P501



GAS FIRED WATER HEATER SCHEDULE												
ID	MANUFACTURER AND MODEL NUMBER	LOCATION	SERVICE	INPUT LOAD (BTU/H)	EFFICIENCY (%)	TYPE	RECOVERY RATE @ 90 F DELTA T	TANK SIZE (GAL)	HEIGHT/ DIAMETER (IN)	ELECTRICAL		NOTES
										(AMPS)	VPH	
WH-1	AO SMITH BTX-80	SEE PLANS	DOMESTIC	76,000	96	GAS	96	50	72 / 22	5	120/1	1,2,3

- HIGH EFFICIENCY NATURAL GAS FIRED WATER HEATER.
- PROVIDE WITH CONDENSATE NEUTRALIZATION KIT.
- PROVIDE WITH DRAIN PAN.

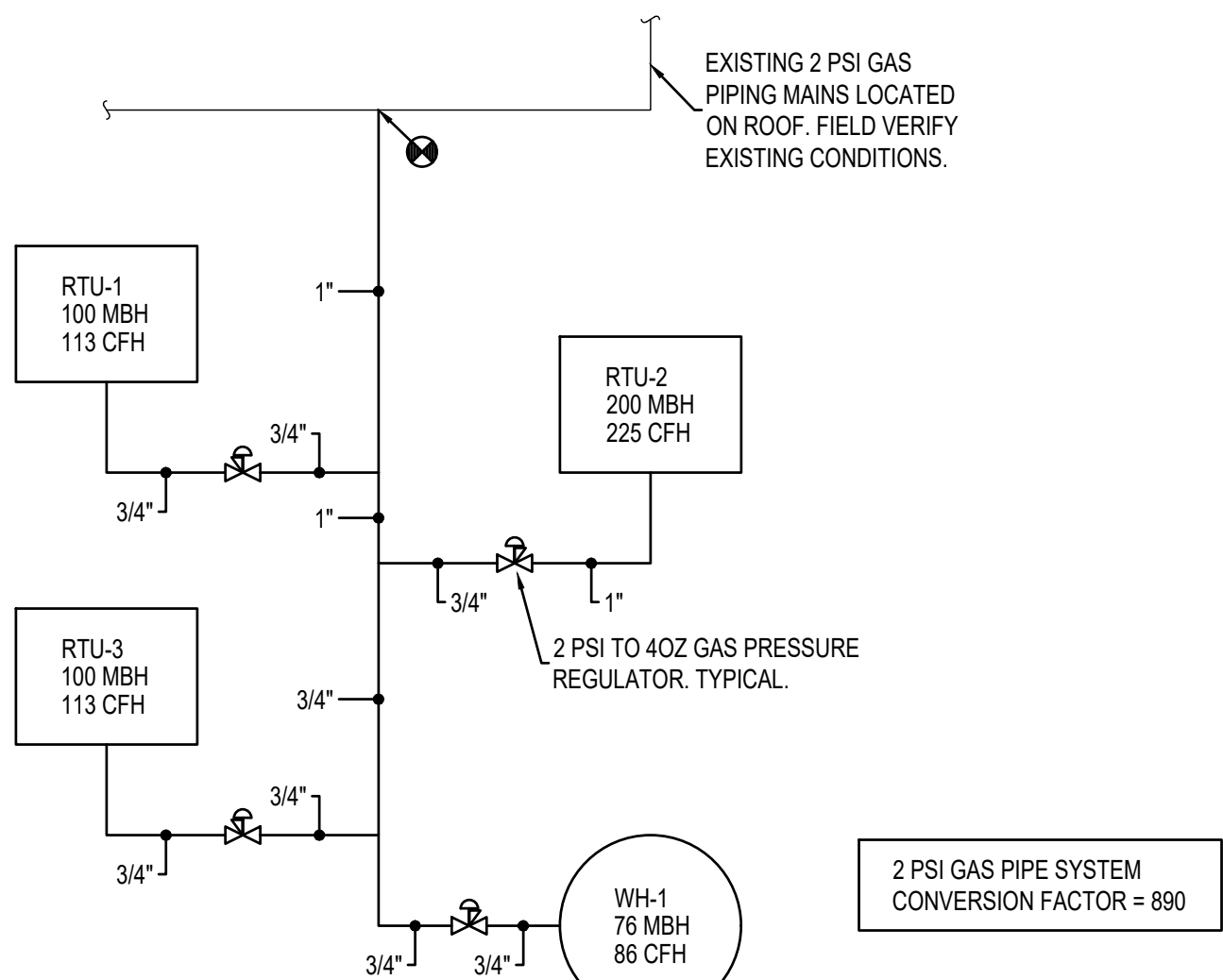
DOMESTIC PUMP SCHEDULE														
ID	MANUFACTURER AND MODEL NUMBER	LOCATION	TYPE	FLUID		PUMP			ELECTRICAL				NOTES	
				FLOW RATE (GPM)	WORKING FLUID	HEAD LOSS (FT)	EFFICIENCY (%)	CONSTRUCTION	MOTOR SIZE (HP)	MOTOR BHP (HP)	MOTOR SPEED (RPM)	VOLTPH/Hz		
RCP-1	B&G PL-36	SEE DRAWINGS	IN-LINE	2	WATER	26	N/A		ALL BRONZE	1/6	N/A	3300	115/1/60	--

EXPANSION TANK SCHEDULE												
ID	MANUFACTURER AND MODEL NUMBER	LOCATION	TYPE	FLUID		PHYSICAL					NPT FITTING (IN)	NOTES
				WORKING FLUID	MIN. TANK/ ACCEPTANCE (GAL)	TANK SIZE (GAL)	RELIEF VALVE (PSIG)	DIA/ HEIGHT (IN)				
DET-1	AMTROL ST-12C-DD	SEE DRAWINGS	DIAPHRAGM	WATER	3.2	6.4	150	12/18			3/4	1,2

- DIAPHRAGM: HEAVY DUTY BUTY NSF/ANSI #1.
- ANTIMICROBIAL POLYPROPYLENE LINER.

