



NORTH BUILDING ADDITION - NEW CONSTRUCTION

5995 W. AMELIA EARHART DRIVE,
SALT LAKE CITY UTAH, 84116

Architect: Geroux Architects, PLLC
Contact: Joe Geroux, NCARB
917.686.6517

Owner Contact:

FEBRUARY 6, 2024

PRICING SET

ZONING: Salt Lake City Zoning Ordinance (Title 21A)

Zone: Salt Lake City M-1
Use: Warehouse
Lot Size: 23.49 Acres
Allowable Building Height: Maximum Structure Height 85'-0" (Not Applicable)
Actual Building Height: 32'-6"
Minimum Yard Requirements: Front & Corner Side 15'; Interior Side & Rear None

SUBCONSULTANTS:

Structural Engineer:
Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Mechanical Engineering:
Van Boerum & Frank Associates
181 E. 5600 S.
Murray, Utah 84107

Electrical Engineering:
Hunt Electric Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

LEGAL DESCRIPTION:

Lot 4, Bonneville Center Plat A Amended Lots 3 & 4.
7596-2684 7834-0864 7859-1093 9076-7702

DEFERRED SUBMITTALS:

- Fire Alarm Systems / Controls
- Fire Suppression Calculations / Shop Drawing Submittal
- Metal Stud Framing & Siesmic Connections
- Seismic Bracing for Mechanical, Electrical, Plumbing Components per ASCE 7.

PROJECT DATA : SQFT AREA

North Addition = 6,566 sqft
Total Area = 6,566 sqft

EXISTING MINIMUM R-VALUES:

GLAZING (FENESTRATION U-FACTOR) = N/A
ROOF R-VALUE = R-30
STEEL FRAMED WALL R-VALUE = R-21
SLAB R-VALUE (within 4' of interior foundation) = R-10

GENERAL NOTES

- IF THERE ARE ANY CONFLICTS BETWEEN ITEMS ON DRAWINGS AND GENERAL NOTES, THE MOST STRINGENT REQUIREMENT AND HIGHEST PRICE PRODUCT GOVERNS
- ACTUAL SITE DIMENSIONS MAY VARY, CONTRACTOR TO VERIFY ALL DIMENSIONS BEFORE STARTING WORK. CONTRACTOR TO NOTIFY ARCHITECT UPON DISCOVERY OF ANY DISCREPENCIES.

PROJECT DESCRIPTION:

The project includes the addition of new warehouse floor area for storage and distribution.

CODE REVIEW:

Applicable Codes: Salt Lake City Building Department & Fire Department
2021 IEBC - Compliance Method = Prescriptive Compliance Method (Ch 3 & 5).
2021 International Building Code (IBC) w/ Utah State Amendments which Include:
Building, Plumbing, Fuel/Gas, Mechanical, and Fire Codes.
2020 National Electrical Code (NEC) State of Utah Title 15A requirements have been compiled into these CD's
2021 International Energy Conservation Code (IECC)
ANSI A117.1-2009

Occupancy	Existing Building = F-1
Construction Type	Existing Building = IIB (Ch 6)
Allowable Area	Unlimited
Actual Area	5,566 sqft (Addition)
Allowable Height	N/A (See 503.1.3) (Also Note 504.4 = 12 Stories w/ Sprinkler).
Actual Height	1 Story - 32'-6"
Number of Exits	Required = 2 Provided = 3
Seismic Design	Category = DII Design Wind Speed = See Structural
Sprinklers:	Provided with Existing Building. Will be provided to new addition. NFPA 13. Sprinkler Design is by Deferred Submittal. GC to Submit layout for Approval by Salt Lake City Fire Department and Authority Having Jurisdiction.
Fire Separations	508.3. Non - Required Existing 2 Hour Exterior Wall. All Penetrations will be fire caulked. 1 Hour provided between Electrical
Fire Resistance Ratings	See IBC Table 601 for Type II B - Building Elements (Hours) Primary Structure = 0 Bearing Walls = 0 (0 Exterior; 0 Interior) Non Bearing Walls & Partitions Ext. / Int. = 0 (Electrical Load requires this to have a 1 Hour Separation) Floor Construction = 0 (12" Concrete Slab on Grade) Roof Construction = 0 Exterior Walls based on Fire Separation Distance (Hours) = Greater than 30'-0" = 0
Plumbing Fixtures	All minimum required plumbing fixtures are provided in existing building per IBC table 2902.1.



LEVEL ONE - KEY PLAN

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AG003	ARCHITECTURAL SPECIFICATIONS	X		
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C003	UTILITY PLAN	X		
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S501	FOOTING FOUNDATION DETAILS	X		
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E11	PANEL SCHEDULES	X		
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E302	ENLARGED LIGHTING PLAN	X		
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Issued/Revisions

No.	Description	Date
1	CLIENT REVIEW / PRICING	02/06/24
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Project Name
ALBANY MECHANICAL

Sheet Title
**LIFE SAFETY PLAN
ASSEMBLY TYPES**

Scale Date
Date **02.06.2024**

GxA Project No.
Project No. **24-002**

Sheet No.

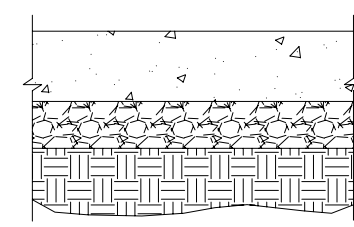
AG001

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

GENERAL NOTES

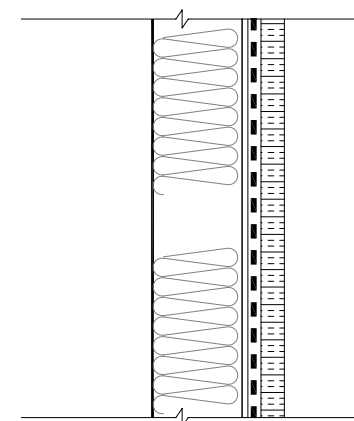
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- CONTRACTOR TO TAKE NECESSARY MEASURES TO PROTECT THE EXISTING BUILDING FROM DAMAGE
- CONTRACTOR TO LOCATE EXIT SIGNAGE AT EXITS AND ALONG EXIT ACCESS TRAVEL TO CLEARLY IDENTIFY LOCATION OF AND DIRECTION TO NEAREST EXIT. (SEE ELECTRICAL DWGS).
- CONTRACTOR TO COORDINATE WITH LOCAL FIRE MARSHAL AS REQUIRED.
- COORDINATE ADDITIONAL DEFERRED FIRE SPRINKLER SUBMITTAL AS REQUIRED WITH FIRE MARSHAL AND AUTHORITY HAVING JURISDICTION.

FLOOR ASSEMBLY



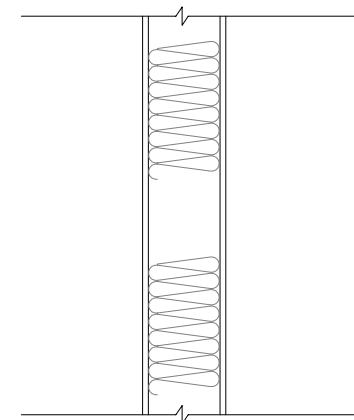
- F1 SLAB ON GRADE (WAREHOUSE)**
- 12" CONCRETE FLOOR SLAB
 - 4" THICK GRAVEL

EXTERIOR WALL ASSEMBLY



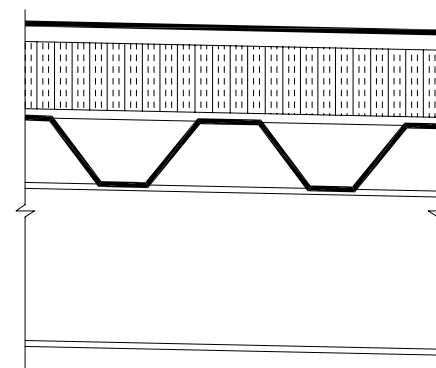
- EW1 EFIS ON METAL STUD**
- 2" MIN. EIFS (NON-COMBUSTIBLE, R-20 MIN.)
 - 1/4" DRAINAGE PLANE
 - TYVEK OR SIMILAR BUILDING WRAP OR LIQUID APPLIED AIR / VAPOR BARRIER
 - 5/8" EXTERIOR GYPSUM BOARD
 - 4" STRUCTURAL STEEL STUDS
 - R - 13 UNFACED BATT INSULATION
 - INTERIOR WAREHOUSE LINER
 - WHITE REFLECTIVE INSULATION / VAPOR BARRIER

INTERIOR WALL ASSEMBLY



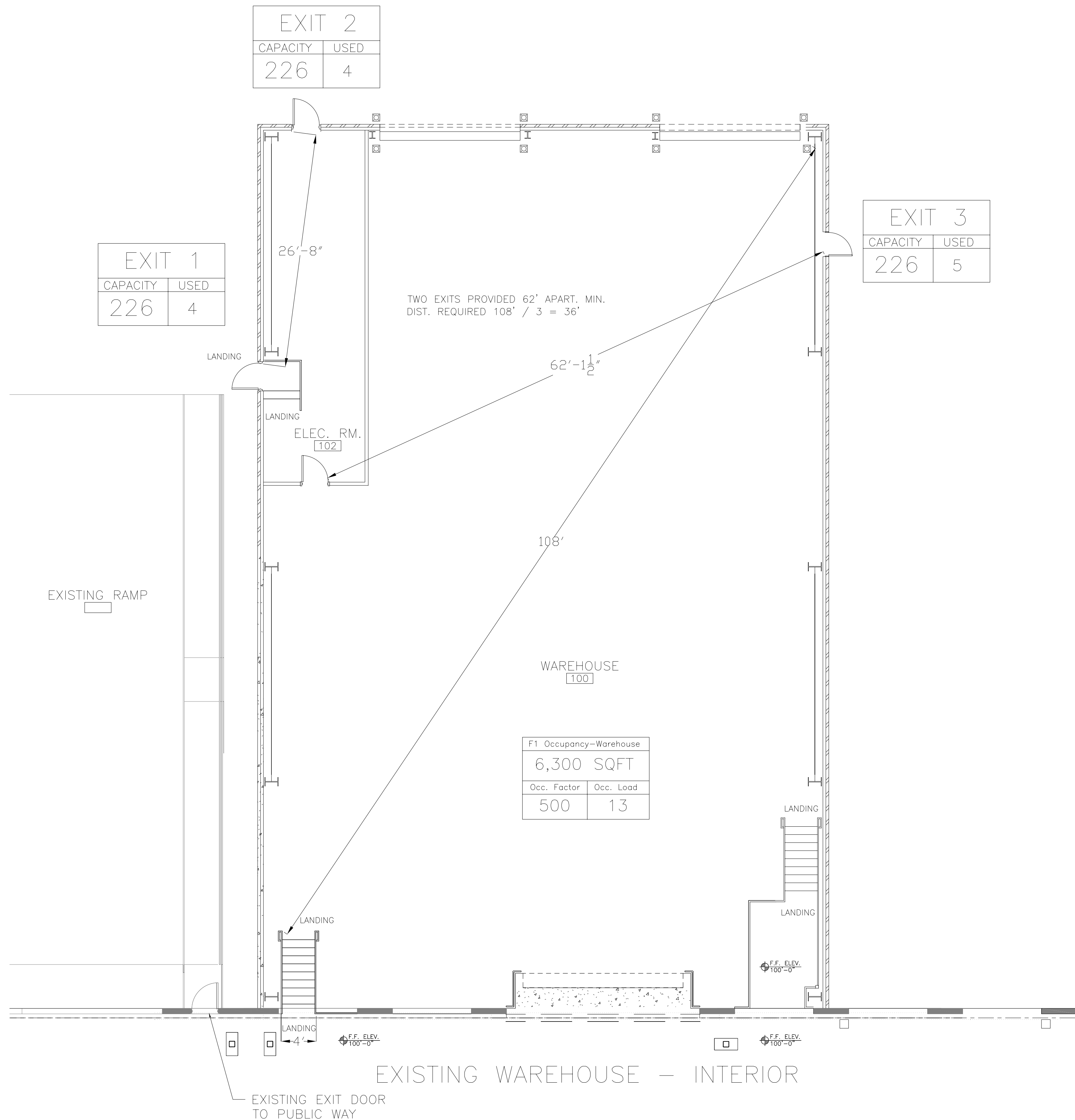
- IW1 METAL STUD WITH GYPSUM
1 HR FIRE RATED WALL - UL 263 / U419)**
- 5/8" TYPE X GYPSUM BOARD
 - 6" STEEL STUD (SPACING PER STRUCT).
 - FIBERGLASS INSULATION OPTIONAL
 - 5/8" TYPE X GYPSUM BOARD

ROOF ASSEMBLY



- R1 INSULATED ROOF MEMBRANE ON METAL DECK**
- 60 MIL WHITE SINGLE PLY PVC ROOF MEMBRANE (FULLY ADHERED)
 - 1/2" 100 PSI POLYISO COVERBOARD
 - TAPERED INSULATION AS REQUIRED FOR DRAINAGE
 - 6" POLYISO INSULATION BOARD (R-30)
 - VAPOR BARRIER MEMBRANE
 - 5/8" FIBER FACED GYPSUM ROOF BOARD
 - STEEL DECK ON SLOPED STRUCTURAL STEEL

(NOTE - TAPERED INSULATION & CRICKETS FOR ALL MECHANICAL PADS AND ROOF DRAINAGE PER ROOF PLAN.)