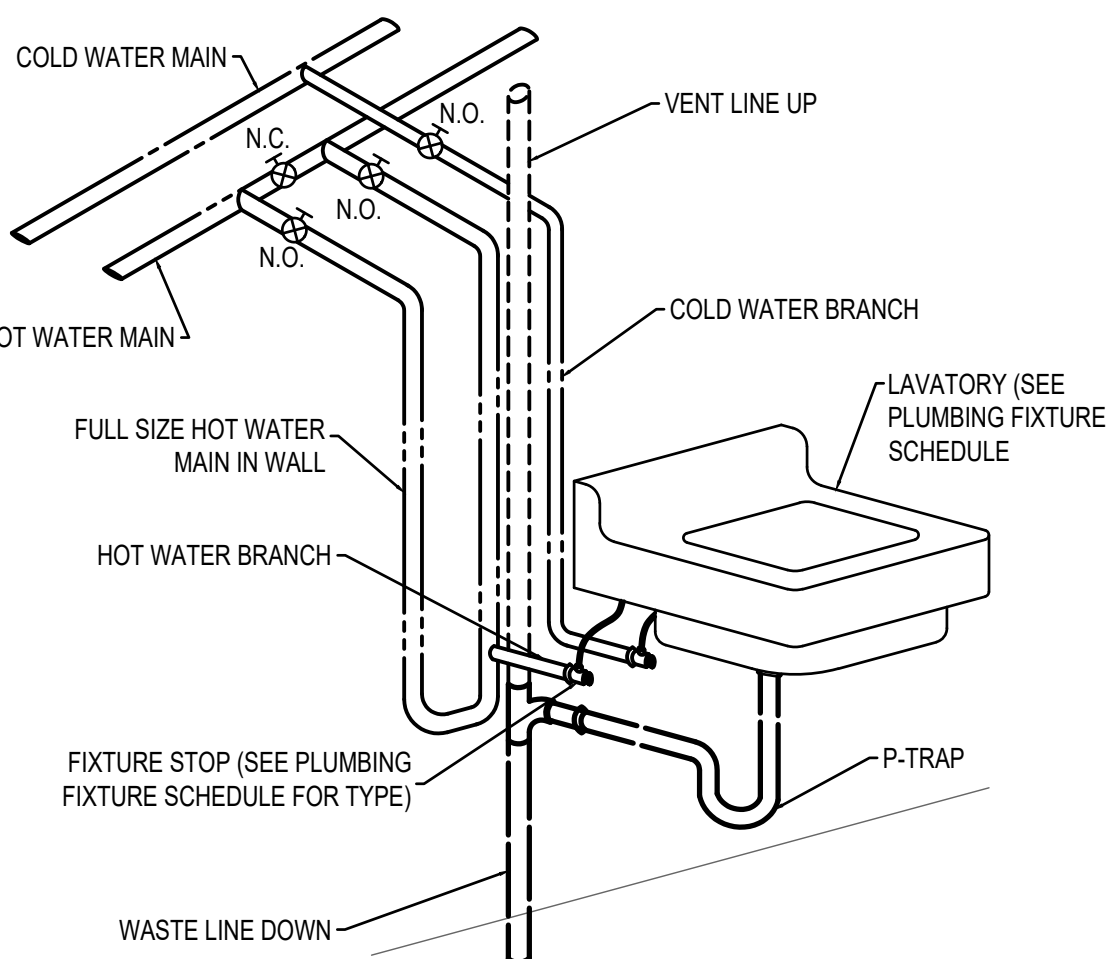
PLUMBING FIXTURE SCHEDULE												
ID	FIXTURE	CW (IN)	HW (IN)	W (IN)	V (IN)	NOTES	SPECIFICATION					
FD-1	FLOOR DRAIN	--	--	2	1 1/2	RESTROOM	FLOOR DRAIN (RESTROOM): SMITH FIGURE 2059Y FLOOR DRAIN WITH CAST IRON BODY AND FLASHING COLLAR WITH 6" SQUARE NICKEL BRONZE ADJUSTABLE STRAINER HEAD WITH SECURED GRATE. PROVIDE WITH TRAP GUARD.					
S-1	SINK	1/2	1/2	2	1 1/2	UNDERMOUNT, SINGLE COMPARTMENT	UNDERMOUNT SINK: KOHLER STRIVE K-5283 36" MINIMUM BASE CABINET WIDTH, SINGLE BOWL, 9" DEPTH, PREMIUM 16-GAUGE STAINLESS STEEL MATERIAL; KOHLER K-696 PULL-DOWN KITCHEN SINK FAUCET, POLISHED CHROME FINISH, SINGLE HANDLE AND HIGH-ARC SPOUT WITH 1.5 GPM FLOW; FLEXIBLE STAINLESS STEEL SUPPLIES WITH LOOSE KEY ANGLE STOPS. PROVIDE WITH STAINLESS STEEL CUP STRAINER AND CAST BRASS P-TRAP WITH CLEAN-OUT PLUG.					
DSP-1	GARBAGE DISPOSER	--	--	1 1/2	1 1/2		GARBAGE DISPOSER: INSINKERATOR BADGER 5 FOOD WASTE DISPOSER WITH 1/2 HORSEPOWER MOTOR, 120 VOLT/1 PHASE POWER CONNECTION, 6.9 AMPS, CONTROLLED BY WALL SWITCH.					
L-1	LAVATORY	1/2	1/2	1 1/2	1 1/2	UNDER-MOUNT	LAVATORY (UNDER-MOUNT): KOHLER K-2882-0 VERTICYL VITREOUS CHINA WITH OVERFLOW, K-7129 OPEN GRID STRAINER, SLOAN EAG-350 BATTERY OPERATED SENSOR FAUCET, 0.5 GPM FLOW RATE, INFRARED SENSOR, POLISHED CHROME FINISH. PROVIDE WITH WATTS LFUSG-8 UNDER SINK THERMOSTATIC MIXING VALVE TO MEET ASSE 1070; PROVIDE WITH CHECK VALVE ON HW AND CW INLETS. PROVIDE LOOSE KEY ANGLE STOPS AND CHROME PLATED COPPER SUPPLIES AND 17 GA. CAST BRASS, CHROME PLATED P-TRAP. COVER ALL EXPOSED PIPING WITH WHITE "HAND-LAY GUARD" PROTECTOR AS NEEDED TO MEET ADA REQUIREMENTS.					
WC-1	WATER CLOSET	3/4	--	4	2	FLOOR MOUNTED, FLUSH TANK	WATER CLOSET (FLOOR MOUNT, FLUSH TANK): KOHLER K-3575 "WELLWORTH" ELONGATED BOWL, 1.28 GPF TANK TYPE TOILET WITH TRIP LEVER ON WIDE SIDE OF FIXTURE; BEMIS 3155C "DURAGUARD" WHITE, SOLID PLASTIC, OPEN FRONT SEAT, LESS COVER; FLEXIBLE STAINLESS STEEL SUPPLY WITH LOOSE KEY ANGLE STOP.					
WC-2	WATER CLOSET	3/4	--	4	2	ADA, WALL MOUNTED, IN-WALL FLUSH TANK	WATER CLOSET (WALL MOUNT, IN-WALL FLUSH TANK, ADA): DURAVIT D-CODE WALL-MOUNTED TOILET #233509, WHITE ANTIBACTERIAL, CERAMIC GLAZE, 1.60 L GPF, ADA COMPLIANT, WD5008 ACTUATOR PLATE A1 FOR TOILET MECHANICAL ACTUATION FOR SURFACE MOUNTED INSTALLATION, DUAL FLUSH WITH MOUNTING FRAME AND HARDWARE INCLUDED. PROVIDE WITH WD1022 TOILET IN-WALL TANK & CARRIER. SEE ARCHITECTURAL DRAWINGS FOR MOUNTING HEIGHTS.					
U-1	URINAL	3/4	--	2	2	WALL HUNG, 1/8 GPF, SENSOR FLUSH VALVE	URINAL: KOHLER K-4991-ET VITREOUS CHINA ADA URINAL WITH 3/4" TOP SPUD, SLOAN 186 SMO-0, 125-DBP-OR 0.125 GPF EXPOSED SENSOR FLUSH VALVE. SMITH 0637 URINAL SUPPORT. SEE ARCHITECTURAL DRAWINGS FOR MOUNTING HEIGHTS.					
SS-1	SERVICE SINK	3/4	3/4	3	2	--	JANITOR SINK (FLOOR MOUNTED, CORNER): KOHLER K6710, WHITE/RY, 28 X 28 INCH, ENAMELED CAST IRON FLOOR-MOUNTED CORNER MODEL, K9146-3" DRAIN WITH STRAINER, NO. K8940 REMOVABLE VINYL-COATED RIM GUARD; CHICAGO 897 FAUCET WITH VACUUM BREAKER, SCREWDRIVER STOPS IN SHANKS, 5 FOOT RUBBER HOSE AND WALL HOOK, 853.					
HB-1	HOSE BIBB	3/4	--	--	--	--	HOSE BIBB: ACORN 8121CP-LP BENT NOSE HOSE VALVE POLISHED CHROME-PLATED WITH VACUUM BREAKER, 3/4" MALE HOSE THREAD AND LOOSE KEY HANDLE. COORDINATE EXACT NOSE CONFIGURATION WITH INSTALLATION.					
WO-1	WATER OUTLET	1/2	--	--	--	--	WATER OUTLET CONNECTION: WATER-TITE 82088 WASHING MACHINE OUTLET BOX WITH QUARTER TURN BALL VALVE.					
EW-1	ELECTRIC WATER COOLER	1/2	--	1 1/2	1 1/2	SINGLE STATION WITH BOTTLE FILLER STATION	ELECTRIC WATER COOLER: ELKAY MODEL L28WSLK ED200 BOTTLE FILLING STATION WITH SINGLE ADA COOLER, FILTERED REFRIGERATED WATER, WALL MOUNT WITH FLEXIBLE BUBBLER GUARD, STAINLESS STEEL BOWLS AND CONTROL BUTTONS ON FRONT. COMPRESSOR TO BE 115 V, 60 HZ WITH CAPACITY TO DELIVER AT LEAST 8.0 GPH OF 50°F FILTERED WATER, 1-1/2" CAST BRASS CHROME-PLATED P-TRAPS.					

1. ALL UNDER GROUND WASTE AND VENT SHALL BE 2" OR GREATER PER DRAWINGS.



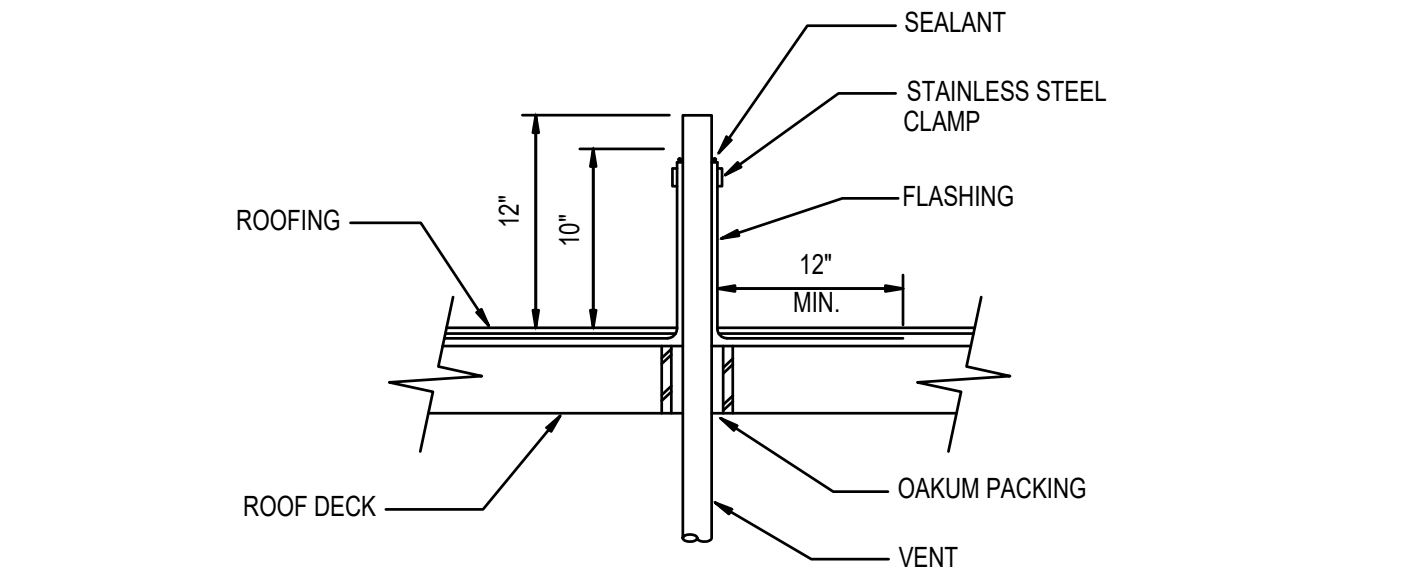
A7 GAS PIPING SCHEMATIC

P501 NO SCALE



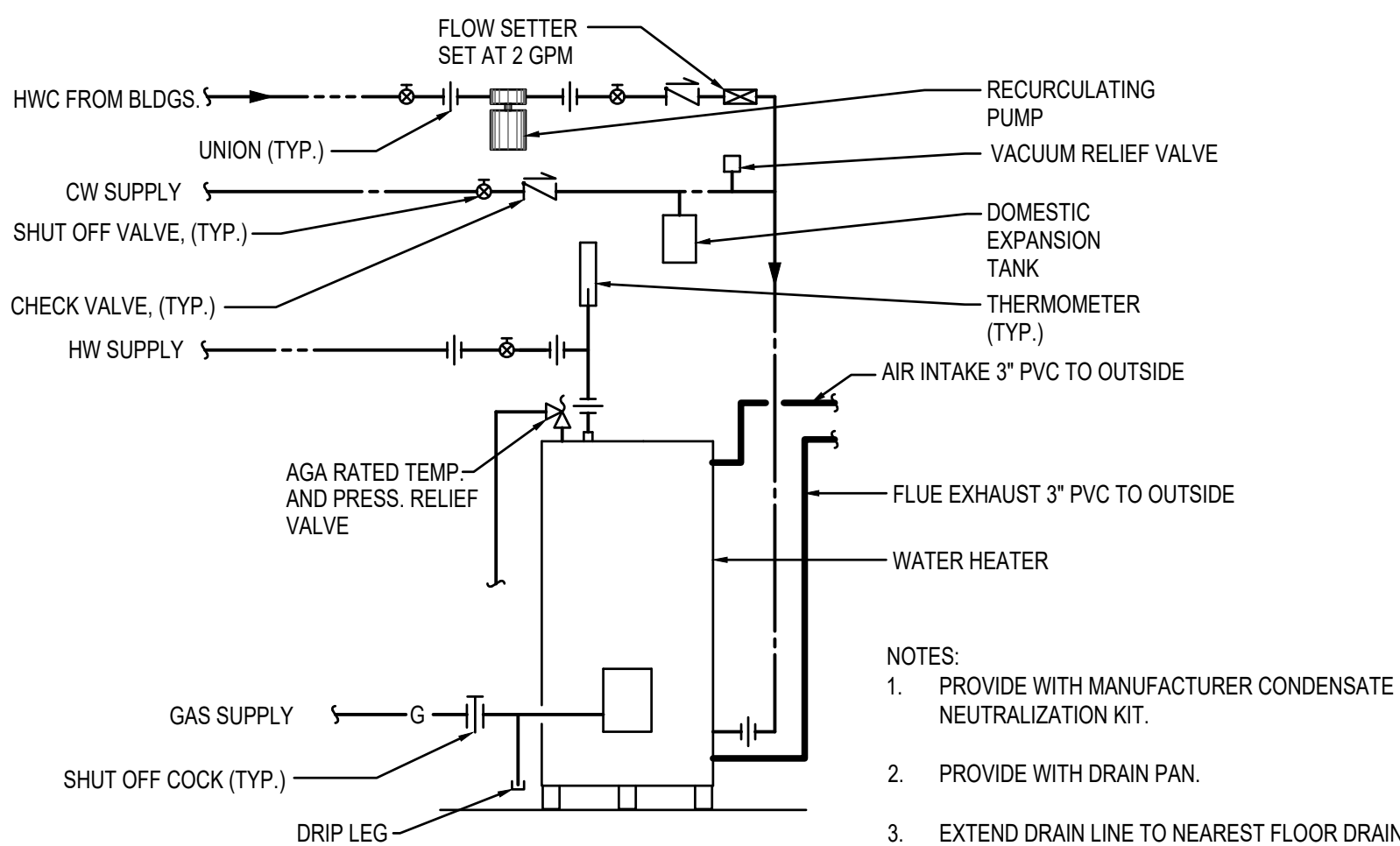
A5 LAVATORY PIPING DETAIL

P501 NO SCALE



B3 VENT THRU ROOF FLASHING & SLEEVING

P501 NO SCALE



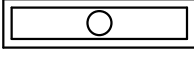




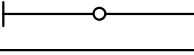
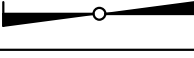
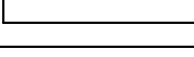

























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







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






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











































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ABBREVIATIONS	
ABBREV.	DESCRIPTION
A	AMP OR AMPS
AC	ABOVE COUNTER
AFF	ABOVE FINISHED FLOOR
AHJ	AUTHORITY HAVING JURISDICTION
AL	ALUMINUM
C	CONDUIT
CB	CIRCUIT BREAKER
CKT	CIRCUIT
CLG	CEILING
CORR	CORRIDOR
CU	COPPER
D	DRYER
DISP	DISPOSAL
DW	DISHWASHER
EM	EMERGENCY
EMT	ELECTRIC METALLIC TUBING
EWC	ELECTRIC WATER COOLER
E, EX	EXISTING
FA	FIRE ALARM
FACP	FIRE ALARM CONTROL PANEL
FLA	FULL LOAD AMPS
FMC	FLEXIBLE METAL CONDUIT
GND	GROUND CONDUCTOR
HP	HORSE POWER
IG	ISOLATED GROUND
IMC	INTERMEDIATE METAL CONDUIT
INS	INSULATED
ISO	ISOLATED
KVA	KILO VOLT AMPERES
KW	KILOWATTS
LFMC	LIQUID TIGHT METAL CONDUIT
LTG	LIGHTING
LVL	LEVEL
MCM	MINIMUM CIRCUIT AMPS
MCB	MAIN CIRCUIT BREAKER
MDU	MEDIA DISTRIBUTION UNIT
MLO	MAIN LUGS ONLY
MW	MICROWAVE
NIC	NOT IN CONTRACT
NL	NIGHT LIGHT
OC	ON CENTER(S)
OCP	OVER CURRENT PROTECTION
PFR	PHASE FAILURE RELAY
RCPT	RECEPTACLES
REQ	REQUIREMENTS
RELT	REDUCED ENERGY LET THROUGH DEVICE
RMC	RIGID METAL CONDUIT
RMP	ROCKY MOUNTAIN POWER
RNC	RIGID NONMETALLIC CONDUIT
SPD	SURGE PROTECTION DEVICE
SS	SURGE SUPPRESSION
TR	TAMPER RESISTANT
Typ	TYPICAL
TTB	TELEPHONE TERMINAL BOARD
UG	UNDERGROUND
W	WASHER
WP	WEATHERPROOF
XFMR	TRANSFORMER

LIGHTING SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	LED LIGHT FIXTURE
	LED LIGHT FIXTURE - EMERGENCY
	RECESSED LED DOWN LIGHT
	RECESSED LED DOWNLIGHT - EMERGENCY
	RECESSED LED WALL WASH OR SPOT FIXTURE
	LED STRIP LIGHT
	LED STRIP LIGHT - EMERGENCY
	LED LINEAR LIGHT
	LED LINEAR LIGHT - EMERGENCY
	SURFACE OR PENDANT MOUNTED LED LIGHT
	SURFACE OR PENDANT MOUNTED LED LIGHT - EMERGENCY
	RECESSED LED WAFER LIGHT
	RECESSED LED WAFER LIGHT - EMERGENCY
	WALL MOUNTED LED LIGHT FIXTURE
	WALL MOUNTED LED LIGHT FIXTURE - EMERGENCY
	LED TRACK LIGHT HEAD
	FAN
	LED WALL MOUNTED EXIT SIGN - SINGLE SIDED - ARROWS INDICATE DIRECTION
	LED EXIT SIGN - SINGLE SIDED - ARROWS INDICATE DIRECTION
	LED EXIT SIGN - DOUBLE SIDED
	LED EXIT SIGN WITH INTEGRAL EMERGENCY LIGHT
	LED EMERGENCY LIGHT WITH INTEGRAL BATTERY
	SINGLE-POLE SWITCH
	SWITCH - LOWER CASE LETTER INDICATES ZONE
	SWITCH - LOW VOLTAGE
	SWITCH - 3 WAY
	SWITCH - 4 WAY
	SWITCH - DIMMER
	OCCUPANCY SENSOR - CEILING MOUNTED
	OCCUPANCY SENSOR - WALL MOUNTED
	OCCUPANCY SENSOR - CEILING MOUNTED DUAL CIRCUIT
	PHOTOCELL
	MANUAL OVERRIDE SWITCH

CALLOUTS/NOTES LEGEND	
SYMBOL	DESCRIPTION
	MECHANICAL EQUIPMENT CALLOUT
	REVISION CALLOUT
	LIGHT FIXTURE CALLOUT, TOP = TYPE, MIDDLE = CKT #, BOTTOM = SWITCH
	OWNER PROVIDED EQUIPMENT CALLOUT
	KEYED NOTE
	WIRE CONDUIT - ALUMINUM
	WIRE CONDUIT - COPPER
	DETAIL CALLOUT
	ELEVATION CALLOUT

FIRE ALARM SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	FIRE ALARM CONTROL PANEL
	NAC PANEL
	FIRE/SMOKE DAMPER
	SMOKE DETECTOR WITH VISUAL - CEILING MOUNTED
	SMOKE DETECTOR WITH VISUAL - WALL MOUNTED
	SMOKE DETECTOR
	COMBINATION SMOKE/CARBON DETECTOR

POWER/DATA SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	SIMPLEX RECEPTACLE
	TWIST-LOCK RECEPTACLE
	DUPLEX RECEPTACLE
	DUPLEX RECEPTACLE - GFCI
	DROP DUPLEX RECEPTACLE - GFCI
	HALF-SWITCHED DUPLEX RECEPTACLE
	FLOOR BOX WITH DUPLEX 120V RECEPTACLE
	FLOOR BOX WITH 4-PLEX RECEPTACLE AND VOICE/DATA OUTLET
	FOURPLEX RECEPTACLE
	FOURPLEX RECEPTACLE - GFCI
	BLANK FACE - GFCI
	DROP FOURPLEX RECEPTACLE - GFCI
	SPECIAL PURPOSE RECEPTACLE - THREE PHASE
	SPECIAL PURPOSE RECEPTACLE - SINGLE PHASE
	NON-FUSED DISCONNECT SWITCH
	FUSED DISCONNECT SWITCH
	COMBINATION STARTER/FUSED DISCONNECT
	STARTER
	ELECTRICAL PANEL
	TELEPHONE TERMINAL BOARD W/GROUND BUSS BAR
	DOOR CONTACTOR
	ELECTRIC STRIKE
	EMERGENCY POWER OFF
	MAGNETIC LOCK
	PUSH BUTTON
	POWER PACK
	ROOM CONTROLLER
	REQUEST TO EXIT
	THERMAL SWITCH
	ELECTRIC MOTOR
	J-BOX
	J-BOX FOR DATA/VOICE
	METER
	VARIABLE FREQUENCY DRIVE
	TV OUTLET, REFER TO AV / DATA DRAWINGS FOR CABLE TYPE
	POWER COMMUNICATIONS POLE FOR WORKSTATION FURNITURE
	VOICE RECEPTACLE
	DATA RECEPTACLE
	COMBINATION VOICE/DATA RECEPTACLE
	CEILING MOUNT SECURITY CAMERA ("TYPE" INDICATES TYPE OF CAMERA)
	WALL MOUNT SECURITY CAMERA ("TYPE" INDICATES TYPE OF CAMERA)
	CARD READER
	SECURITY SYSTEM KEYPAD
	DOORBELL - EXTERIOR
	DOORBELL - INTERIOR
	ADA PUSHBUTTON
	HOME RUN TO PANELBOARD
	CIRCUIT BREAKER
	ELECTRONIC TRIP CIRCUIT BREAKER
	FUSE

SITE LIGHTING SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	POLE LIGHT
	POLE LIGHT - TWIN HEAD
	BOLLARD LIGHT

GENERAL NOTES	
1.	REFER TO THE MECHANICAL SHEETS FOR THE EXACT LOCATION OF THE MECHANICAL EQUIPMENT.
2.	ALL METALLIC CONDUITS, JOINTS, FITTINGS, ETC., IN CONTACT WITH THE GROUNDS SHALL BE SPIRALLY WRAPPED WITH 3M SCOTCHWRAP-51, 20 MIL TAPE (OR APPROVED EQUAL).
3.	ALL UNDERGROUND CONDUIT SHALL BE BURIED 24" MINIMUM UNDER THE GROUND.
4.	FLEXIBLE CONDUITS CAN ONLY BE USED FOR SHORT RUNS (6' MAXIMUM).
5.	NO CONDUITS SHALL RUN IN DUCT WORK.
6.	ALL DUPLEX OUTLETS AND SWITCHES SHALL BE 20A, 120V SPEC GRADE, HUBBELL AND PASS & SEYMOUR AND LEVITON ARE APPROVED MANUFACTURERS.
7.	INSTALL EXIT SIGNS ON THE WALL IF POSSIBLE.
8.	SWITCHBOARDS, PANELBOARDS, AND MOTOR CONTROL CENTERS SHALL BE FIELD MARKED TO WARN QUALIFIED PERSONS OF POTENTIAL ARC FLASH HAZARDS. THE MARKING SHALL BE LOCATED SO AS TO BE CLEARLY VISIBLE TO QUALIFIED PERSONS BEFORE EXAMINATION, ADJUSTMENT, SERVICING, OR MAINTENANCE OF THE EQUIPMENT. (NEC 110-16).
9.	FOR 20 AMP CIRCUITS, USE NO. 10 THHN CONDUCTORS FOR CONDUCTOR LENGTH OVER 100 FEET, NO. 8 THHN OVER 200 FEET, NO. 6 THHN OVER 300 FEET AND NO. 4 THHN OVER 400 FEET.
10.	IF CONDUITS ARE TO BE PLACED WITHIN THE PT SLAB, CONDUITS ARE TO BE SPACED NO CLOSER THAN 4 CONDUIT DIAMETERS OR 4" O.C, WHICHEVER IS GREATER. LIMIT CONDUIT OUTER DIAMETER TO ONE FOURTH OF THE SLAB THICKNESS AND PLACE WITHIN THE CENTER THIRD OF SLAB THICKNESS. CONDUIT IS NOT TO BE TIED TO PARALLEL REBAR AND/OR TENDONS, NOR SHOULD REINFORCEMENT BE MOVED, BENT OR CUT TO ACCOMODATE CONDUIT.
11.	AIC RATINGS OF ALL OVERCURRENT PROTECTIVE DEVICES SHALL BE EQUAL TO OR GREATER THAN THE AIC RATING SHOWN IN THE PLANS.
12.	SERVICE EQUIPMENT SHALL BE FIELD MARKED WITH CALCULATED MAXIMUM AVAILABLE FAULT CURRENT AND THE DATE IT WAS CALCULATED (NEC 110-24).
13.	IN OTHER THAN DWELLING UNITS, IN ADDITION TO THE REQUIREMENTS IN 1.10.16(A), A PERMANENT LABEL SHALL BE FIELD MARKED WITH CALCULATED MAXIMUM AVAILABLE FAULT CURRENT AND THE DATE IT WAS CALCULATED (NEC 110-24). (1) NOMINAL SYSTEM VOLTAGE. (2) AVAILABLE FAULT CURRENT AT THE SERVICE OVERCURRENT PROTECTIVE DEVICES. (3) THE CLEARING TIME OF SERVICE OVERCURRENT PROTECTIVE DEVICES BASED ON THE AVAILABLE FAULT CURRENT AT THE SERVICE EQUIPMENT. (4) THE DATE THE LABEL WAS APPLIED.