Subconsultants
Dunn & Associates
 380 W. 800 S. #100
 Salt Lake City, Utah 84101

Van Boerum & Frank Assoc.
 181 E. 5600 S.
 Murray, Utah 84107

Hunt Electric, Inc.
 1863 Alexander Street
 Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

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Project Name
ALBANY - NORTH ADD.

ARCHITECTURAL
SITE PLAN

Scale
1/64"=1'-0"
Date
02.06.2024
Drawn
GxA
Project No.
24-002

Sheet No.

A100

SALT LAKE CITY PARKING CALCULATIONS (SLC 21A.44.030)

Existing Building (Industrial Use) = Light Manufacturing 345,250 Sqft

Required

Minimum = 1 space per 1,000 Sqft = 346 Stalls
Maximum = No Maximum

Provided = 535 Parking Stalls (Existing)

New Manufacturing / Warehouse = 6,500 Sqft

Required

Minimum = 1 space per 1,000 Sqft = 7 Stalls

Provided

7 Parking Stalls (Existing)

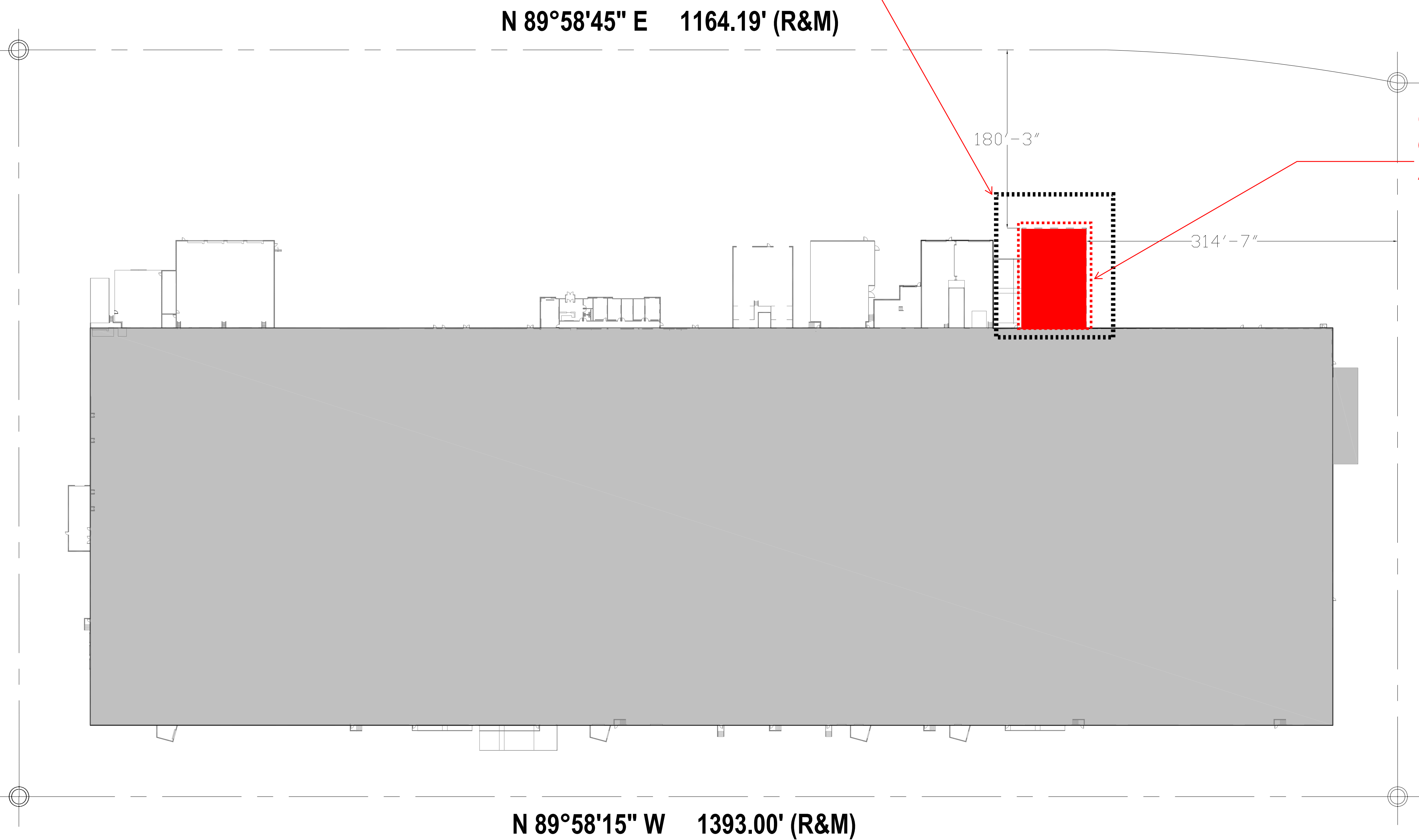
2 PARKING CALCULATION

A100 SCALE: N/A

NEW BUILDING ADDITION

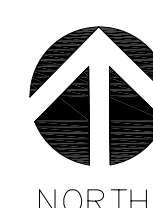
GC TO VERIFY MINIMUM CUTTING OF EXISTING ASPHALT TO ACCOMODATE EXCAVATION AND NEW FOUNDATION SYSTEMS.

D
C
B
A



1 ARCHITECTURAL SITE PLAN

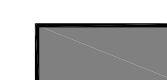
A100 SCALE: 1/64"=1'-0"



GENERAL NOTES

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Project Name
ALBANY NORTH ADD.

Sheet Title
DEMOLITION FLOOR PLAN

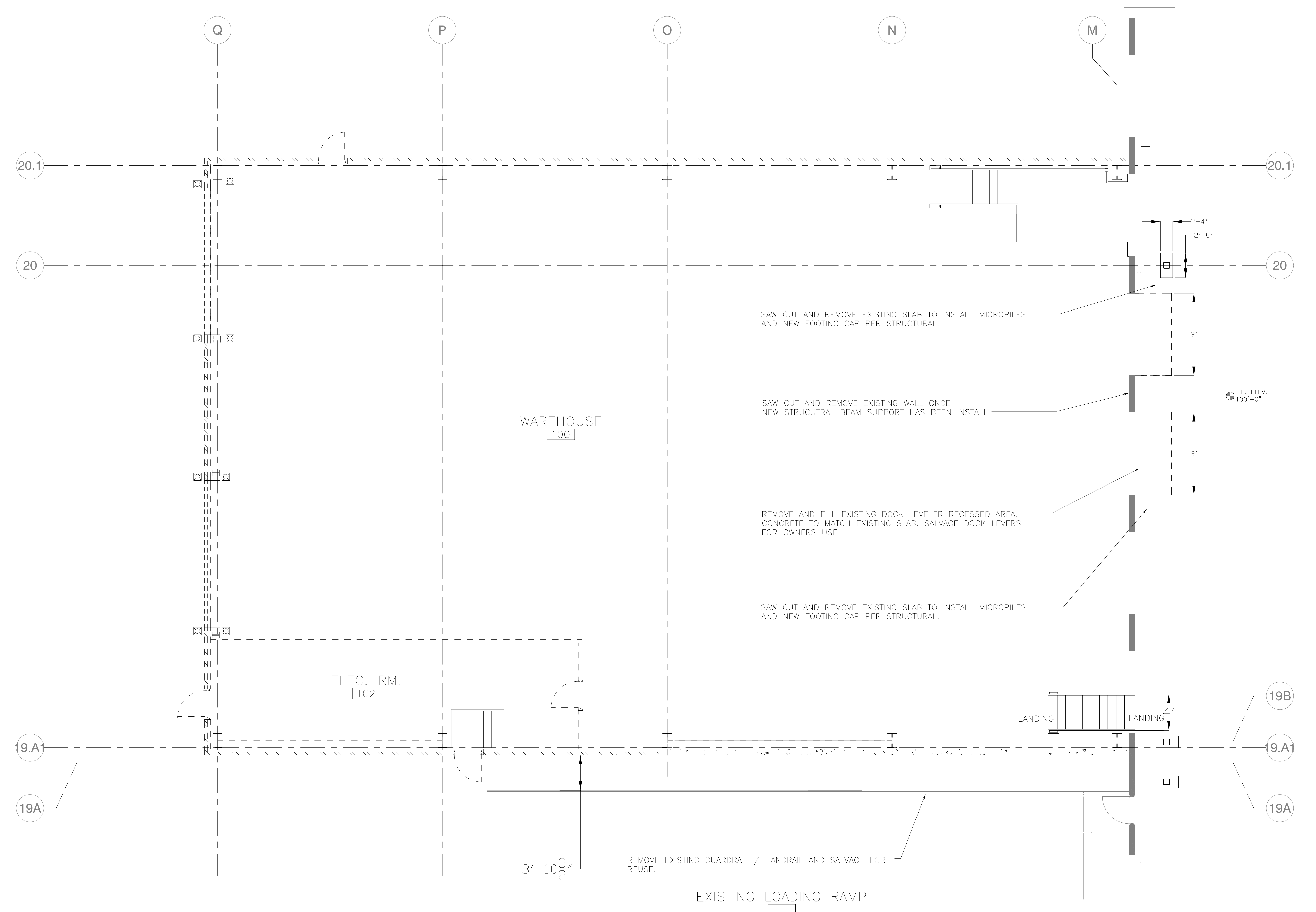
Scale	3/16"=1'-0"	Date	02.06.2024
Drawn	GxA	Project No.	24-002

Sheet No.

A200



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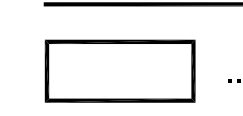


1
A200 **DEMOLITION FLOOR PLAN**
SCALE: 3/16" = 1'-0"

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Project Name
ALBANY NORTH ADD.

Sheet Title
LEVEL ONE FLOOR PLAN

Scale
3/16"=1'-0" Date
02.06.2024

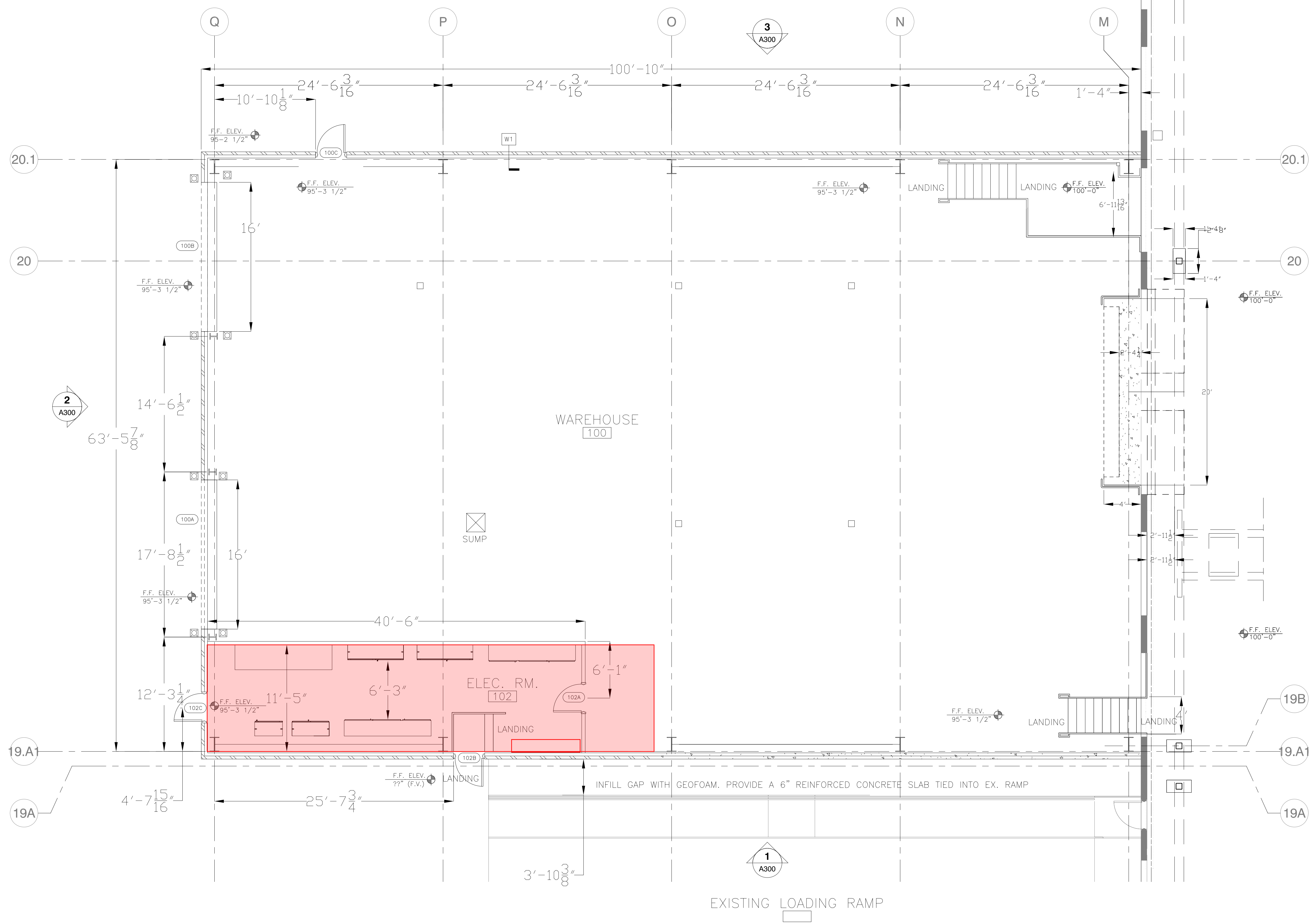
Drawn
GxA Project No.
24-002

Sheet No.