SEISMIC BRACING REQUIREMENTS	
HUNT ELECTRIC IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL REQUIRED SEISMIC BRACING BY THE 2018 INTERNATIONAL BUILDING CODE SECTION 1613 EARTHQUAKE LOADS PARAGRAPH 1613.1 SCOPE. RESTRAINT MUST BE PROVIDED FOR THE FOLLOWING CONDITIONS UNLESS OTHERWISE EXCLUDED BY CHAPTER 13 OF ASCE 7-10: (1) I _p > 1.0, (2) MEP COMPONENTS > 400 POUNDS AND SUPPORTED BY A FLOOR OR ROOF, (3) MEP COMPONENTS > 20 POUNDS AND SUPPORTED BY A CEILING OR WALL; OR (4) MEP DISTRIBUTION SYSTEMS WEIGHING > 5 PLF.	
HUNT ELECTRIC WILL PROVIDE A COMPLETE SUBMITTAL FOR ALL ELECTRICAL EQUIPMENT TO INCLUDE LOCATION OF EACH SEISMIC BRACE, TYPE AND DESIGN OF THE BRACING, AND A DETAIL OF THE SEISMIC BRACING. THE SUBMITTAL SHALL BE ON 36" X 24" SHEETS AND AT A SCALE OF 1/4" = 1'-0" ALONG WITH A COMPLETE SET OF CALCULATIONS.	
THE SUBMITTAL WILL CLEARLY INDICATE WHICH ITEMS ARE REQUIRED TO BE BRACED AND THE MINIMUM BRACING REQUIREMENTS (E.G. PER IBC 103.1 AND CHAPTER 13 OF ASCE 7-10). IN ADDITION THE SUBMITTAL SHALL BE PROVIDED BY A LICENSED PROFESSIONAL ENGINEER LICENSED IN UTAH.	

DEFERRED SUBMITTAL	
FIRE ALARM SHOP DRAWINGS, SUBMITTALS, BATTERY CALCULATIONS AND VOLTAGE DROP CALCULATIONS ARE A DEFERRED SUBMITTAL. THESE ITEMS SHAL BE SUBMITTED TO FIRE MARSHAL AND APPROVED BY FIRE MARSHAL PRIOR TO BEGINNING ANY WORK ON THE FIRE ALARM SYSTEM.	

ELECTRICAL SHEET INDEX	
SHEET	NAME
E001	NOTES, LEGENDS, SCHEDULES
E002	POWER SINGLE LINE DIAGRAM - SERVICE B CONTINUES PHASE 2
E003	ELECTRICAL DETAILS
E006	ELECTRICAL SCHEDULES
E007	ELECTRICAL SCHEDULES
E101	LEVEL 1 MEZZANINE ELECTRICAL POWER PLAN
E102	LEVEL 2 MEZZANINE ELECTRICAL POWER PLAN
E201	LEVEL 1 MEZZANINE ELECTRICAL LIGHTING PLAN
E202	LEVEL 2 MEZZANINE ELECTRICAL LIGHTING PLAN

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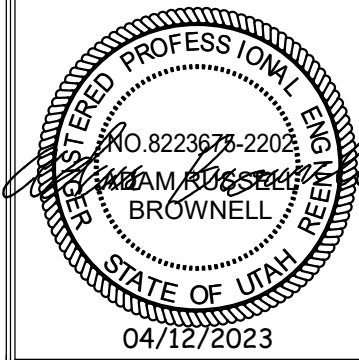
08/02/2023

1863 W ALEXANDER ST.
(2410 SO.)
SALT LAKE CITY, UTAH 84119
PHONE : 801-975-8844
FAX : 801-975-0509

HUNT
ELECTRIC, INC.

QUALITY, INTEGRITY, PERFORMANCE & VERSATILITY

DESIGN-BUILD SERVICES



DATE	
DESCRIPTION	
NO.	◀◀
PROJ. MGR.:	EL
DRAWN BY:	HE
ENGINEER:	AB

ALBANY MEZZANINE

5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

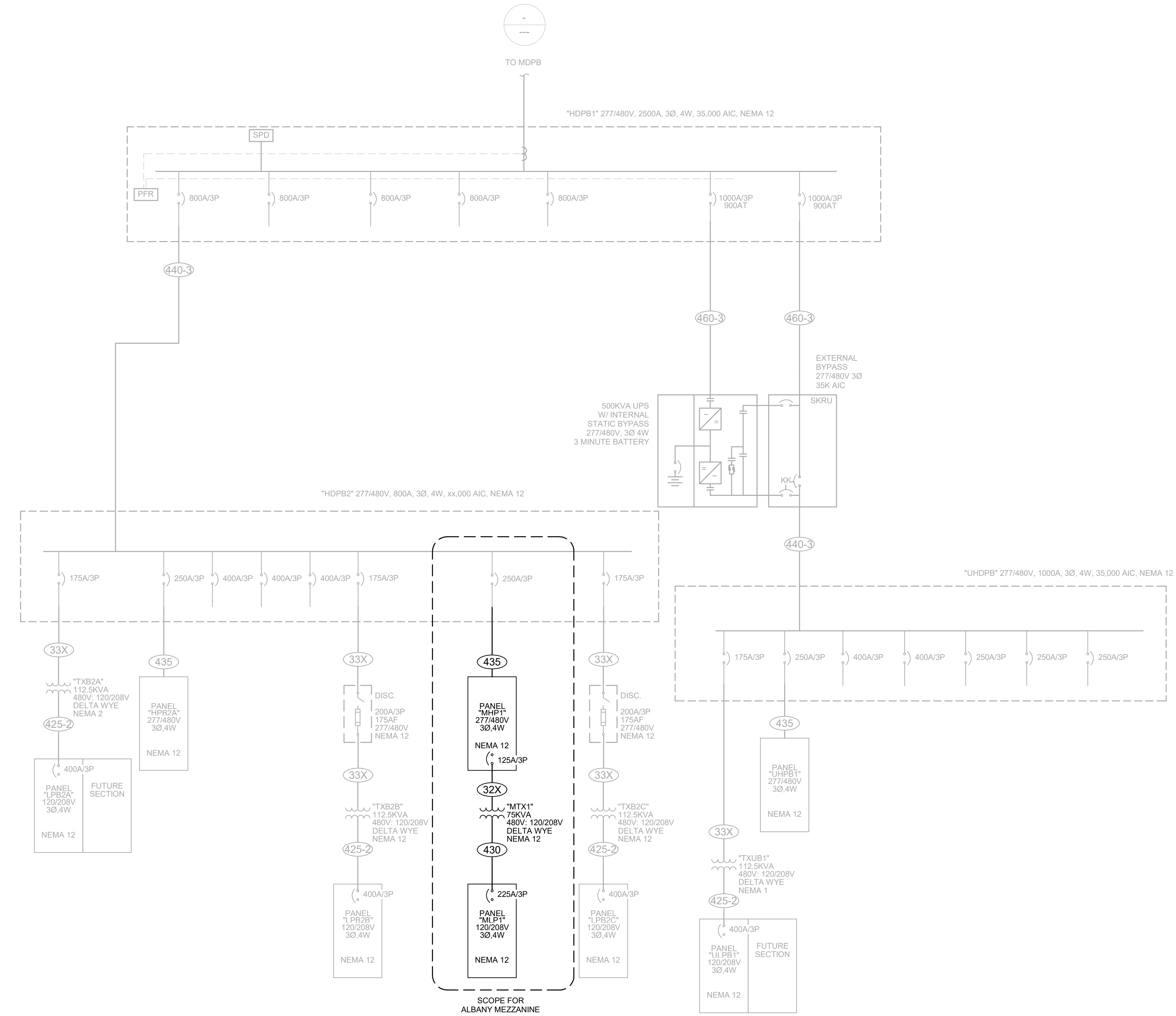
NOTES, LEGENDS, SCHEDULES

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E001



ALUMINUM FEEDER SCHEDULE										
TYPE	MAX PROT	CONDUCTOR AMPS	SETS	CONDUIT SIZE	CONDUCTOR QTY	CONDUCTOR SIZE	GND	PARALLEL	COPPER	ALUMINUM
31X		120		1-1/2"	3	1/0	4	N	N	Y
32X	-	135	-	1-1/2"	3	2/0	4	N	N	Y
33X	-	155	-	2"	3	3/0	4	N	N	Y
330-2	-	460	2	3"	3	300	1/0	Y	N	Y
425-2	400	410	2	3"	4	250	1	Y	N	Y
430	-	230	-	3"	4	300	2	N	N	Y
435	-	250	-	3"	4	350	2	N	N	Y
440-3	800	810	3	3"	4	400	3/0	Y	N	Y
460-3	1000	1020	3	4"	4	600	4/0	Y	N	Y
475-8	3000	3080	8	5"	4	750	600	Y	N	Y
475-11	4000	4235	11	5"	4	750	750	Y	N	Y

1 POWER SINGLE LINE DIAGRAM - SERVICE B CONTINUED PHASE 2
SCALE: NTS

+

DATE

DESCRIPTION

NO.

PROJ. MGR.:

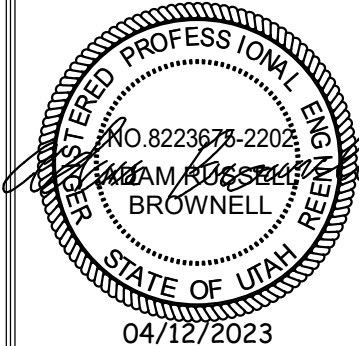
DRAWN BY:

ENGINEER:

1863 W ALEXANDER ST.
(2410 SO.)
SALT LAKE CITY, UTAH 84119
PHONE : 801-975-8844
FAX : 801-975-0595



DESIGN-BUILD SERVICES
QUALITY, INTEGRITY, PERFORMANCE & VERSATILITY



DATE

DESCRIPTION

NO.

PROJ. MGR.:

DRAWN BY:

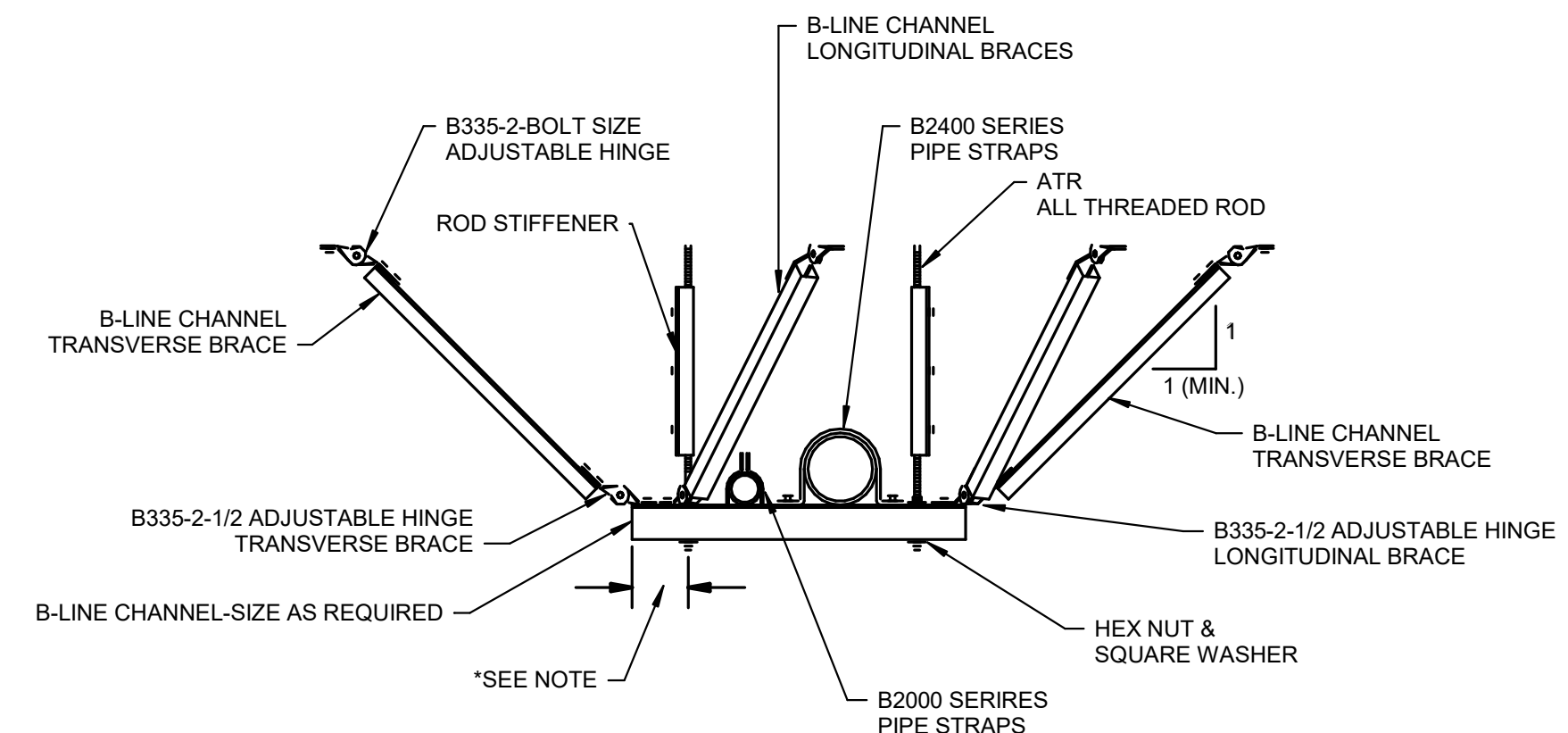
ENGINEER:

ALBANY MEZZANINE
5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
ELECTRICAL DETAILS
PERMIT SET

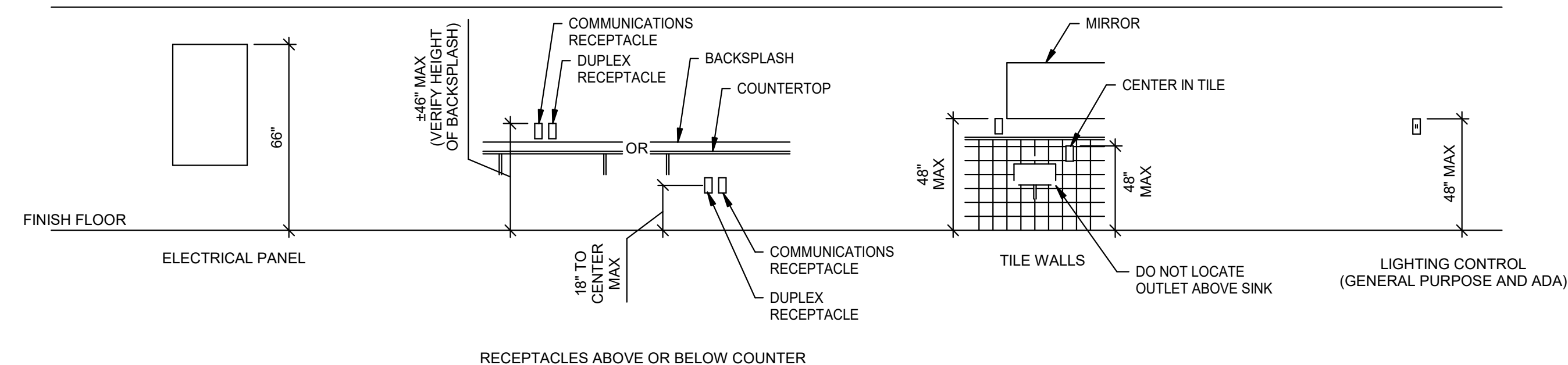
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SCALE:
NTS

E003

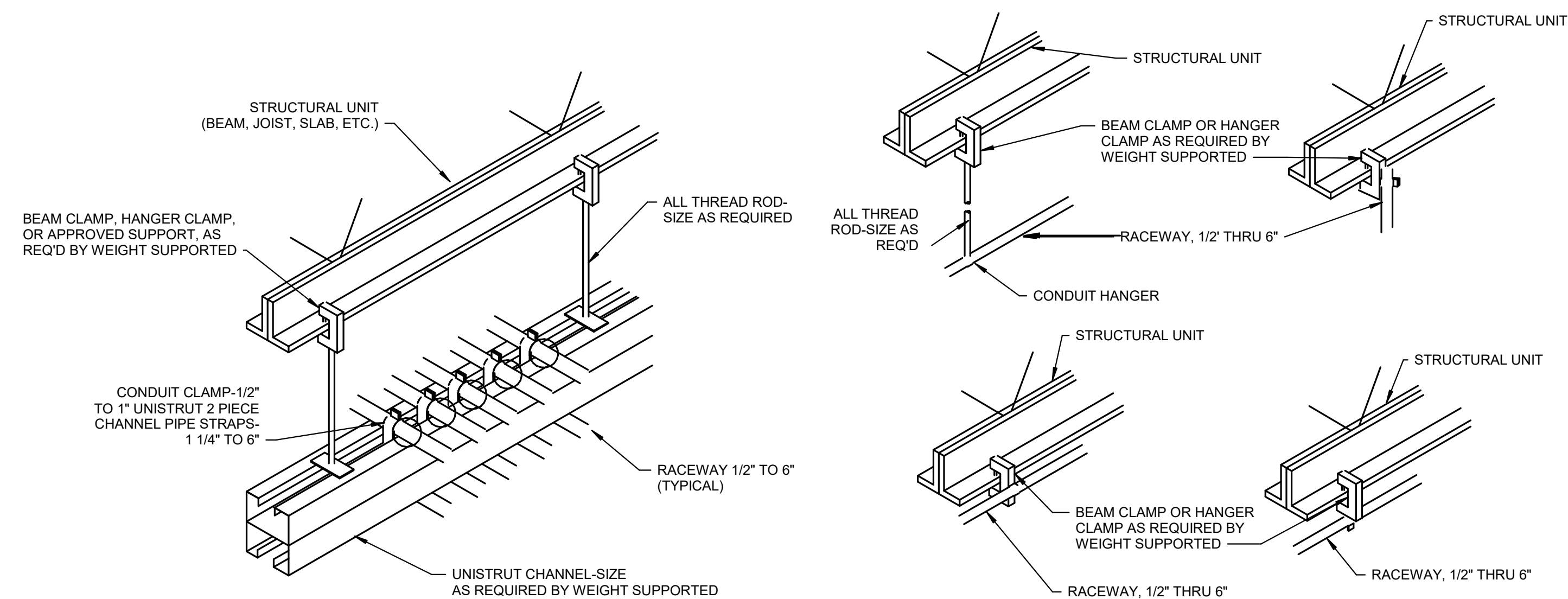


- NOTES:
- 1). B335-2 ADJUSTABLE HINGES FOR LONGITUDINAL BRACES MAY BE ATTACHED ON EITHER SIDE ADJACENT TO THE ALL THREAD ROD OR ATTACHED TO THE ALL THREAD ROD ITSELF.
 - 2). B335-2 ADJUSTABLE HINGES FOR TRANSVERSE BRACES MAY BE ATTACHED TO THE ALL THREAD ROD.
 - 3). TWO B335-W ADJUSTABLE HINGES MAY BE ATTACHED TO THE STRUT TRAPEZE USING THE SAME BOLT OR ALL THREAD ROD.
 - 4). IT IS NOT NECESSARY TO INSTALL BOTH TRANSVERSE BRACES AND LONGITUDINAL BRACES ON SAME TRAPEZE SUPPORT. EITHER SET OF BRACES MAY BE REMOVED TO FORM A LONGITUDINAL BRACE ONLY OR A TRANSVERSE BRACE ONLY IF DESIRED.
 - 5). LONGITUDINAL BRACES, WHEN NEEDED, MUST BE INSTALLED AT BOTH ENDS OF TRAPEZE.
 - 6). THE EQUIPMENT SHOWN ON THIS TRAPEZE SUPPORT IS GENERIC IN NATURE. ANY NUMBER OF PIPES, CONDUITS, DUCTWORK OR CABLE TRAY MAY BE SUPPORTED FOLLOWING THE SYSTEM WEIGHT AND SUPPORT SPANS LISTED IN APPENDIX 2 - TABLE 1.
- * DETERMINE LENGTH OF TRAPEZE, MAKING SURE SUFFICIENT LENGTH IS ADDED TO ATTACH THE ALL THREAD ROD AND BRACING ATTACHMENTS.