A201

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

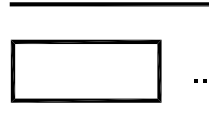


1 LEVEL ONE FLOOR PLAN
A201 SCALE: 3/16" = 1'-0"

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Sheet Title
LEVEL ONE FLOOR PLAN

Scale
3/16"=1'-0" Date
02.06.2024

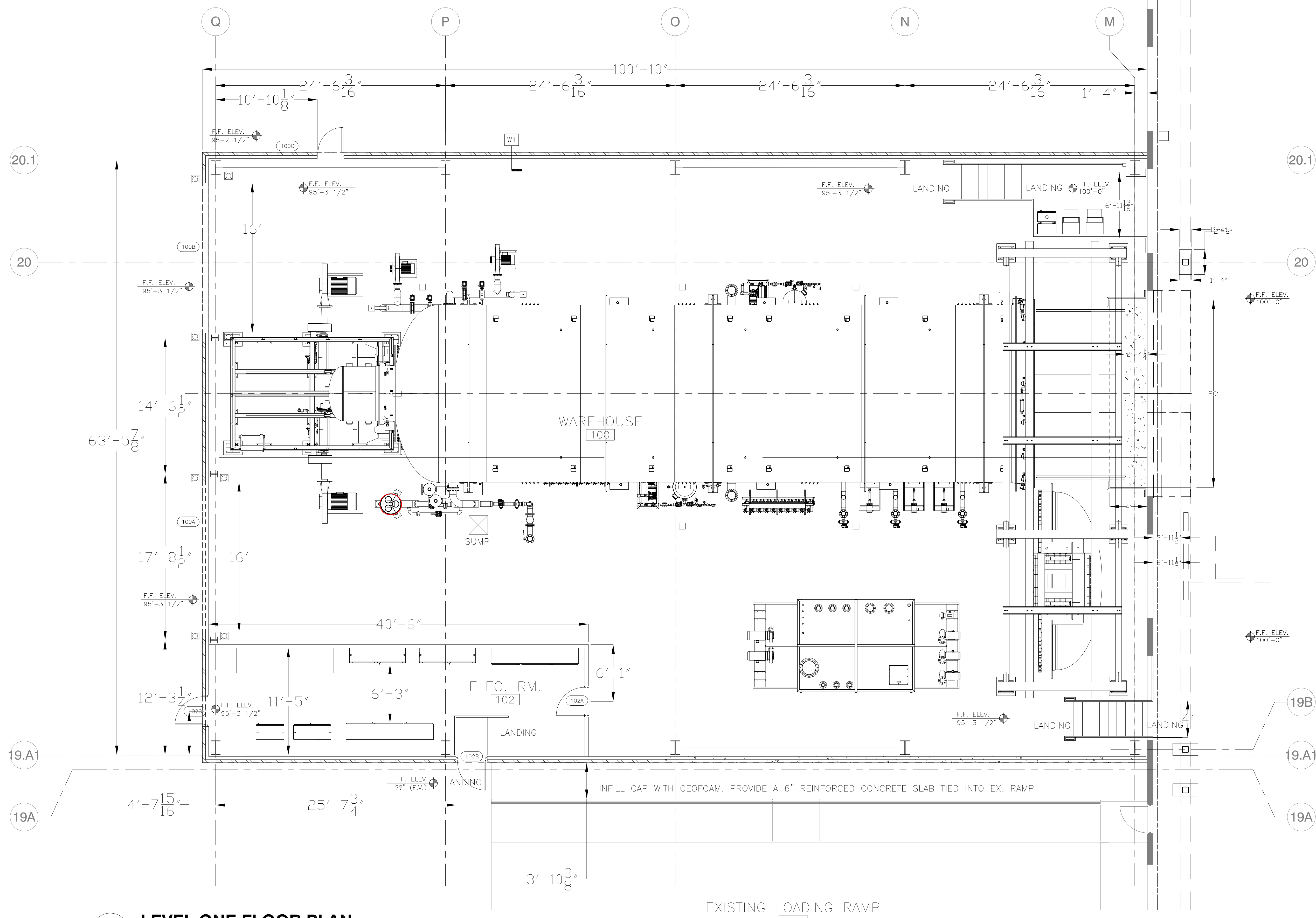
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GxA Project No.
24-002

Sheet No.



A201.1

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116



1 LEVEL ONE FLOOR PLAN
A201.1 SCALE: 3/16" = 1'-0"

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Project Name
ALBANY NORTH - ADD.

Sheet Title
ROOF PLAN

Scale
3/16"=1'-0"

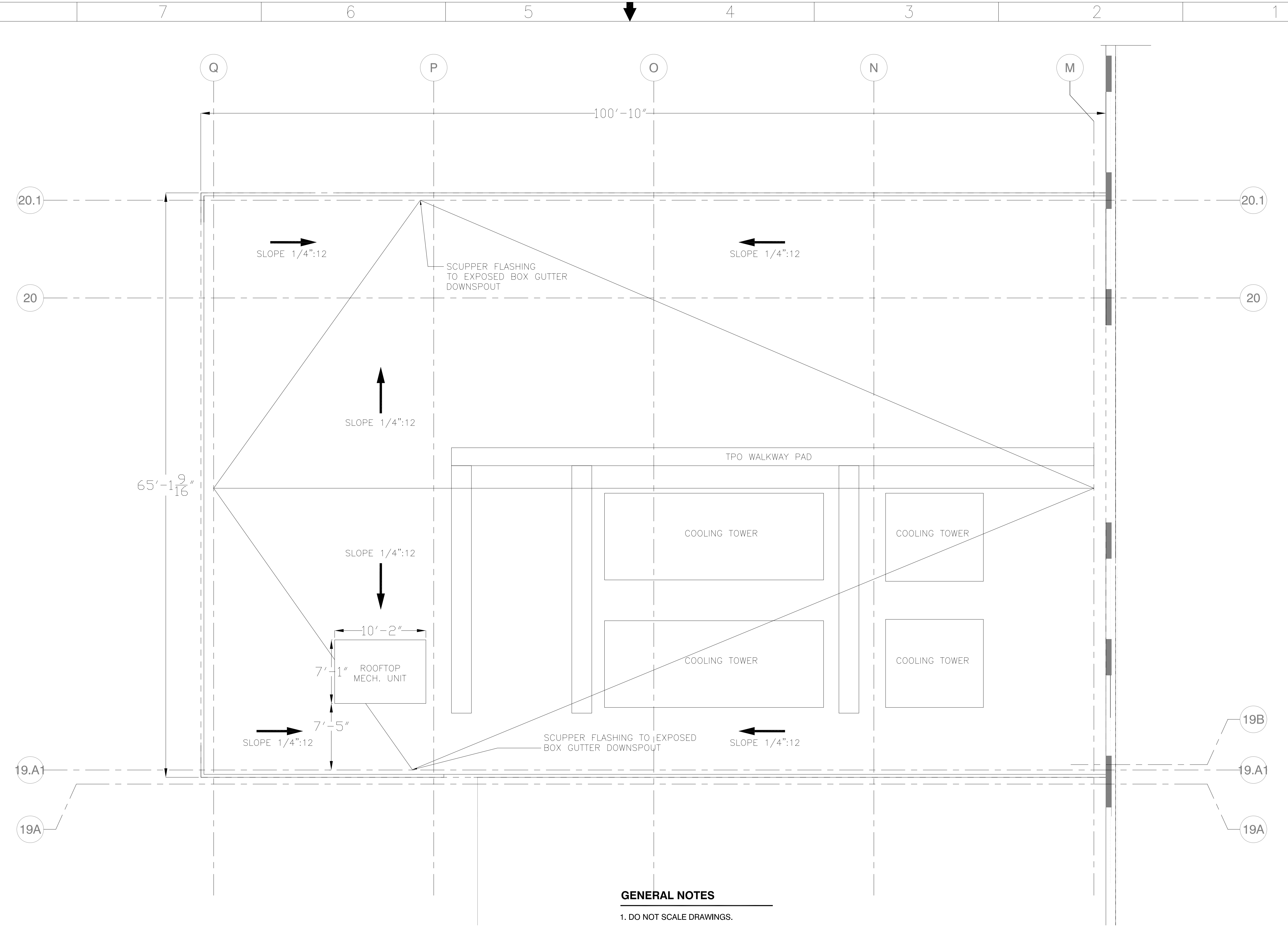
Date
02.06.2024

Drawn
GxA

Project No.
24-002

Sheet No.
A202

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116



1 ROOF PLAN
SCALE: 3/16" = 1'-0"



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- CONTRACTOR TO TAKE NECESSARY MEASURES TO PROTECT THE EXISTING BUILDING FROM DAMAGE
- CONTRACTOR TO PROVIDE SHOP DRAWINGS AND SUBMITTALS TO ARCHITECT / OWNER FOR REVIEW AND APPROVAL.
- REFER TO STRUCTURAL DRAWINGS AND CALCULATIONS TO CONFIRM ALL SIZES AND ATTACHMENTS.
- CONTRACTOR TO COMPLY WITH ALL REQUIRED SPECIAL INSPECTIONS IMPOSED BY TOWN OF ALTA.
- CONTRACTOR TO LEAVE JOB SITE CLEAN OF ALL DEBRIS AT ALL TIMES. COORDINATE WITH ALBANY FACILITIES DIRECTOR ON CONSTRUCTION STAGING AREA, MITIGATION PLAN AND DUMPSTER LOCATION AS NECESSARY. BUILDING OWNER TO PROVIDE RESTROOM FOR ANY CONSTRUCTION WORKERS.
- CONTRACTOR TO COORDINATE WITH LOCAL FIRE MARSHALL AS REQUIRED.
- COORDINATE ADDITIONAL DEFERRED FIRE SPRINKLER SUBMITTAL AS REQUIRED WITH FIRE MARSHALL AND AUTHORITY HAVING JURISDICTION.

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Project Name
ALBANY MEZZANINE

Scale
GENERAL STRUCTURAL NOTES

Date
2024.02.02

Project No.
230103

Drawings and ELEVATIONS, AS THEY RELATE TO THE BUILDING IN GENERAL, i.e. GRID TO GRID DIMENSIONS OR DECK BEARING ELEVATIONS ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

S-001

GENERAL STRUCTURAL NOTES

- G. Protected Zones: No connectors, other than those on the design drawings, shall be made within the protected zone of the SLRS as identified in AISC 341.
1. Locate headed studs, welds, miscellaneous metal, etc. outside of the protected zone.
 2. Paint the protected zones with bright paint before and after fire coating operations to identify them.
- H. All welds not noted on drawings shall be minimum 1/4" fillet welds.
1. All structural steel members shall be considered as unrestrained fire-resistance-rated assembly.
4. Welding of Reinforcing Steel or Bolts
- A. Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars with deformed bar anchors, structural bolts, or headed stud anchors.
 - B. Do not weld anchor bolts, including "back" welds.
 - C. Headed Stud Anchoring and Deformed Bar Anchor welding shall conform to the manufacturer's specifications.

CONCRETE
Concrete shall be supplied in accordance with ACI 318 and the following requirements:

Concrete Use	Comp. Strength f'c (psi)	Exposure Classes per ACI 318 19.3.1 (a,b,c)	Nominal Max Aggregate Size
Footings / Pile Caps	4000	F0, S0, W0, C1	1 1/2"
Other Walls	4000	F0, S0, W0, C0	3/4"
Interior Slabs on Grade (d,e,f)	4000	F0, S0, W0, C0	3/4"
Light Wt. Concrete on Steel Deck (g)	3000	F0, S0, W0, C0	3/4"

- Table Footnotes:**
- a. Cement type (ASTM C150 or C595), max. water/cement ratio and fly ash to comply with ACI 318 Table 19.3.2.1.
 - b. Air content ± 1.5%, to comply with ACI 318 Tables 19.3.2.1 and 19.3.3.1, initially measured at point of final placement and point of discharge. Subsequent measurements shall occur only at point of discharge provided air content is adjusted to account for placement losses. Air content shall be adjusted for the use of admixtures, fly ash and aggregate size, with ASTM C260 (when used).
 - c. Calcium chloride shall not be added to the concrete mix. Unreinforced concrete slabs on grade may use calcium chloride as permitted by ACI 318 Table 19.3.2.1.
 - d. For any exposed slab on grade, the contractor is to notify the engineer of record at least 7 days prior to any pours to discuss the concrete mix design being used as well as present their means and methods of addressing concrete phenomena such as cracking, curling, spalling, etc.
 - e. Interior slabs on grade shall have a drying shrinkage maximum of 0.004% by ASTM C157 (7-day soak time permitted). Test results shall be submitted with mix designs.
 - f. For slab on grade 6" or thicker, a minimum aggregate size of 1 1/2" is permitted.
 - g. Contractor shall provide verification that mix design for lightweight concrete over metal deck has a maximum dry weight density of 110 ± 5 pcf per ASTM C-507.