2 TYPICAL MOUNTING HEIGHTS
SCALE: NTS

1 TYPICAL TRAPEZE TRANSVERSE AND LONGITUDINAL BRACING DIAGRAM
SCALE: NTS



3 TYPICAL CONDUIT RACKING AND SUPPORT DIAGRAMS
SCALE: NTS

LIGHTING FIXTURE SCHEDULE - CLEANROOM					
TYPE	MANUFACTURER	CATALOG NUMBER	LAMP	TOTAL VA	DESCRIPTION
A1	LITHONIA LIGHTING	2BLT4 40L ADP EZ1 LP835	LED	34 VA	2'X4' RECESSED TROFFER LIGHT
C1	LITHONIA LIGHTING	LDN6 35/10 LO6AR LD MVOLT EZ10	LED	13 VA	6" RECESSED LED DOWNLIGHT
D1	LITHONIA LIGHTING OR EQUAL	CLX L48 5000LM SEF FDL MVOLT GZ10 35K 80CRI WH	LED	35 VA	4' LENSED LED STRIP LIGHT FIXTURE
ELV	TBD	TBD	LED	28 VA	ELEVATOR PIT LIGHT WITH EMERGENCY BATTERY BACKUP
EX1	LITHONIA LIGHTING	LQM S W 3 G 120/277 M6	LED	2 VA	SINGLE SIDED LED EXIT SIGN
G	LITHONIA LIGHTING	WL4 30L EZ1 LP850 MSD7	LED	28 VA	4' WALL BRACKET AND SURFACE MOUNT LED W/ EMERGENCY BATTERY PACK
HB1	GE CURRENT	ABV3 0 90 57 1D NA TQ 42 A D W	LED	158 VA	LED HIGH BAY LIGHT FIXTURE
HB1E	GE CURRENT	ABV3 0 90 57 1D NA TQ 42 A D W EL1	LED	158 VA	LED HIGH BAY LIGHT FIXTURE W/ EMERGENCY BATTERY PACK
P1	LITHONIA LIGHTING	LDN6CYL 35/20 LO6AR LD MVOLT EZ10 ACC DWHG	LED	23 VA	6" CYLINDER LED CEILING PENDANT
P1E	LITHONIA LIGHTING	LDN6CYL 35/20 LO6AR LD MVOLT EZ10 ACC DWHG	LED	23 VA	6" CYLINDER LED CEILING PENDANT W/ EMERGENCY BATTERY PACK
SL8	MARK ARCHITECTURAL LIGHTING	S4LD 8FT CRD 1H35 EZB 277 WHT	LED	104 VA	SLOT 8" LED LINEAR PENDANT
SL8E	MARK ARCHITECTURAL LIGHTING	S4LD 8FT CRD 1H35 EZB 277 WHT	LED	104 VA	SLOT 8" LED LINEAR PENDANT W/ EMERGENCY BATTERY PACK

MECHANICAL EQUIPMENT SCHEDULE CLEANROOM															
IDENTIFICATION	EQUIPMENT NAME	QUANTITY	RATING	PHASE	VOLTAGE	FLA/RLA	MCA	MOCP/ MFS	NON-FUSED DISC SIZE	FUSED DISC SIZE	RK-1 FUSE SIZE	VFD	WIRE SIZE AND QTY	GROUND WIRE SIZE	NOTES
CD-2	CEILING DIFFUSER	1	-	1	120 V	-	-	-	-	-	-	-	(2) #12	(1) #12	1
EF-1	CEILING DIFFUSER	1	1/3	1	120 V	-	-	-	-	-	-	-	(2) #12	(1) #12	1
EF-2	CEILING DIFFUSER	1	1/4	1	120 V	-	-	-	-	-	-	-	(2) #12	(1) #12	1
EF-3	CEILING DIFFUSER	1	-	1	120 V	-	-	-	-	-	-	-	(2) #12	(1) #12	1
ELEV-1	ELEVATOR	1	-	3	480 V	65	-	-	-	90A	-	-	(3) #2	(1) #8	-
RCP-1	RECIRC PUMP	1	1/6 HP	1	120 V	-	-	-	-	-	-	-	(2) #14	(1) #14	-
RTU-1	ROOFTOP UNIT	1	-	3	480 V	-	11.5	15	-	-	-	-	(3) #12	(1) #12	1
RTU-2	ROOFTOP UNIT	1	-	3	480 V	-	21.5	25	-	-	-	-	(3) #12	(1) #12	1
RTU-3	ROOFTOP UNIT	1	-	3	480 V	-	10.1	15	-	-	-	-	(3) #12	(1) #12	1
RTU-4	ROOFTOP UNIT	1	-	3	480 V	-	11.5	15	-	-	-	-	(3) #12	(1) #12	1
WH-1	WATER HEATER	1	-	1	120 V	-	-	-	-	-	-	-	(2) #12	(1) #12	1


- NOTES:
1. PROVIDE DISCONNECT SWITCH.
 2. DISCONNECT SWITCH SHALL HAVE BREAK-BEFORE-MAKE CONTACT. TIE TO VFD EMERGENCY STOP CIRCUIT TO STOP VFD BEFORE SWITCH IS PLACED IN THE OPEN POSITION.
 3. PROVIDE THERMAL OVERLOAD SWITCH FOR DISCONNECTING MEANS.
 4. PROVIDE COMBO STARTER/DISCONNECT WITH H.O.A. SWITCH AND (2) N.O. AND (2) N.C. CONTACTS.
 5. TIE TO 120 VOLT POWER THROUGH FIRE ALARM RELAY. TIE RELAY TO FIRE ALARM CONTROL PANEL FOR CLOSURE OF DAMPER UPON ALARM.
 6. CHILLER A SINGLE POINT CONNECTION.
 7. PROVIDE 120V/20A/1P TOGGLE SWITCH DISCONNECT ADJACENT TO UNIT.
 8. PROVIDE DUCT SMOKE DETECTOR IN RETURN AIR DUCT. TIE AIR HANDLER CONTROLS TO FIRE ALARM SYSTEM FOR SHUT DOWN OF UNIT UPON ALARM.
 9. DISCONNECT CONVENIENCE OUTLET PROVIDED WITH UNIT.
 10. CONTROLLED VIA WALL SWITCH.
 11. INTEGRAL DISCONNECT. DIRECT CONNECT TO UNIT.
 12. RUN POWER TO CU-xx FIRST, THEN FROM CU-xx TO FC-xx. VERIFY WITH SUBMITTALS.
 13. CORD AND PLUG CONNECTED.
 14. PROVIDE DUCT DETECTOR.
 15. VFD PROVIDED BY MECHANICAL INSTALL BY ELECTRICAL.

PANEL: MLP1													
LOCATION: SUPPLY FROM: MTX1 MOUNTTING: SURFACE ENCLOSURE: TYPE 1				VOLTS: 120/208 WYE PHASES: 3 WIRES: 4				A.I.C. RATING: MAINS TYPE: MCB MAINS RATING: 225 A MCB RATING: 225 A SUB-FEED LUGS					
CKT	CIRCUIT DESCRIPTION	TRIP	POLES	A		B		C		POLES	TRIP	CIRCUIT DESCRIPTION	CKT
1	MEZ LEVEL 2 OFFICES 210, 211, 212, 213	20 A	1	1440...	800 VA					1	20 A	ELEVATOR PIT SUMP PUMP	2
3	MEZ LEVEL 2 OFFICES 214, 215, 216, 217	20 A	1			1260...	180 VA			1	20 A	ELEVATOR PIT RECEPTACLE	4
5	MEZ LEVEL 2 CONF. ROOM RECEPS	20 A	1					1080...	180 VA	1	20 A	ELEVATOR PIT RECEPTACLE	6
7	MEZ LEVEL 2 BREAKROOM RECEPS	20 A	1	720 VA	180 VA					2	20 A	MEZ LEVEL 2 WORKSTATION FURNITURE	8
9	MEZ LEVEL 2 BREAKROOM FRIDGE	20 A	1			800 VA	0 VA			--	--		10
11	MEZ LEVEL 2 BREAKROOM FRIDGE	20 A	1					800 VA	180 VA	2	20 A	MEZ LEVEL 2 WORKSTATION FURNITURE	12
13	MEZ LEVEL 2 STORAGE CONV. RECEPS	20 A	1	720 VA	0 VA					--	--		14
15	MEZ LEVEL 2 BATHROOM RECEPS	20 A	1			1540...	360 VA			1	20 A	MEZ LEVEL 2 WORKSTATION FURNITURE	16
17	MEZ LEVEL 2 ELECTRIC WATER COOLER	20 A	1					500 VA	360 VA	1	20 A	OPEN OFFICE AREA RECEPS.	18
19	MEZ LEVEL 2 BREAKROOM MICROWAVE	20 A	1	1800...	186 VA					1	20 A	EF-1	20
21	MEZ LEVEL 2 BREAKROOM MICROWAVE	20 A	1			1800...	186 VA			1	20 A	EF-2	22
23	MEZ LEVEL 2 BREAKROOM MICROWAVE	20 A	1					1800...	366 VA	1	20 A	EF-3	24
25	MEZ LEVEL 2 BREAKROOM MICROWAVE	20 A	1	1800...	500 VA					1	20 A	CD-2	26
27	MEZ LEVEL 2 BREAKROOM MICROWAVE	20 A	1			1800...	119 VA			1	20 A	RCP-1	28
29	MEZ LEVEL 2 BREAKROOM MICROWAVE	20 A	1					1800...	180 VA	1	20 A	WH-1	30
31	MEZ LEVEL 2 BREAKROOM COUNTER RECEPS	20 A	1	540 VA	0 VA					1	20 A	SPARE	32
33	MEZ LEVEL 2 BREAKROOM VENDING	20 A	1			360 VA	0 VA			1	20 A	SPARE	34
35	MEZ LEVEL 2 BREAKROOM VENDING	20 A	1					360 VA	0 VA	1	20 A	SPARE	36
37	MEZ LEVEL 2 GARBAGE DISPOSAL	20 A	1	800 VA	0 VA					1	20 A	SPARE	38
39	ELEVATOR CAB LIGHTS	20 A	1			0 VA	0 VA			1	20 A	SPARE	40
41	SPARE	20 A	1					0 VA	0 VA	1	20 A	SPARE	42
TOTAL LOAD:				9486 VA		8405 VA		7606 VA					
TOTAL AMPS:				80 A		71 A		63 A					
LOAD CLASSIFICATION				CONNECTED LOAD		DEMAND FACTOR		ESTIMATED DEMAND		PANEL TOTALS			
RECEPTACLE				23140 VA		71.61%		16570 VA					
LIGHTING				0 VA		0.00%		0 VA					
										TOTAL CONN. LOAD: 25497 VA			
										TOTAL EST. DEMAND: 18927 VA			
										TOTAL CONN.: 71 A			
										TOTAL EST. DEMAND: 53 A			
NOTES:													