- Materials unless noted otherwise:**
- A. Normal Weight aggregates—ASTM C33
 - B. Light Weight aggregates—ASTM C330
 - C. Fly Ash, Class C or F Pozzolan—ASTM C618
- D. Reinforcing Steel**
1. General—ASTM A615 Grade 60
 2. Subject to the above requirements, ASTM A615 Grade 75 steel may be used at the contractor's option, except in special moment frames, special concrete shear walls, shear struts or torsional reinforcement.
 - E. Deformed Bar Anchors (DBA)—ASTM A496
 - F. Headed Stud Anchors (HSA)—ASTM A108
 - G. Anchor Bolts: See steel anchor and web section(s) of general notes.
 - H. No aluminum content or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.
- 2. Reinforce concrete slabs over metal deck with the following welded wire reinforcement (minimum), based on the thickness of concrete above upper deck flutes. Welded wire reinforcement shall be placed "1" to "1 1/2" below the top of the slab:**
- A. Slab (above upper deck flute) 3/4" or thinner—6" x 6" W1-W4W1.4
 - B. Slab (above upper deck flute) 4" or thinner, but thicker than 3/4"—6" x 6" W2-W1W2.1
 - C. Slab (above upper deck flute) thicker than 4"—6" x 6" W2-W1W2.9
- 3. Welded wire reinforcement may be substituted with macro synthetic fibers "coarse fibers" (per ASTM C-1116), made from virgin polyolefin, with equivalent diameter between 0.016" and 0.05", having minimum aspect ratio (length/equivalent diameter) of 50, at a minimum rate of 4 lb / cubic yard. Fibers above finished slab shall not be burned off. Do not use fibers in architecturally finished or colored concrete.**
- 4. Other Structural Shapes (M, C, etc), Threaded Rod—ASTM A36**
- A. Supporting forms and shoring shall not be removed until structural members have acquired sufficient strength to safely support their own weight and any construction load to which they may be subjected. In no case, however, shall forms and shoring be removed in less than 24 hours after concrete placement.
 - B. Suspended slabs shall be re-supported after form removal until concrete reaches its 28-day specified compressive strength.

- Reinforcement shall have the following concrete clear cover:**
- A. Cast-in-place Concrete
 1. Cast against and permanently exposed to earth—3"
 2. Formed concrete exposed to earth or weather:
 - 48 thru #8 bars—2"
 - 45 and smaller bars—1 1/2"
 3. Concrete not exposed to weather or in contact with ground:
 - Slabs, Walls, Joists: #1 bars and smaller—3/4"
 - Beams, Columns: Primary Reinforcement, Ties—1 1/2"
- Strips, Spirals**
- Construction Joints and Control Joints:**
- A. Provide a beveled 2" x 4" continuous or intermittent keyway in all horizontal and vertical construction joints including between top of footing and foundation walls. In addition, all joints shall be internally roughened to a full amplitude of approximately 1/4".
 - B. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. Control joints shall be completed within 12 hours of concrete placement. Control joints may be installed by either:
 1. Saw cut with depth of 1/4 the thickness of the slab
 2. Tied joints with depth of 1/4 the thickness of the slab
 - C. Install control joints in slabs on grade at a spacing not to exceed 30 times the slab thickness in any direction, unless noted otherwise. Construction joints in walls on grade shall not exceed 30 times the wall thickness in any direction.
 - D. Install construction joints in slabs at a spacing not to exceed 30 times the wall center, except in concrete shear walls. For masonry walls above, align joints in concrete walls with masonry control joints.
 - E. Construction joints are not permitted in suspended slabs or beams unless specifically noted on the construction documents or submitted by the Contractor to the Engineer of Record for review.
- 7. Construction**
- A. Use chains or other support devices recommended by the CRSI to support bar and tie reinforcement bars and WWR prior to placing concrete. WWR shall be continuously supported at 30" or center maximum. Reinforcing steel for slabs on grade shall be adequately supported on precast concrete elements. Lifts of reinforcement off the grade during placement of concrete is not permitted.
 - B. Contractor shall coordinate placement of all openings, cuts, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
 - C. All embeds and dowels shall be securely tied to formwork or to adjacent rebar prior to concrete placement.
 - D. No pipes, ducts, sleeves, etc. shall be placed in structural concrete unless specifically detailed or approved by the structural engineer. Penetrations through walls when approved shall be built into the wall prior to concrete placement. Penetrations will be allowed in footings or grade beams unless detailed. Piping shall be routed around these elements and footings stopped to avoid piping.
 - E. Reinforcing bars shall not be welded unless specifically shown on drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for DBAs or HSAs.
 - F. Reinforcing bars shall not be field bent, except as shown on the contract drawings or permitted by the EOR.
 - G. Top of concrete columns shall be flush (±1/4") with bottom of supported cast-in-place members.

- 8. Detailing**
- A. Lap splice lengths shall be detailed to comply with the "Reinforcing Bar Lap Splice Schedule" contained within the contract drawings.
 1. Do not splice strips and ties. Do not splice vertical bars in retaining walls unless specifically shown.
 2. At shear wall boundary elements lap lengths shall be increased by 25%.
 3. Splices may be made with mechanical splices capable of 125% of yield strength of the bar being spliced (Type 1). Splices located within lateral resisting elements shall also develop the tensile strength of the bar (Type 2). Mechanical splices shall be the positive connecting type coupler and shall meet all ACI requirements. Use "Cadevel", "Lentor" Standard Couplers, "Bar-Lock" or equal with mechanical protector. If mechanical splices are used, splices or couplers on adjacent bars shall be staggered a minimum of 24" apart along the longitudinal axis of the reinforcing bars.
 - B. All 90, 135 and 180° hooks shown graphically in the drawings shall be detailed as ACI standard hooks, unless noted otherwise.
 - C. All joints provide reinforcing dowels to match the member reinforcing unless noted otherwise.
 - D. At all discontinuous control or construction slab on grade joints, provide (2) #4 x 48".
 - E. Provide corner bars at intersecting walls corners using the same bar size and spacing as the horizontal wall reinforcing.
 - F. All vertical reinforcing shall be detailed to footings, or at the structure below with the same size and spacing as the vertical reinforcing for the element above. Dowels extending into footings shall terminate with a 60° standard hook and shall extend to within 4" of the bottom of the footing. Footing dowels (#8 bars and smaller) with hooks need not extend more than 20" into footings.
 - G. See details for reinforcing around miscellaneous openings. All recesses that interrupt reinforcing shall be reinforced the same as an opening. H. Reference ACI 318 for additional detailing requirements.
- 9. Contractor required to submit concrete mix design for review by the engineer prior to any placement of concrete.**
- 10. All concrete shall be mixed and placed per ACI 304. Contractor shall refer to and follow the recommendations in ACI 305R for hot weather concreting and ACI 308R for cold weather concreting.**
- 11. Construction activity or storage of materials shall not take place on newly placed concrete until the concrete achieves sufficient strength to provide adequate support.**

- COMPOSITE STEEL BEAMS**
1. All beams supporting concrete over metal deck shall have headed stud anchors.
 2. Composite beams are indicated on the framing plans with a suffix (n). The number inside parentheses indicates the number of studs for this beam or section of beam. Beams or sections of beams shall have the studs spaced uniformly over the beam or section. The maximum spacing shall not exceed 36" on center.
 3. All headed stud anchors shall conform to ASTM A-108. Dimensions shall comply with AISC. Use 3/4" diameter studs. Headed studs shall extend 1 1/2" minimum (2" maximum) above the top of the steel deck after welding. Headed studs shall be applied through the metal deck to the top flange of the steel section or welded directly to the steel section.
 4. The minimum center spacing of stud connectors shall be 6 diameters along the longitudinal axis of the supporting composite beam and 4 diameters transverse to the longitudinal axis of the supporting composite beam.
 5. Composite beams shall be preassembled as shown on plans. On the plans, "c=0.07" denotes preamber dimension (upward) in inches.
 6. Camber tolerances shall be ±.14". 2".
 7. Slab shall be screeded to a constant thickness as indicated.

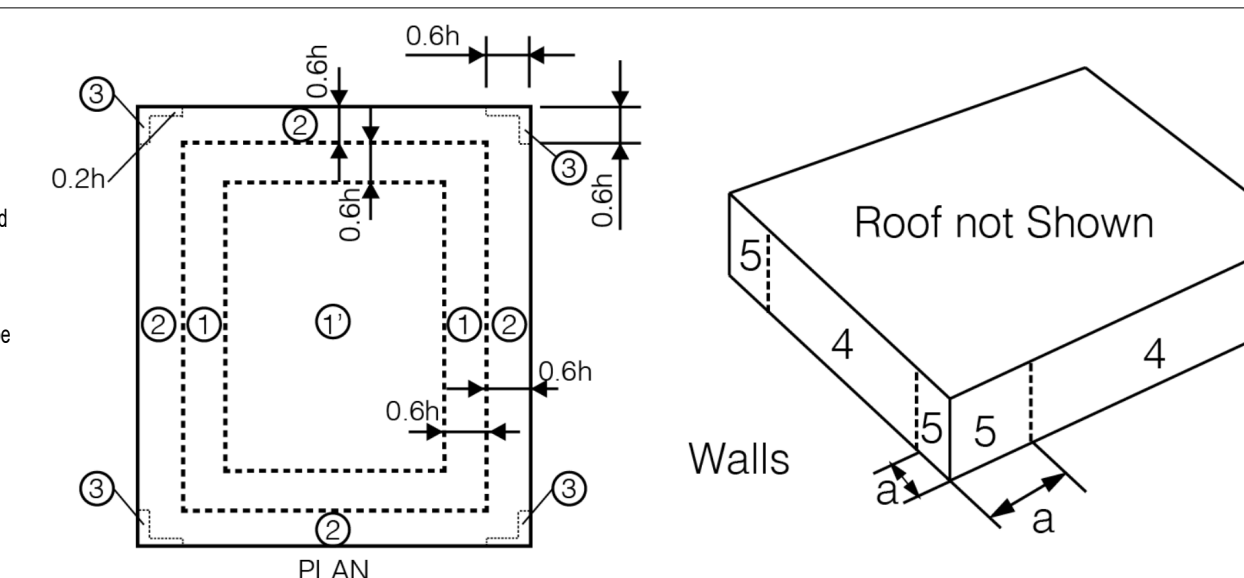


Figure 30.3-2A Roof Pressure Diagram (Zones 1-3, including overhangs "OH" where applicable) and **Figure 30.3-1** Wall Pressure Diagram (Zones 4-5)

- FOUNDATION**
1. Soils Investigation Report—None
 2. Soil bearing pressure—1500 psf - Assumed for design
 3. Frost Protection—30" minimum
 4. Clear excavation of debris and loose soil prior to placing footings. All footings shall bear on undisturbed natural sub-grade or engineered compacted fill as noted in these drawings.

- EARTHWORK**
1. Consult the project specifications for earthwork requirements. In absence of information, refer to the following notes.
 2. Clearing: Remove all existing structures and associated foundations, slabs, fencing, asphalt, concrete, and incidental structures as necessary for project completion. The building area shall be stripped of all vegetation, topsoil, and debris. Following stripping, all undocumented fill soils and any remaining loose natural soils shall be excavated to expose competent natural soils.
 3. Contractor shall provide temporary shoring for excavations as required.
 4. Contractor shall provide measures necessary to prevent damage to or settlement of new or existing construction and utilities on or adjacent to project site.
 5. Contractor shall provide dewatering as required to protect the site from flooding.
 6. Proof roll the entire build-out area with normal compaction equipment to check for the presence of unsuitable fills, soft spots, or other undesirable materials or conditions. Remove sub-grade materials that are unsuitable and replace with compacted structural fill or 2,000 psi lean concrete.
 7. Compacted structural fill: All fill materials shall be a well-graded granular material with a maximum size less than 3" and with not more than 15% passing a #200 sieve. Fill beneath footings shall be compacted to at least 95% of the maximum laboratory density as determined by ASTM D 1557. All fills shall be tested. Compacted structural fill shall be placed in lifts not exceeding 9" in uncompactable thickness.
 8. Floor slabs shall be underlain by a granular layer at least 4" thick. The granular layer shall have a maximum size less than 1" with not more than 5% passing a #200 sieve and shall be compacted to at least 90% of the maximum laboratory density as determined by ASTM D 1557.
 9. The special inspector shall review all excavations and fill placement prior to placing concrete.

- STRUCTURAL STEEL**
1. Codes and Standards: Fabrication, Erection and Quality Control of structural steel shall comply with the latest edition of the following:
 - A. American Institute of Steel Construction (AISC) 360, "Specification for Structural Steel Buildings," with "Commentary".
 - B. AISC 341 "Seismic Provisions for Structural Steel Buildings."
 - C. AISC 303 "Code of Standard Practice" excluding sections 3.1.4, 4.4 and 4.4.1.
 - D. AISC "Specification for Structural Joints Using High Strength Bolts"
 - E. American Welding Society (AWS), Structural Welding Codes D1.1, D1.3, D1.4, and D1.8, except as modified by the "Steel Construction Manual"
 2. Material
 - A. Wide Flange Sections—ASTM A992 (50 ksi)
Notch-toughness requirements apply for Group 3, 4, and 5 shapes with flange thickness greater than 1 1/2" and plate 2" and thicker which are a part of the Seismic Load Resisting System (SLRS). Minimum Charpy V-Notch requirements are 20 ft-lbs at 70°F.
 - B. Plate
 1. Typical—ASTM A36
 2. Braced/Moment Frames—ASTM A572 Grade 50
 - C. Pipe
 1. Typical—ASTM A53 Grade B Type E/S
 - D. Hollow Structural Shapes
 1. Rectangular—ASTM A500 Grade C (50 ksi)
 2. Round—ASTM A500 Grade C (46 ksi)
 - E. Other Structural Shapes (M, C, etc), Threaded Rod—ASTM A36
 - F. Bolted Connections—ASTM F1554 Grade A325 with ASTM A563 heavy hex nuts and ASTM F436 washers.
 - G. Anchor Bolts
 1. All Columns unless noted otherwise: ASTM F1554 Grade 105 with ASTM A563 heavy hex nuts. Nuts to be snug tight.
 2. Braced Frame/Moment Frame Columns unless noted otherwise: ASTM F1554 Grade 105 (equivalent to A193 Grade B7) with ASTM A563 heavy hex nuts with 5/16" min plate washers. Bottom assembly to include double heavy hex nuts with similar washer. Nuts to be snug tight.
 - H. Weld Filler Metal
 1. Shielded Metal Arc Welding—AWS A5.1, low-hydrogen only
Low-hydrogen restrictions do not apply when welding sheet steels in accordance with AWS D1.3, including attaching these steels to structural members.
 2. Gas-Metal Arc Welding—AWS A5.18
 3. Flux-Cored Arc Welding—AWS A5.20
 4. E70T-4 or E70T-11 electrodes are not permitted.
 4. Intermixing of welds made from self-shielded electrodes with welds made by other processes is not allowed in seismic critical welds, unless tested in accordance with AWS D1.8, annex B. The Field Erection Contractor is responsible for verifying that intermixing of self-shielded weld metal with weld metal of other processes will not occur, or alternatively, the welding procedure is qualified by testing.
 5. Where ductile critical welds are required, provide filler metals meeting the following minimum mechanical properties: 58ksi yield strength, 70ksi tensile strength, 22% elongation, Charpy V-Notch toughnesses of 20ft-lbs at 0°F and 40 ft-lbs at 70°F.
 - I. Deformed Bar Anchors (DBA)—ASTM A496
 - J. Headed Stud Anchors (HSA)—ASTM A108
 - K. Non-Shrink Grout—ASTM C1107 Grade B
Non-shrink grout shall be prepackaged, non-metallic, and non-gaseous. Furnish certified independent test data to Structural Engineer.

- 3. All steel, connectors and embeds exposed to weather shall be galvanized, unless noted otherwise.**
- M. All steel, connectors and embeds exposed to weather shall be galvanized, unless noted otherwise.**
- A. Welds may be performed in the shop or the field. Designations of field welds on the Contract Documents are shown where it is anticipated field welds may be required, and are shown only for the purpose of assisting the Contractor in the bidding process. The Contractor shall coordinate the welding sequence between sub-contractors, and any costs associated with variations in the welding sequence are outside the scope of the Design Engineer, and are the responsibility of the Contractor. Field welding is to be minimized where possible. Contractor is to verify that the sequencing of welds meets all safety regulations, and the requirements of the Construction Documents and their referenced codes. Welding in the Y region of wide flange members is prohibited unless noted otherwise.**
- B. Provide full depth web stiffener plates at one side of all beams at all bearing points, unless noted otherwise. Stiffener plates shall be the thickness called out below unless noted otherwise. Stiffeners shall be welded on both sides of the plate-to-flange and plate-to-web interfaces.**
- FLANGE WIDTH—STIFFENER THICKNESS & WELD SIZE**
- | Flange Width | Stiffener Thickness | Weld Size |
|--------------------|---------------------|-----------|
| Less than 8 1/4" | 1 1/4" | 3/16" |
| 8 1/4" to 12 1/4" | 3/8" | 3/16" |
| 12 1/4" to 16 1/2" | 1/2" | 5/16" |
| 16 1/2" to 20 3/4" | 5/8" | 3/8" |
- C. Bolting and Fasteners**
1. Ordinary steel-to-steel connections, simple span framing, and beam/girder-to-bearing plates are the standard connection used throughout the design drawings, unless noted otherwise:
 - a. Use A325N bolts or tension-controlled bolts.
 - b. Tighten these fasteners to a "snug tight" condition.
 - c. Where a steel-to-steel connection is not shown, provide a framed connection per AISC for one half the total uniform load capacity of the beam for the span and steel specification.
 2. Prestressed connections are shown on the structural design drawings. They join steel-to-steel connections, unless noted otherwise:
 - a. Use A325N or A325X bolts or tension-controlled bolts.
 - b. Pretension these fasteners as required by AISC "Specification for Structural Joints Using ASTM A325 or A490 Bolts."
 3. Slip Critical connections (SC) are shown on the structural design drawings. They join steel-to-steel connections in Seismic Load-Resisting Systems (SLRS).
 4. Fasteners and washers shall not be reused. Scrap dirty, rusted, or water-contaminated bolt assemblies.
- D. Reduced Beam Sections**
1. Fabrication of the reduced flange sections of beams used in SLRS is restricted to mechanically guided thermal cutting processes. Freehand cutting is not permitted.
 2. Flange cuts shall meet the requirements of AISC 358.
 3. Repair of gages, notches, mill imperfections, shall conform to the requirements of the AISC and AWS provisions.
- E. Weld Access Holes and Temporary Attachments**
1. Fabricate beam copes and weld access holes using the geometry described in AISC 360 Section J1.6.
 2. Runoff tabs are to be removed unless noted otherwise.
- F. Backup Bars: Remove backup bars from connections in demand critical welds, unless noted otherwise. Backgauge the root and weld to sound metal. Reweld the gapped areas and add a 5/16" reinforcing fillet weld. Backup bars may remain for top flange beam welds provided the backup bar is welded to column flanges with 5/16" fillet weld.**

GENERAL

1. The structural notes are intended to complement the project specifications. Specific notes and details in the drawings shall govern over the structural notes and typical details.
2. Typical details and sections shall apply where specific details are not shown.
3. The contractor shall verify all site conditions and dimensions. If actual conditions differ from those shown in the contract drawings, the contractor shall immediately notify the architect/engineer before proceeding with the fabrication or construction of any affected elements. Discrepancies should be brought to the attention of the architect prior to fabrication or construction.
4. Drawings shall not be scaled for the purpose of preparing shop drawings or for construction. Where dimensions on the design drawings are not provided or inferred, the contractor may scale drawings only to estimate member lengths for the purpose of bidding.
5. Changes to these contract drawings may be made only by an authorized representative of Dunn Associates, Inc. Dunn Associates, Inc. shall not be held responsible or liable for any claims arising directly or indirectly from changes made without written authorization by an authorized representative of Dunn Associates, Inc.
6. Omissions or conflicts between the contract drawings and/or specifications shall be brought to the attention of the architect/engineer before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the architect/engineer at no additional cost to the owner.
7. The contractor shall submit a written request to the architect/engineer before proceeding with any changes, substitutions, or modifications. Any work done by the contractor before receiving written approval will be at the contractor's risk. These contract documents note and describe potential bid alternate details that may be requested from and approved by the Engineer of Record during the bidding and negotiation phase. The contractor may also submit to the architect/engineer for approval other substitutions or modifications to the design drawings as bid alternates during the bidding and negotiation phase. Field modifications to structural elements are not permitted without notification and approval by the Engineer of Record.
8. The contractor shall coordinate with all trades any items that are to be integrated into the structural system such as openings, penetrations, mechanical and electrical equipment, etc. Structural drawings do not show all openings. Refer to other discipline drawings. Sizes and locations of mechanical and other equipment that differs from those shown on the contract drawings shall be reported to the architect/engineer. Contractor shall take measures as required to insure that construction loads shall not exceed design loads for the structure.
9. Any structural items shown on other discipline's drawings that are not shown on the structural drawings, but that are noted as "refer to structural drawings" for additional information, shall be brought to the attention of the structural engineer by the contractor.
10. Items such as fireproofing, waterproofing, insulation, vapor barrier, etc., may be shown on related structural drawings for reference only. Refer to the architectural drawings or specifications for more information.
11. The contractor shall be responsible for means, methods, techniques, sequences, and procedures in order to comply with the contract drawings and specifications. The contractor shall provide adequate shoring and bracing as required for the chosen method of erection. Shoring and bracing shall remain in place until final connections for the permanent members are completed. The building shall not be considered stable until all connections are completed. Walls shall not be considered self-supporting and shall be braced until the roof/roof system is completed.
12. Site observations by a field representative of Dunn Associates, Inc. shall not be construed as approval of construction, the procedures, nor special inspection.
13. All work shall be done in accordance with OSHA requirements. Potential conflicts between these documents and OSHA requirements shall be brought to the attention of the structural engineer before proceeding with the work.
14. Shop Drawings and submittals:
 - A. Shop drawings include plans, details, calculations and/or other relevant design information. Review of shop drawings and submittals by Dunn Associates, Inc. is for general compliance only and is not intended for approval. The shop drawing review shall not relieve the contractor of the responsibility of completing the project according to the contract documents.
 - B. Submittals for the following items shall be submitted to the Project Architect/Engineer for review prior to fabrication and/or installation:
 1. Concrete Mix Design
 2. Concrete Reinforcing
 3. Anchorage and Embeds
 4. Structural Steel
 5. Deferred Design Items
 - C. Quality control submittals shall be submitted to special inspector for review prior to fabrication/installation. Courtesy copies shall be provided to the project architect and engineer for their records.
 - D. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings. The structural drawings shall be used in conjunction with the architectural and other consultants' drawings. See the Architectural Drawings for dimensions, doors, non-bearing interior and exterior walls, elevations, slopes, stairs, curbs, drains, recesses, doorframes, railings, waterproving, finishes, chamfers, kerfs, etc.
 - E. Shop drawings made from reproductions of the structural drawings will be rejected unless the contractor signs a release agreement prior to the shop drawings being reviewed. The contractor may also obtain electronic files of the plan sheets after signing a release agreement. Electronic files of the detail sheets and section sheets will not be made available.
 - F. The Contractor may choose to submit shop drawings and submittals for review electronically. The Contractor may do this provided a minimum of one hard copy set is submitted to the project. The submittal will be stamped as received by Dunn Associates, Inc. when the hard copy is received by our office. Hard copies of small submittals need not be submitted if the Contractor receives the approval for this exception by the Engineer of Record.

BASES OF DESIGN

1. Governing Building Code—International Building Code 2018
2. Risk Category—II
3. Floor Live Loads
 - A. Uniformly Distributed Loads
 1. Offices + Partitions—80 psf (reducible)
 2. Heavy Storage—250 psf (unreducible)
 - B. Start Loading (deferred submittal)—100 psf (unreducible)
4. Concentrated Loads
 1. Office—2000 lbs
5. Roof Live Load (Not concurrent with Roof Snow Load)—20 psf or 300 lbs
6. Seismic Design Criteria
 - A. Mapped Spectral Response Accelerations
 1. 0.2-Second (Short) Period Acceleration—SS = 1.321
 2. 1-Second Acceleration—S1 = 0.469
 - B. Design Spectral Response Accelerations
 1. 0.2-Second (Short) Period Acceleration—SDS = 1.057
 2. 1-Second Acceleration—SD1 = 0.570
 - C. Site Class (Soil Profile)—D - Default
 - D. Seismic Importance Factor—IP = 1.0
 - E. Seismic Design Category—D
 - F. Lateral Force Resisting System(s)—Ordinary Steel Moment Frames
 1. Response Modification Coefficient—R = 3.5
 2. System Overstrength Factor—Do = 3
 3. Deflection Amplification Factor—CD = 3
 4. Design Base Shear—V = CwW + 0.132W, where W is structural weight
 - G. Analysis Procedure—Equivalent Lateral Force
7. Serviceability Criteria
 - A. Interior Seismic/Wind Drift—Δ ≤ 0.02h (h is story height)
 - B. Deflection Limits—Total—Live/Snow/Wind
 - Floors—L/400—L/360
 - Stairs—L/480—L/600(1/4" max)
 - Perimeter—L/600(3/8" max)

BUILDING MAXIMUM STORY DRIFT

Level	Floor to floor height (feet)	Elastic Story Drift		Inelastic Story Drift	
		Story Drift (inch)	Drift Ratio	Story Drift (inch)	Drift Ratio
2	18' - 1"	2.75"	0.0067	8.25"	0.02

Wind Pressure Summary for C&C Zones based Upon Areas Ch 38 Pt 1 (Table 1 of 2)