PANEL: MHP1											
LOCATION: SUPPLY FROM: HDPB2 MOUNTING: SURFACE ENCLOSURE: TYPE 1				VOLTS: 480/277 WYE PHASES: 3 WIRES: 4				A.I.C. RATING: MAINS TYPE: MCB MAINS RATING: 300 A MCB RATING: 125 A			
CKT	CIRCUIT DESCRIPTION	TRIP	POLES	A	B	C	POLES	TRIP	CIRCUIT DESCRIPTION	CKT	
1	MTX1	125 A	3	9486...						2	
3	--	--	--		8405...					4	
5	--	--	--			7606...				6	
7	MEZ LEVEL 2 LIGHTING	20 A	1	728 VA						8	
9	MEZ LEVEL 2 LIGHTING	20 A	1		1596...					10	
11	MEZ LEVEL 2 LIGHTING	20 A	1			763 VA				12	
13	ELEVATOR PIT LIGHT	20 A	1	28 VA						14	
15	ELEVATOR PIT LIGHT	20 A	1		28 VA					16	
17	MEZ LEVEL 2 LOBBY LIGHTING	20 A	1			3743...				18	
19	RTU-1	20 A	3	2549...						20	
21	--	--	--		2549...					22	
23	--	--	--			2549...				24	
25	RTU-2	20 A	3	4764...						26	
27	--	--	--		4764...					28	
29	--	--	--			4764...				30	
31	RTU-3	20 A	3	2238...						32	
33	--	--	--		2238...					34	
35	--	--	--			2238...				36	
37	RTU-4	20 A	3	2549...						38	
39	--	--	--		2549...					40	
41	--	--	--			2549...				42	
TOTAL LOAD:				22343 VA	22130 VA	24213 VA					
TOTAL AMPS:				81 A	80 A	88 A					
LOAD CLASSIFICATION				CONNECTED LOAD	DEMAND FACTOR	ESTIMATED DEMAND	PANEL TOTALS				
RECEPTACLE				23140 VA	71.61%	16570 VA	TOTAL CONN. LOAD: 68686 VA				
LIGHTING				4494 VA	125.00%	5618 VA	TOTAL EST. DEMAND: 63240 VA				
							TOTAL CONN.: 83 A				
							TOTAL EST. DEMAND: 76 A				
NOTES:											

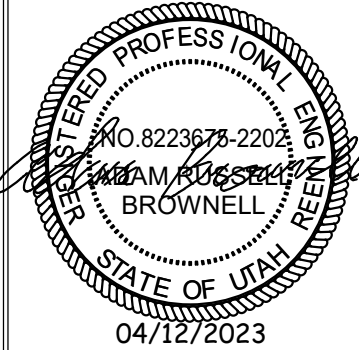
NEC DEMAND CALCULATION CLEANROOM														
Panel Name	RECEPTACLE Connected	RECEPTACLE Demand Factor	Power Connected	Power Demand Factor	Motor Connected	Motor Demand Factor	Heating Connected	Heating Demand Factor	HVAC Connected	HVAC Demand Factor	Total Connected	Total Demand Factor	Total Estimated Demand	Total Estimated Demand Current
HDPB3	7740 VA	100.00%	500 VA	100.00%	4500 VA	108.33%			1500 VA	100.00%	134849 VA	101.07%	136291 VA	164 A
LPB3A	7740 VA	100.00%									7740 VA	100.00%	7740 VA	21 A
TXB3A	7740 VA	100.00%									7740 VA	100.00%	7740 VA	21 A

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(2410 SO.)
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FAX 801-975-0509



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DESIGN-BUILD SERVICES



NO.	◀▶
PROJ. MGR.:	
DRAWN BY:	HE
ENGINEER:	
DESCRIPTION	
DATE	

ALBANY MEZZANINE

5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

ELECTRICAL SCHEDULES

PERMIT SET

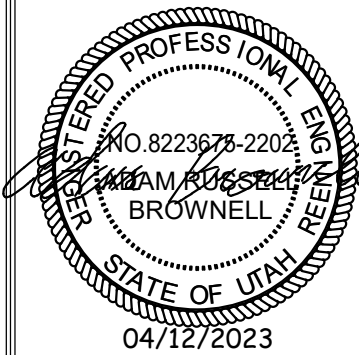
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DESIGN-BUILD SERVICES



DATE

DESCRIPTION

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PROJ. MGR.:

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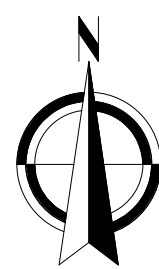
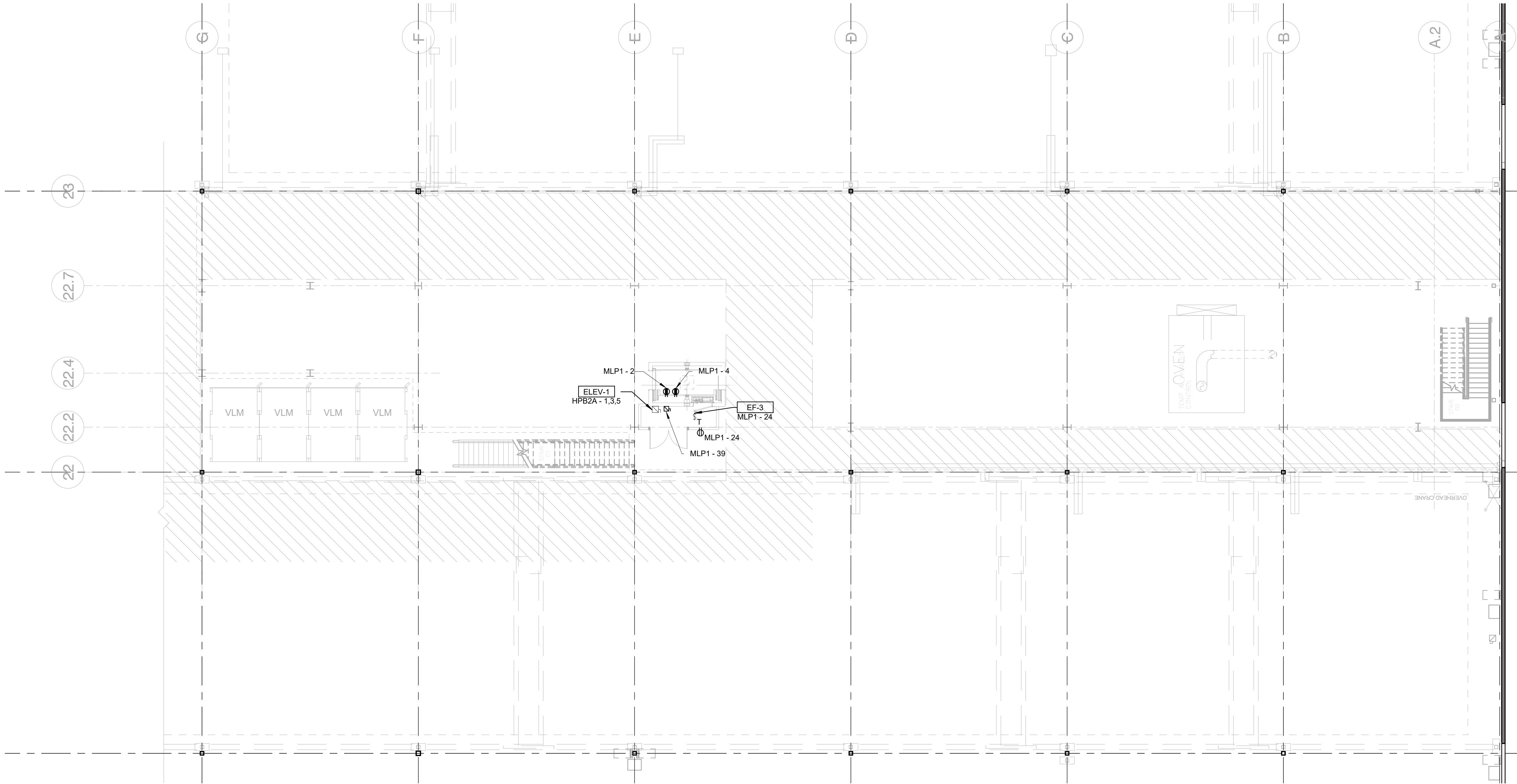
ENGINEER:

ALBANY MEZZANINE
5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
LEVEL 1 MEZZANINE ELECTRICAL POWER PLAN
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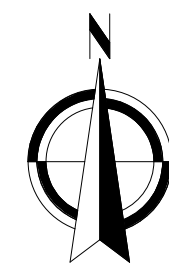
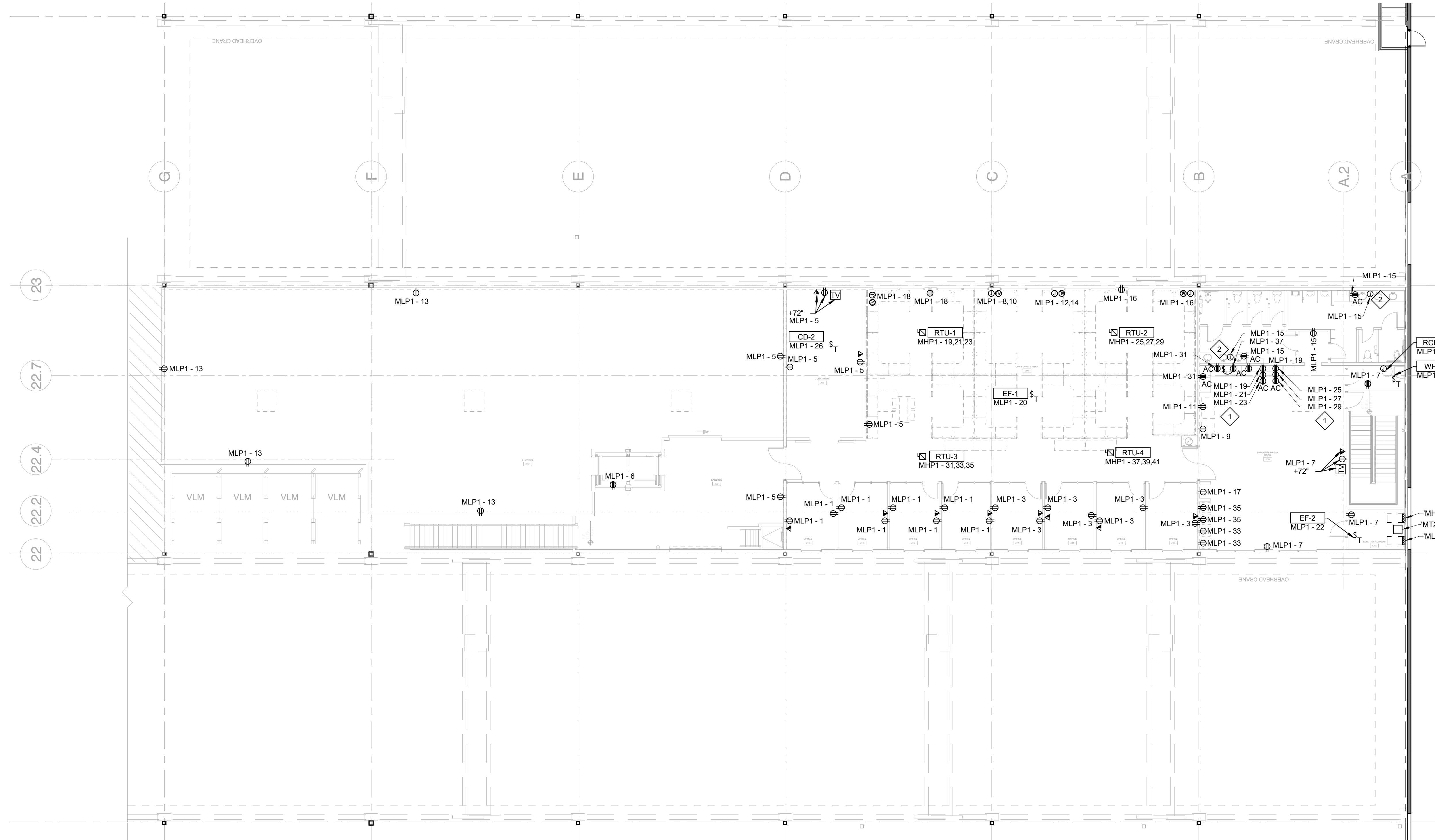


KEYED NOTES



2 LEVEL 1 MEZZANINE ELECTRICAL POWER PLAN

SCALE: 3/32" = 1'-0"



1

LEVEL 2 MEZZANINE ELECTRICAL POWER PLAN

SCALE: 3/32" = 1'-0"

#

KEYED NOTES

- COORDINATE LOCATION OF BREAKROOM MICROWAVES WITH ARCHITECT/OWNER PRIOR TO INSTALLATION.
- POWER FOR FUTURE BATTERY POWERED FAUCET.