All wind pressures include a load factor of 0.6

Z	Figure	A =	A =	A =	A =				
0	1	2	3	4	5				
n	e	psf	psf	psf	psf				
1	38.3-2A	9.60	-27.23	9.60	-25.44	9.60	-23.06	9.60	-21.27
1	38.3-2A	9.60	-15.65	9.60	-15.65	9.60	-15.65	9.60	-15.65
2	38.3-2A	9.60	-35.93	9.60	-33.62	9.60	-30.56	9.60	-28.25
3	38.3-2A	9.60	-48.96	9.60	-44.34	9.60	-38.24	9.60	-33.62
4	38.3-1	15.65	-16.95	14.95	-16.26	14.94	-15.34	13.34	-14.65
5	38.3-1	15.65	-28.86	14.95	-19.47	14.94	-17.64	13.34	-16.26

Wind Pressure Summary for C&C Zones based Upon Areas Ch 38 Pt 1 (Table 2 of 2)

All wind pressures include a load factor of 0.6

Z	Figure	A =	A =	A =	A =		
0	1	2	3	4	5		
n	e	psf	psf	psf	psf		
1	38.3-2A	9.60	-19.47	9.60	-17.89	9.60	-17.09
1	38.3-2A	9.60	-13.46	9.60	-10.58	9.60	-9.60
2	38.3-2A	9.60	-25.94	9.60	-22.89	9.60	-22.89
3	38.3-2A	9.60	-29.00	9.60	-22.89	9.60	

GENERAL STRUCTURAL NOTES

SPECIAL INSPECTION, TESTING AND STRUCTURAL OBSERVATION REQUIREMENTS

- Special Inspections and Testing
 - Special inspections and testing as required per the approved construction documents and per IBC Chapter 17 shall be provided for this project unless waived by the Building Official.
 - An independent agency or agencies, employed by the Owner, shall perform the special inspection and testing services required.
 - The special inspection and testing requirements of this section of the General Structural Notes and the special inspection tables serve as the Engineer of Record's statement of special inspections and structural observations required by IBC Chapter 17.
- Contractor Responsibilities (1704.4)
 - Each Contractor responsible for the construction of a main wind or seismic force-resisting system, a designated seismic system, or a wind or seismic force-resisting component listed in the statement of special inspections shall submit a written statement of responsibility to the Building Official and Owner prior to commencing with the work involved. It shall contain acknowledgment of awareness of the special requirements contained in the statement of special inspection.
 - The Contractor shall coordinate and cooperate with all the required inspections, testing, and/or structural observations required for the project.
 - The Contractor shall maintain access to and exposure of the work which requires special inspection or testing.
 - The Contractor shall not proceed with subsequent work until required inspections, testing, and/or structural observations have been provided.
 - The Contractor shall correct all work found to be deficient, and re-test at no additional cost to the Owner.
 - The Contractor shall notify the Engineer of Record at least (7) days prior to any required structural observations.
 - Submit all required documentation to the Special Inspector for review.
- Special Inspector Responsibilities (1704.2)
 - Prior to the start of the construction, each approved agency shall provide written documentation to the Building Official, demonstrating the competence and relevant experience or training of the special inspectors who will perform the special inspections and tests during construction.
 - Special Inspectors shall keep records of their inspections and testing.
 - Inspection reports shall indicate whether the work inspected was or was not completed in conformance to the approved construction documents.
 - Non-conforming work and/or discrepancies shall be brought to the Contractor's immediate attention for correction.
 - The Special Inspector shall notify the Architect/Engineer of any non-conforming work or discrepancies that the Contractor cannot readily correct.
 - Any uncorrected non-conforming work or discrepancies shall be brought to the attention of the Architect/Engineer and the Building Official prior to completion of that phase of the work.
 - Submit the following to the Building Official:
 - Special Inspections and Testing Reports.
 - Certificates of Compliance for:
 - Fabrication of structural elements from approved fabricators.
 - The seismic qualifications of nonstructural components, supports, and attachments.
 - Reports of:
 - Pre-construction tests for shotcrete.
 - Materials properties verifying compliance with the requirements of AWS D1.4 for weldability for reinforcing bars other than ASTM A706.
 - Mill tests for ASTM A615 reinforcing bars used to resist earthquake induced forces in special moment frames, special structural walls or coupling beams in structures assigned to Seismic Design Category B, C, D, E, or F.
- Special Inspections (1705)
 - Special Cases (1705.1.1): Special inspection and tests shall be required for proposed work that is, in the opinion of the Building Official, unusual in its nature, such as, but not limited to, the following:
 - Construction materials and systems that are alternatives to materials and systems prescribed by the IBC.
 - Unusual design applications of materials described in the IBC.
 - Materials and systems required to be installed in accordance with additional manufacturer's instructions that prescribe requirements not contained in the IBC or in standards referenced by the IBC.
 - Steel Construction (1705.2): The special inspections and non-destructive testing of steel construction in buildings shall be in accordance with the following:
 - Structural Steel. Special inspections and non-destructive testing of structural steel elements in buildings, structures, and portions thereof shall be in accordance with the Quality Assurance inspection requirements of AISC 360 and tables in the statement of special inspection documents. Exception: Railing systems composed of structural steel elements shall be limited to welding inspection of welds at the base of cantilevered rail posts.
 - Cold-Formed Steel Deck. Special inspections and qualification of welding special inspectors for cold-formed steel floor and roof deck shall be in accordance with the Quality Assurance inspection requirements of SDI/AQCC.
 - Quality Control Submittals for Structural Steel
 - Provide Level III non-destructive testing (NDT) personnel certifications.
 - Provide welder qualification records to verify project welders are tested and qualified in accordance with AWS D1.1 before welding structural or miscellaneous steels, D1.3 before welding sheet steels (1/8" gage and thinner), and D1.4 before welding reinforcing steel. Submit documentation to the approved inspection agency for review before welding.
 - Special, restricted welder qualification testing is required for welders joining the bottom-flange through the weld access hole connection in demand critical welds. Qualify welders for the minimum groove angle and maximum deposition rate used in production. Follow the instructions for supplemental qualification testing in AWS D1.8, Section 5.1. Qualification testing must take place within two years from the start of this project.
 - Provide welder identification methodology. The fabricator/erector shall maintain a system by which the welder who has welded a joint or member can be identified. Stamps, if used, shall be the low stress type.
 - Provide welding procedures that comply with AWS D1.1, D1.3, D1.4, D1.8, as required by the project. Welding procedures shall be made available to welders and inspectors.
 - Provide weld filler metal product data sheets identifying optimum welding parameters and storage conditions with each welding procedure submittal.
 - Identify the maximum welding heat input per inch of weld (KJ/in) permitted by the welding procedures.
 - Identify the maximum deposition rate that will be used while welding on any demand critical weld.
 - Provide typical welding filler metal Certificates of Conformance that identify the WPS Heat Input Envelope.
 - Provide a preliminary welding repair procedure to follow should welding repairs be required within the Seismic Protected Zone.
 - Provide bolt storage and installation procedures to the approved inspection agency for review.
 - Provide mill/material test reports (MTR) or certificates of conformance (CC) that verify compliance of furnished materials to the requirements of the approved contract documents. MTRs or CCs are required for structural shapes, plate, metal deck, fasteners, headed studs, DBAs, weld filler metal, and all assemblies used as primary, load-bearing members. Maintain the heat number traceability of structural shapes and plate used as primary, load-bearing members. Maintain the heat number traceability of structural shapes and plate used as primary, load-bearing members. Maintain the heat number traceability of structural shapes and plate used as primary, load-bearing members.
 - Structural Steel Non-Destructive Testing (NDT) Personnel Qualifications
 - NDT personnel will:
 - Qualify in accordance with the recommended practices of the American Society of Nondestructive Testing, SNT-TC-1A, latest edition.
 - Pass eye examinations meeting: (1) ASTM requirements at least once a year, and (2) AWS D1.1 every three years.
 - Be certified in accordance with the AWS QC-1, latest edition.
 - Level III must be qualified by ASNT testing in the applicable method under review.
 - Only Level II and Level III technicians, qualified by testing in the applicable method, are permitted to interpret nondestructive testing results.
 - Only Senior Certified or Certified Welding Inspectors (SCWI, CWI) are permitted to evaluate welds. Certified Associate Welding Inspectors may evaluate welds when under the direct supervision of a SCWI or CWI.
 - Approved Inspection Agency will certify the following:
 - Level III inspector has reviewed the NDT procedures.
 - Project ultrasonic testing technicians, testing demand critical welds, are trained and qualified in accordance with AWS D1.8, Annex E.
 - That the Inspection Agencies' ultrasonic testing procedures are qualified by weld mockups similar to AWS D1.1, Annex S.
- Structural Steel: Special inspection and non-destructive testing (NDT) are required during the fabrication and erection of any load-bearing members and assemblies. Special inspection, except NDT, may be waived when the work is performed in a fabricating shop, or by an erector approved by the Building Official to perform work without Special Inspection. NDT of welds completed in an approved fabricator's shop may be performed by the fabricator when approved by the Building Official. When the fabricator performs the NDT, the fabricator shall submit the NDT reports for review by the Special Inspector. Special inspection and NDT shall be provided per the special inspection tables for structural steel in the construction documents.
 - Perform all welding and welding special inspection activities in accordance with AWS D1.1, D1.3, D1.4, and D1.8, AISC 360 Chapter N, and AISC 341 Chapter J, as appropriate for the material form and welding methods employed. Approved methods and acceptance criteria are established in these codes.
 - Perform all bolting and bolting inspection activities in accordance with AISC Specification for Structural Steel Joints Using High Strength Bolts, AISC 360 Chapter N, and AISC 341 Chapter J, as applicable.
 - Non-Destructive Testing (NDT) of welds is required as follows:
 - Ultrasonic testing (UT), magnetic particle testing (MT), penetrant testing (PT), and radiographic testing (RT), when required, shall be performed in accordance with AWS D1.1/D1.1M. Acceptance criteria shall be in accordance with AWS D1.1/D1.1M for statically loaded structures, unless otherwise designated on the design drawings or project specifications.
 - All NDT shall be documented. NDT reports shall be distributed to the fabricator/erector, the Building Official, the Contractor, and the Architect.
 - Amount of NDT is permitted to be reduced according to AISC 360 Chapter N and AISC 341 Chapter J if appropriate criteria are met, and if approved by the Building Official and the Engineer of Record.
 - Requirements for structures in Seismic Design Categories C thru F:
 - Ultrasonic test all complete joint penetration groove.
 - Magnetic particle test or penetrant test all thermally cut surfaces of access holes for flange or web thicknesses exceeding 2".
 - Magnetic particle test or penetrant test all thermally cut surfaces of beam copes access holes for flange or web thicknesses exceeding 1 1/2" for members of the seismic force resisting system in Seismic Design Categories C thru F. Any crack shall be deemed unacceptable.
- Special Inspections and Testing for Non-Shrink Grout are required as follows:
 - Periodic special inspection verifying the use of required mix design.
 - Samples of non-shrink grout shall be tested for compressive strength at least daily, with additional tests required for each additional 10 bags mixed per day.
- Concrete (1705.3): Special inspections and tests of concrete construction shall be performed in accordance with Table 1705.3 in the construction documents.
 - Special inspections of welding of and qualifications of special inspectors for reinforcing bars shall be in accordance with the requirements of AWS D1.4 for special inspectors and for special inspector qualifications.
 - In the absence of sufficient data or documentation providing evidence of conformance to quality standards for concrete materials, the building official shall require testing in accordance with the appropriate standards and criteria for the materials.

- Sole (1705.6): Special inspections and tests of existing site soil conditions, fill materials and placement, and load-bearing requirements shall be performed in accordance with the approved soils report and Table 1705.6 in the construction documents. The Special Inspector shall verify that during fill placement, proper materials and procedures are used. The approved geotechnical report and the construction documents shall be used to determine compliance.
- Driven Deep Foundations (1705.7): Special inspections and tests shall be performed during installation of driven deep foundation elements as specified in Table 1705.7 in the construction documents. The approved geotechnical report and the construction documents shall be used to determine compliance.
- Helical Pile Foundations (1705.8): Continuous special inspections shall be performed during installation of helical pile foundations. Information recorded shall include installation equipment used, pile dimensions, tip elevations, final depth, final installation torque and other pertinent data as required. The approved geotechnical report and the construction documents shall be used in accordance with the approved construction documents.
- Fabricated Items (1705.10): Where fabrication of structural, load bearing or lateral load-resisting members or assemblies is being conducted on the premises of a fabricator's shop, special inspections of the fabricated items shall be performed during fabrication. Special inspections during fabrication are not required where the work is done on the premises of a fabricator registered and approved by the building official to perform such work without special inspection. At the completion of fabrication, the approved fabricators shall submit a certificate of compliance to the Owner for submission to the Building Official stating that the work was performed in accordance with the approved construction documents.
- See architectural drawings for additional required inspections pertaining to sprayed fire-resistant materials (1705.14), mastic and intumescent fire-resistant coatings (1705.15), EFRS (1705.16), fire resistant penetrations and joints (1705.17), or smoke control systems (1705.18).
- Post-installed Anchors: Special inspections and tests shall be performed during installation of post-installed anchors according to the requirements of the ICC Evaluation Report and table 1705.3 in the construction documents.
- Special Inspections for Seismic Resistance (1705.12): Special inspections for seismic resistance are required for this project per IBC section 1705.12.
 - Structural Steel (1705.12.1): Special Inspections for seismic resistance shall be in accordance with the following as applicable:
 - Seismic Force-Resisting Systems. Special Inspections of structural steel in the seismic force-resisting systems of buildings and structures assigned to Seismic Design Category B, C, D, E, or F shall be performed in accordance with the Quality Assurance requirements of AISC 341 and the construction documents.
 - Structural Steel Elements. Special Inspections of structural steel elements in the seismic force-resisting systems of buildings and structures assigned to Seismic Design Category B, C, D, E, or F other than those covered in section A.1.1 above, including struts, collectors, chords, and foundation elements, shall be performed in accordance with the Quality Assurance requirements of AISC 341 and the construction documents.
 - Designated Seismic Systems (1705.12.4): For structures assigned to Seismic Design Category C, D, E, or F, the Special Inspector shall examine designated seismic systems including seismic qualifications in accordance with ASCE7 Section 13.2.2, and verify that label, anchorage and mounting conforms to the certificate of compliance.
 - Architectural Components (1705.12.5): Periodic special inspection is required for the erection and fastening of exterior cladding, interior and exterior nonbearing walls and interior and exterior veneer in structures assigned to Seismic Design Category D, E, or F.
 - Access Floors. Periodic special inspection is required for the anchorage of access floors in structures assigned to Seismic Design Category D, E, or F.
 - Plumbing, Mechanical, and Electrical Components (1705.12.6): Periodic special inspection of plumbing, mechanical, and electrical components shall be required for the following:
 - Anchorage of electrical equipment for emergency and standby power systems in structures assigned to Seismic Design Category C, D, E, or F.
 - Anchorage of other electrical equipment is structures assigned to Seismic Design Category E or F.
 - Installation and anchorage of piping systems designed to carry hazardous materials and their associated mechanical units in structures assigned to Seismic Design Category C, D, E, or F.
 - Installation and anchorage of ductwork designed to carry hazardous materials in structures assigned to Seismic Design Category C, D, E, or F.
 - Installation and anchorage of vibration isolated systems in structures assigned to Seismic Design Category C, D, E, or F where the approved construction documents require a nominal clearance of 1/4" or less between the equipment support frame and restraint.
 - Installation of mechanical and electrical equipment, including duct work, piping systems, and their structural supports, where automatic fire sprinkler systems are installed in structures assigned to Seismic Design Category C, D, E, or F to verify proper clearances have been maintained. Where flexible sprinkler hose fittings are used, special inspection of minimum clearances is not required.
- Testing for Seismic Resistance (1705.13): Testing for seismic resistance is required for this project per IBC section 1705.13.
 - Structural Steel (1705.13.1):
 - Seismic Force-Resisting Systems. Non-destructive testing of structural steel in the seismic force-resisting systems of structures assigned to Seismic Design Category B, C, D, E, or F shall be performed in accordance with the Quality Assurance requirements of AISC 341.
 - Structural Steel Elements. Non-destructive testing of structural steel elements in the seismic force-resisting systems of structures assigned to Seismic Design Category B, C, D, E, or F shall be performed in accordance with the Quality Assurance requirements of AISC 341.
 - Non-Structural Components (1705.13.2): For structures assigned to Seismic Design Category B, C, D, E, or F where the requirements of ASCE 7 Section 13.2.1 for nonstructural components, supports or attachments are met by seismic qualification as specified in Item #2 therein, the registered design professional of the applicable discipline shall specify on the approved construction documents the requirements for seismic qualification by analysis, testing, or experience data. Certificates of Compliance for the seismic qualification shall be submitted to the Building Official.
- Special Inspections for Wind Resistance (1705.11): Special inspections for wind resistance are not required for this project per IBC Section 1705.11.
- Structural Observations/Site Observations (1704.6): Structural observations are not required for this project per IBC section 1704.6.
 - Site Observations are part of the Dunn Associates, Inc. contract with the Architect/Owner. The stages of construction listed below will serve as suggested stages of construction to be observed. The Contractor shall notify (in writing) the Engineer of Record at least 7 days prior to the following stages of construction so that the Engineer may have the opportunity to review the work.
 - Initial placing of any concrete, including but not limited to footings, slabs on grade or concrete over steel deck
 - Initial erection of structural steel
 - Initial finish work
 - Structural observation/Site observation reports will be provided to the Architect, Distribution to the Contractor, Owner, and/or Building Official will be through the Architect.
- Seismic/Wind Main Force Resisting Systems That Require Special Inspections
 - Steel Moment Frames
 - Composite Steel and Concrete Deck Diaphragms

ABBREVIATIONS

AB	Anchor Bolt	JST	Joint
ABV	Above	K	Kip(s) = 1000 Pounds
ALT	Alternate	KLF	Kips Per Linear Foot
ARCH	Architect	KSF	Kips Per Square Foot
ADDL	Additional		
BB	Bottom Bar	LB	Pounds (#)
BLDG	Building	LOC	Location
BLKG	Blocking		
BLW	Below	MAX	Maximum
BM	Beam	MECH	Mechanical
BOTT	Bottom	MEZZ	Mezzanine
BRDG	Bridging	MFB	Moment Frame Beam
BRG	Bearing	MFC	Moment Frame Column
BTWN	Between	MFR	Manufacturer
BYND	Beyond	MIN	Minimum
		MISC	Miscellaneous
		MTL	Metal
CANT	Cantilevered		
CJ	Control Joint		
CJP	Complete Joint Penetration	NTS	Not To Scale
CL	Center Line	NS	Non-shrink
COL	Column		
CONC	Concrete	OC	On Center
CONN	Connection	OPG	Opening
CONT	Continuous	OPP	Opposite
COORD	Coordinate		
CTR	Center	PAF	Power Actuated Fastener
		PCF	Pounds per Cubic Foot
DB	Deck Bearing	PEN	Penetrate or Penetration
DBA	Deformed Bar Anchor	PERP	Perpendicular
DBL	Double	PJP	Partial Joint Penetration
DCW	Demand Critical Weld	PL	Plate
DET	Detail	PLF	Pounds per Linear Foot
DIAM	Diameter	PRFAB	Prefabricated
DIM	Dimension	PSF	Pounds per Square Foot
DWG	Drawing	PSI	Pounds per Square Inch
(E)	Existing	REIN	Reinforce
EA	Each	REQD	Required
EF	Each Face	RTU	Roof Top Unit
EL	Elevation		
ELEC	Electrical	SCHED	Schedule
ENGR	Engineer	SFRS	Seismic Force Resisting System
EQ	Equal	SIM	Similar
EQUIP	Equipment	SOG	Slab on Grade
EQ SP	Equally Spaced	STD	Standard
EJ	Each Way	STIFF	Stiffener
EJ	Expansion Joint	STL	Steel
EXT	Exterior	STRUCT	Structural
FLR	Floor	T&B	Top and Bottom
FND	Foundation	TB	Top Bar
FTG	Footing	TEMP	Temperature
		THRU	Through
Ø	Gage	TJ	Top of
GALV	Galvanized	TYP	Typical
GSN	General Structural Notes		
		UNO	Unless Noted Otherwise
HORIZ	Horizontal		
HSA	Headed Stud Anchor	VERT	Vertical
HSS	Heavy Structural Section		
IBC	International Building Code	W/	With
ICC	International Code Council	WWR	Welded Wire Reinforcement
INT	Interior	WP	Working Point

DEFINITION OF INSPECTION TASK ABBREVIATIONS

O	Observe: The inspector shall observe these functions on a random, daily basis. Operations need not be delayed pending observations.
P	Perform: These inspections shall be performed prior to the final acceptance of the item.
D	Document: The inspector shall prepare reports indicating that the work has been performed in accordance with the contract documents. The report need not provide detailed measurements for joint fit-up, WPS settings, completed welds, or other individual items listed in the tables. For shop fabrication, the report shall indicate the piece mark of the piece inspected. For field work, the report shall indicate the reference grid lines and floor or elevation inspected. Work not in compliance with the contract documents and whether the noncompliance has been satisfactorily repaired shall be noted in the inspection report.

TABLE N5.6-1 COMBINED WITH TABLE J7-1 INSPECTION TASKS PRIOR TO BOLTING

	AISC 360	AISC 341	VISUAL INSPECTION TASKS PRIOR TO BOLTING		QC		QA	
			TASK	DOC.	TASK	DOC.		
1.	•		Manufacturer's certifications available for fastener materials	O	-	P	-	
2.	•		Fasteners marked in accordance with ASTM requirements	O	-	O	-	
3.	•	•	Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane)	O	-	O	-	
4.	•	•	Proper bolting procedure selected for joint detail	O	-	O	-	
5.	•	•	Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements	O	-	O	-	
6.	•	•	Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	P	D	O	D	
7.	•	•	Proper storage provided for bolts, nuts, washers and other fastener components	O	-	O	-	

TABLE N5.6-2 COMBINED WITH TABLE J7-2 INSPECTION TASKS DURING BOLTING

	AISC 360	AISC 341	VISUAL INSPECTION TASKS DURING BOLTING		QC		QA	
			TASK	DOC.	TASK	DOC.		
1.	•	•	Fastener assemblies of suitable condition placed in all holes and washers (if required) and nuts are positioned as required	O	-	O	-	
2.	•	•	Joint brought to the snug-tight condition prior to the pretensioning operation	O	-	O	-	
3.	•	•	Fastener component not turned by the wrench prevented from rotating	O	-	O	-	
4.	•	•	Fasteners are pretensioned in accordance with the RCSC Specification progressing systematically from the most rigid point toward the free edges	O	-	O	-	

TABLE N5.6-3 COMBINED WITH TABLE J7-3 INSPECTION TASKS AFTER BOLTING

	AISC 360	AISC 341	VISUAL INSPECTION TASKS AFTER BOLTING		QC		QA	
			TASK	DOC.	TASK	DOC.		
1.	•	•	Document acceptance or rejection of bolted connections	P	D	P	D	



GerouxArchitects@gmail.com

Subconsultants

Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Van Boerum & Frank Assoc.
181 E. 5600 S.
Murray, Utah 84107

Hunt Electric, Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

Issued/Revisions

No.	Description	Date

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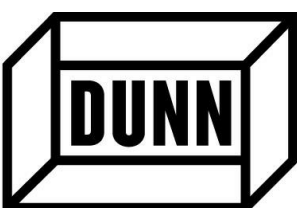
Project Name
ALBANY MEZZANINE

Sheet Title
GENERAL STRUCTURAL NOTES

Scale
Date
2024.02.02

Drawn
Project No.
JDD 230103

Sheet No.
S-002



DUNN ASSOCIATES, INC
Consulting Structural Engineers

WWW.DUNN-SE.COM
PH: 801-575-8877 FAX: 801-575-8875

2024.02.02
PROGRESS SET

NOTE:
THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL MODEL DATED 03.16.2023

DIMENSIONS AND ELEVATIONS, AS THEY RELATE TO THE BUILDING IN GENERAL, IS A GRID TO GRID. DIMENSIONS ON DECK BEARING ELEVATIONS ARE SUPPLIED BY THE ARCHITECT. THEY ARE PROVIDED ON THE STRUCTURAL PLANS AND DETAILS FOR THE CONVENIENCE OF THE CONTRACTOR. VERIFY DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS.

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Project Name
ALBANY MEZZANINE

Sheet Title
GENERAL STRUCTURAL NOTES

<small>Date</small>	2024.02.02
<small>Project No.</small>	230103

S-003

GENERAL STRUCTURAL NOTES

TABLE 1705.6: REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	-	X
2. Verify excavations are extended to proper depth and have reached proper material.	-	X
3. Perform classification and testing of compacted fill materials.	-	X
4. Verify use of proper materials, densities and lift thickness during placement and compaction of compacted fill.	X	-
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	-	X

TABLE 1705.7: REQUIRED SPECIAL INSPECTIONS AND TESTS OF DRIVEN DEEP FOUNDATION ELEMENTS

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. Verify element materials, sizes and lengths comply with the requirements.	X	-
2. Determine capacities of test elements and conduct additional load tests, as required.	X	-
3. Inspect driving operations and maintain complete and accurate records for each element.	X	-
4. Verify placement locations and plumbness, confirm type and size of hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any damage to foundation element.	X	-
5. For steel elements, perform additional special inspections in accordance with Section 1705.2.	-	-
6. For concrete elements and concrete-filled elements, perform tests and additional special inspections in accordance with Section 1705.3.	-	-
7. For specialty elements, perform additional inspections as determined by the registered design professional in responsible charge.	-	-

TABLE J9-1

INSPECTION OF COMPOSITE STRUCTURES PRIOR TO CONCRETE PLACEMENT

	QC		QA	
	TASK	DOC.	TASK	DOC.
Material identification of reinforcing steel (Type/Grade)	0	-	0	-
Determination of carbon equivalent for reinforcing steel other than ASTM A706	0	-	0	-
Proper reinforcing steel size, spacing and orientation	0	-	0	-
Reinforcing steel has not been bent in the field	0	-	0	-
Reinforcing steel has been tied and supported as required	0	-	0	-
Required reinforcing steel clearances have been provided	0	-	0	-
Composite member has required size	0	-	0	-

TABLE J9-2

INSPECTION OF COMPOSITE STRUCTURES DURING CONCRETE PLACEMENT

	QC		QA	
	TASK	DOC.	TASK	DOC.
Concrete: Material identification (mix design, compressive strength, maximum large aggreg size, maximum slump)	0	D	0	D
Limits on water added at the truck or pump	0	D	0	D
Proper placement techniques to limit segregation	0	-	0	-