DATE

DESCRIPTION

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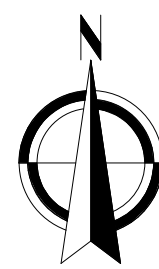
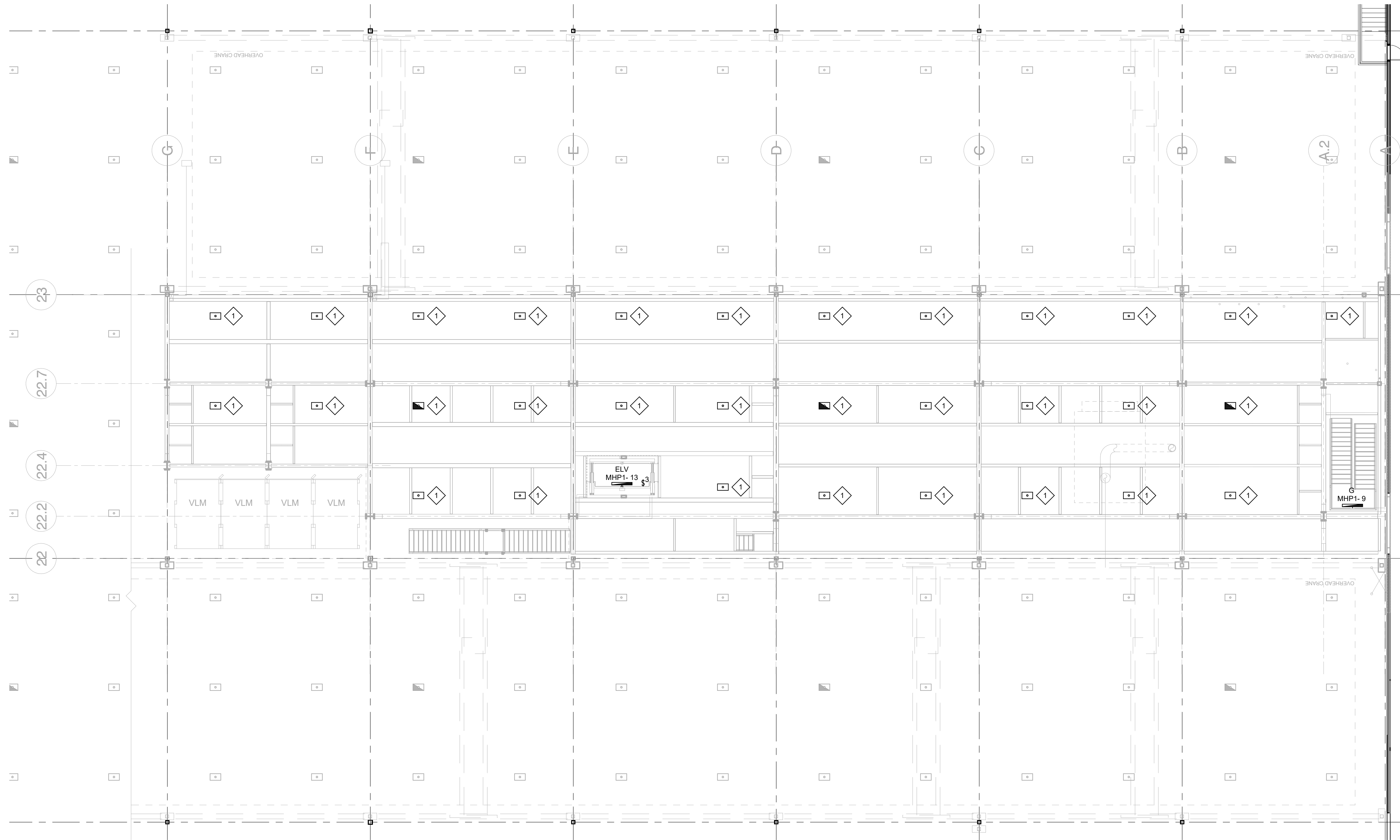
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ENGINEER:

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5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
LEVEL 2 MEZZANINE ELECTRICAL POWER PLAN
PERMIT SET

PRINTED DATE:
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SCALE:
3/32" = 1'-0"



1

LEVEL 1 MEZZANINE ELECTRICAL LIGHTING PLAN

SCALE: 3/32" = 1'-0"

#

KEYED NOTES

1

EXISTING HIGHBAY FIXTURE TO BE SALVAGED AND REUSED FROM DEMOLITION, TIE INTO NEAREST AVAILABLE 277V LIGHTING CIRCUIT ROUTED THROUGH LIGHTING CONTROL PANEL.

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DATE

DESCRIPTION

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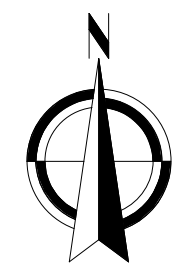
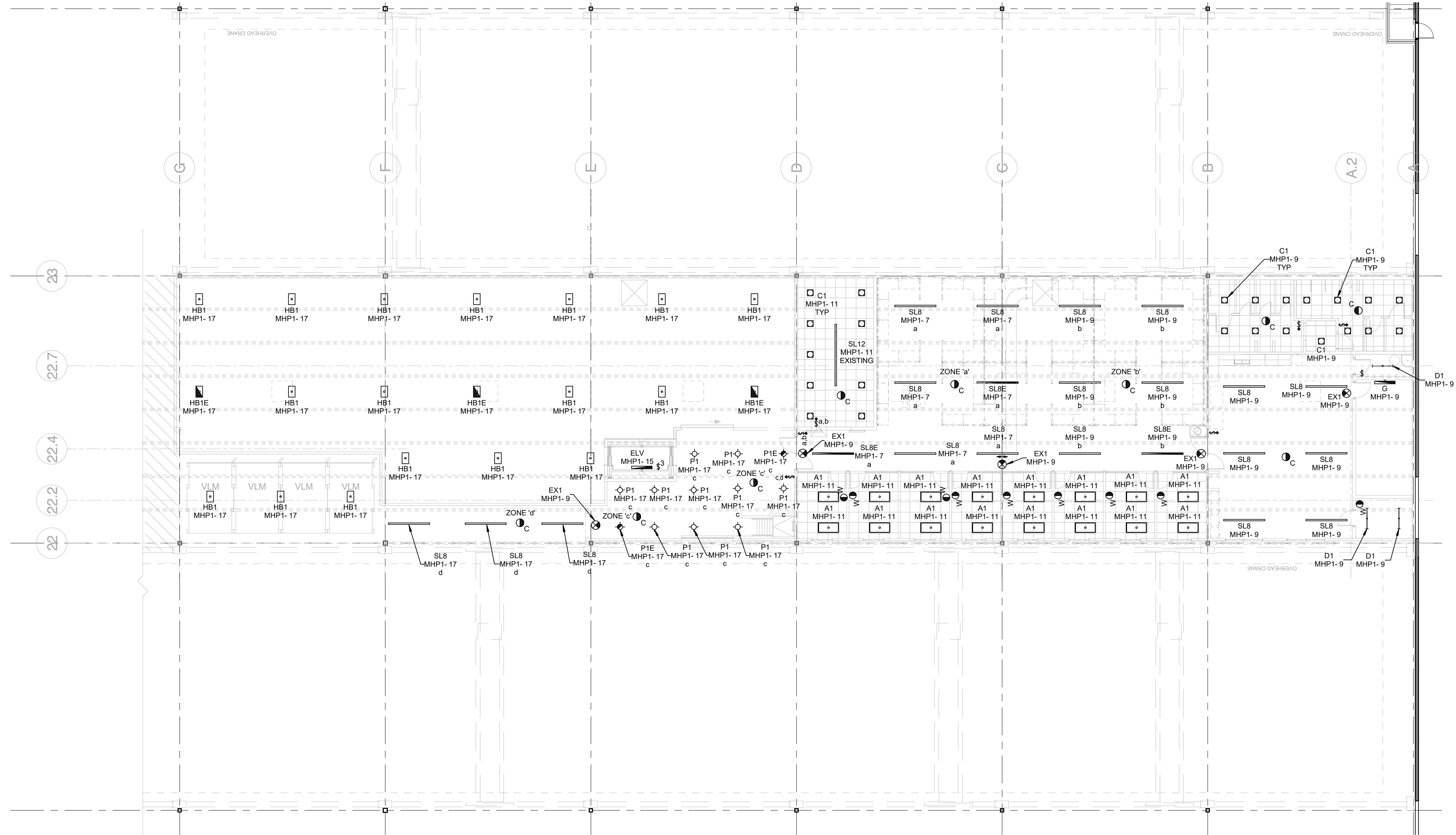
PROJ. MGR.:

DRAWN BY:

ENGINEER:

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5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
LEVEL 1 MEZZANINE ELECTRICAL LIGHTING PLAN
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1 LEVEL 2 MEZZANINE ELECTRICAL LIGHTING PLAN
SCALE: 3/32" = 1'-0"



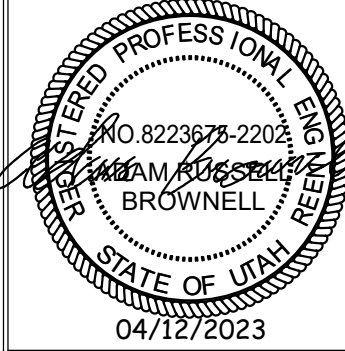
KEYED NOTES

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DESCRIPTION

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ENGINEER:

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5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
LEVEL 2 MEZZANINE ELECTRICAL LIGHTING PLAN
PERMIT SET

PRINTED DATE:
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SCALE:
3/32" = 1'-0"

E202