TABLE J9-3

INSPECTION OF COMPOSITE STRUCTURES AFTER CONCRETE PLACEMENT

	QC		QA	
	TASK	DOC.	TASK	DOC.
Achievement of minimum specified concrete compressive strength at specified age	-	D	-	D

TABLE J8-1

OTHER INSPECTION TASKS

	QC		QA	
	TASK	DOC.	TASK	DOC.
RBS requirements, if applicable • Contour and finish • Dimensional tolerance	P	D	P	D
Protected zone - no holes and unapproved attachments made by fabricator or erector, as applicable	P	D	P	D

TABLE N5.4-1 COMBINED WITH TABLE J6-1 INSPECTION TASKS PRIOR TO WELDING

AISC 360	AISC 341	VISUAL INSPECTION TASKS PRIOR TO WELDING	QC		QA	
			TASK	DOC.	TASK	DOC.
•	•	Welder qualification records and continuity records	P	-	0	-
•	•	Welding procedure specification (WPSs) available	P	-	P	-
•	•	Manufacturer certification for welding consumables available	P	-	P	-
•	•	Material identification (type/grade)	0	-	0	-
•	•	Welder identification system ^a	0	-	0	-
•	•	Fit-up of groove welds (including joint geometry) • Joint preparation • Dimensions (alignment, root opening, root face, bevel) • Cleanliness (condition of steel surfaces) • Tacking (tack weld quality and location) • Backing type and fit (if applicable)	P/O**	-	0	-
•	•	Fit-up of CJP groove welds of HSS T-, Y-, and K-joints without backing (including joint geometry) • Joint preparation • Dimensions (alignment, root opening, root face, bevel) • Cleanliness (condition of steel surfaces) • Tacking (tack weld quality and location)	P	-	0	-
•	•	Configuration and finish of access holes	0	-	0	-
•	•	Fit-up of fillet welds • Dimensions (alignment, gaps at root) • Cleanliness (condition of steel surfaces) • Tacking (tack weld quality and location)	P/O**	-	0	-
•	•	Check welding equipment	0	-	0	-

^aThe fabricator or erector, as applicable, shall maintain a system by which a welder who has welded a joint or member can be identified. Stamps, if used, shall be low stress type.

** Follow performance of this inspection task for ten welds to be made by a given welder, with the welder demonstrating understanding of requirements and possession of skills and tools to verify these items, the Perform designation of this task shall be reduced to Observe, and the welder shall perform this task. Should the inspector determine that the welder has discontinued performance of this task, the task shall be returned to Perform until such time as the inspector has re-established adequate assurance that the welder will perform the inspection tasks listed.

TABLE N5.4-2 COMBINED WITH TABLE J6-2 VISUAL INSPECTION TASKS DURING WELDING

AISC 360	AISC 341	VISUAL INSPECTION TASKS DURING WELDING	QC		QA	
			TASK	DOC.	TASK	DOC.
•	•	WPS followed • Settings on welding equipment • Travel speed • Selected welding materials • Shielding gas type/flow rate • Preheat applied • Interpass temperature maintained (min/max) • Proper position (F, V, H, OH) • Intermix of filler metals avoided unless approved	0	-	0	-
•	•	Use of qualified welders	0	-	0	-
•	•	Control and handling of welding consumables • Packaging • Exposure control	0	-	0	-
•	•	No welding over cracked tack welds	0	-	0	-
•	•	Environmental conditions • Wind speed within limits • Precipitation and temperature	0	-	0	-
•	•	Welding techniques • Interpass and final cleaning • Each pass within profile limitations • Each pass meets quality requirements	0	-	0	-
•	•	Placement and installation of steel headed stud anchors	P	-	P	-

TABLE N5.4-3 COMBINED WITH TABLE J6-3 VISUAL INSPECTION TASKS AFTER WELDING

AISC 360	AISC 341	VISUAL INSPECTION TASKS AFTER WELDING	QC		QA	
			TASK	DOC.	TASK	DOC.
•	•	Welds cleaned	0	-	0	-
•	•	Size, length and location of welds	P	-	P	-
•	•	Welds meet visual acceptance criteria • Crack prohibition • Weld base-metal fusion • Crater cross section • Weld profiles and size • Undercut • Porosity	P	D	P	D
•	•	Arc strikes	P	-	P	-
•	•	k-area ¹	P	D	P	D
•	•	Weld acceptance or rejection of welded joint or member	P	-	P	-
•	•	Placement of reinforcing or contouring fillet welds (if required)	P	D	P	D
•	•	Backing removed, weld tabs removed and finished, and fillet welds added (if required)	P	D	P	D
•	•	Repair activities	P	-	P	D
•	•	Document acceptance or rejection of welded joint or member	P	D	P	D
•	•	No prohibited welds have been added without the approval of the EOR.	0	-	0	-

¹ When welding of doubler plates, continuity plates or stiffeners has been performed in the k-area, visually inspect the web k-area for cracks within 3 in. (75mm) of the weld.

TABLE 1705.3: REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD ^a	IBC REFERENCE
1. Inspect reinforcement, including prestressing tendons, and verify placement	-	X	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. Reinforcing bar welding a. Verify weldability of reinforcing bars other than ASTM A 706; b. Inspect single-pass fillet welds, maximum 5/16"; and c. inspect all other welds	-	X	AWS D1.4 ACI 318: 26.6.4	-
3. Inspect anchors cast in concrete.	-	X	ACI 318; 17.8.2	-
4. Inspect anchors post-installed in hardened concrete member: a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads b. Mechanical anchors and adhesive anchors not defined in 4.a.	X	-	ACI 318: 17.8.2.4	-
5. Verify use of required design mix.	-	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	-	ASTM C 172 ASTM C 31 ACI 318: 26.5, 26.12	1908.10
7. Inspect concrete and shotcrete placement for proper application techniques.	X	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. Verify maintenance of specified curing temperature and techniques.	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9. Inspect prestressed concrete for: a. Application of prestressing forces; and b. Grouting of bonded prestressing tendons.	X	-	ACI 318: 26.10	-
10. Inspect erection of precast concrete members.	-	X	ACI 318: Ch. 26.9	-
11. Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	-	X	ACI 318: 26.11.2	-
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	-	X	ACI 318: 26.11.1.2(b)	-

For SI: 1 inch = 25.4 mm.

a. Where applicable, see Section 1705.12. Special inspection for seismic resistance.
b. Specific requirements for special inspection shall be included in the research report for the anchor issued by an approved source in accordance with 17.8.2 in ACI 318, or other qualification procedures. Where specific requirements are not provided, special inspection requirements shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

No.	Description	Date

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Project Name
ALBANY MEZZANINE

Sheet Title
FOOTING AND FOUNDATION PLAN

Scale
**2024.02.02
PROGRESS SET**

Date
2024.02.02

Project No.
230103

Drawn
JDD

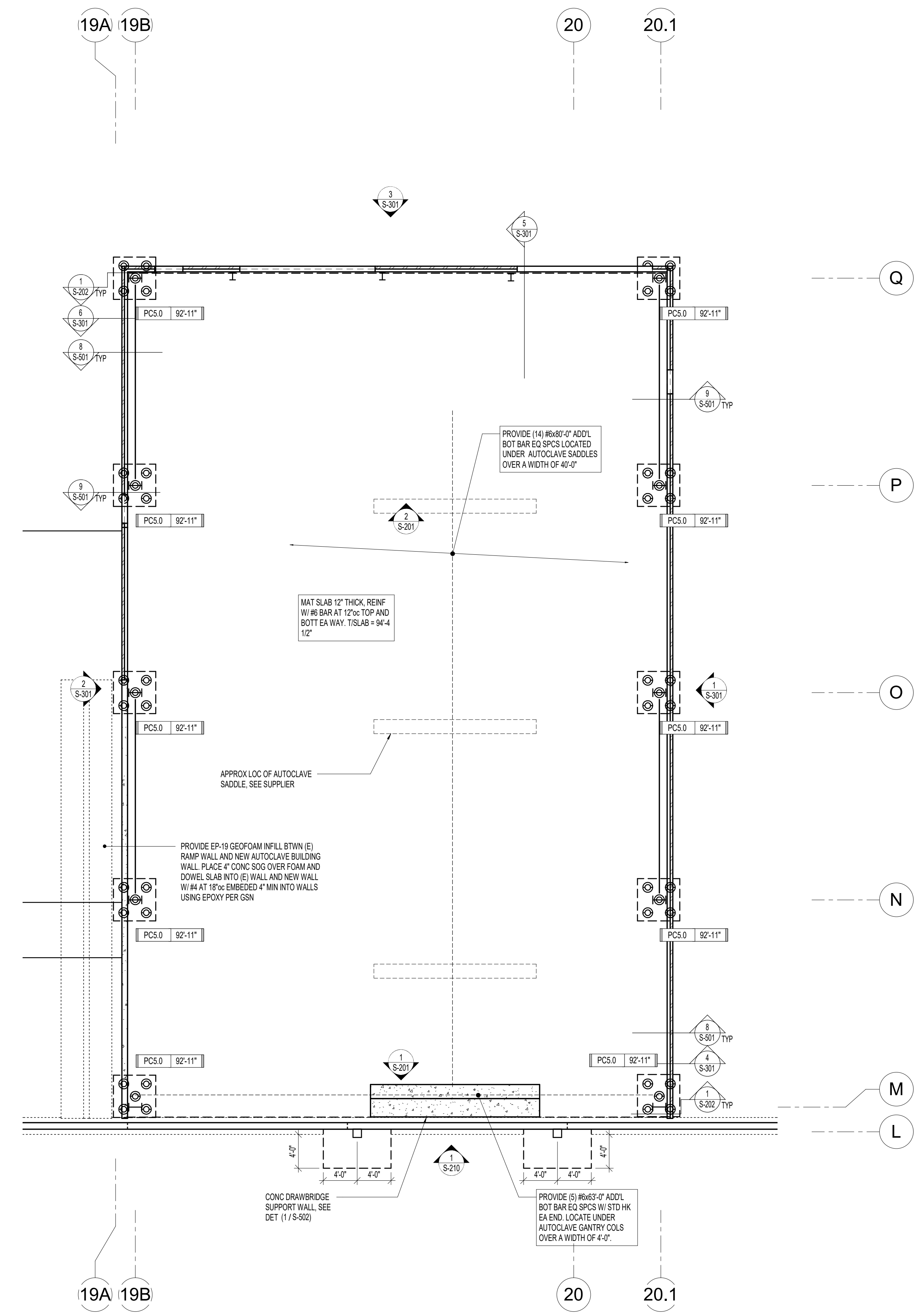
Sheet No.
S-101

FOOTING AND FOUNDATION PLAN NOTES:

- COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
- SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS AND DEPRESSIONS.
- SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
- SEE SCHEDULES ON (S-800) SHEETS FOR:
 - FOOTINGS
 - REINFORCING SPLICE LENGTHS
 - STEEL COLUMNS
 - PILE CAPS
 - PILE CAP LOADING CRITERIA
- SEE TYPICAL FOOTING AND FOUNDATION DETAILS ON (S-500) SHEETS FOR:
 - SLAB CONSTRUCTION AND CONTROL JOINTS
 - FOOTING STEPS
 - CORNER BARS
 - PIPES PERPENDICULAR AND PARALLEL TO FOOTINGS
 - DEPRESSED SLABS
 - REINFORCING AT MISCELLANEOUS OPENING
 - REINFORCING AT SLAB DISCONTINUITIES
 - FROST COVER AND STRUCTURAL FILL
 - FLOOR OFFSETS

MARKS AND SYMBOL LEGEND

	SECTION MARK SHEET NUMBER
	FRAME ELEVATION SHEET NUMBER
	FOOTING DESIGNATION TOP ELEVATION
	DEPRESSED FND WALL POUR SLAB OVER
	FOOTING STEP, SEE DETAILS
	FLOOR OFFSET, SEE DETAILS
	DEPRESSED SLAB, SEE ARCHITECTURAL PLANS FOR EXACT LOCATION AND ELEVATION
	CONCRETE WALL
	STEEL COLUMN
	MICROPILE, BY SUPPLIER
	CONTROL JOINT
	CONCRETE PIER, SEE SCHEDULE
	CONTINUOUS FOOTING, SEE SCHEDULE
	FLOOR DRAIN, SEE ARCHITECTURAL FOR EXACT LOCATION
	GRADE BEAM, SEE DET (-/-)
	SPOT FOOTING, SEE SCHEDULE
	THICKENED SLAB FOOTING, SEE SCHEDULE
	STEEL BASE PLATE, SEE SCHEDULE
	STEEL COLUMN, SEE SCHEDULE
	CONCRETE WALL, SEE SCHEDULE



1 FOOTING AND FOUNDATION PLAN
S-101 SCALE: 1/8" = 1'-0"

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Consulting Structural Engineers

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PH: 801-575-8877 FAX: 801-575-8875

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ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

No.	Description	Date

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Project Name
ALBANY MEZZANINE

Sheet Title
ROOF FRAMING PLAN

Scale
2024.02.02
PROGRESS SET

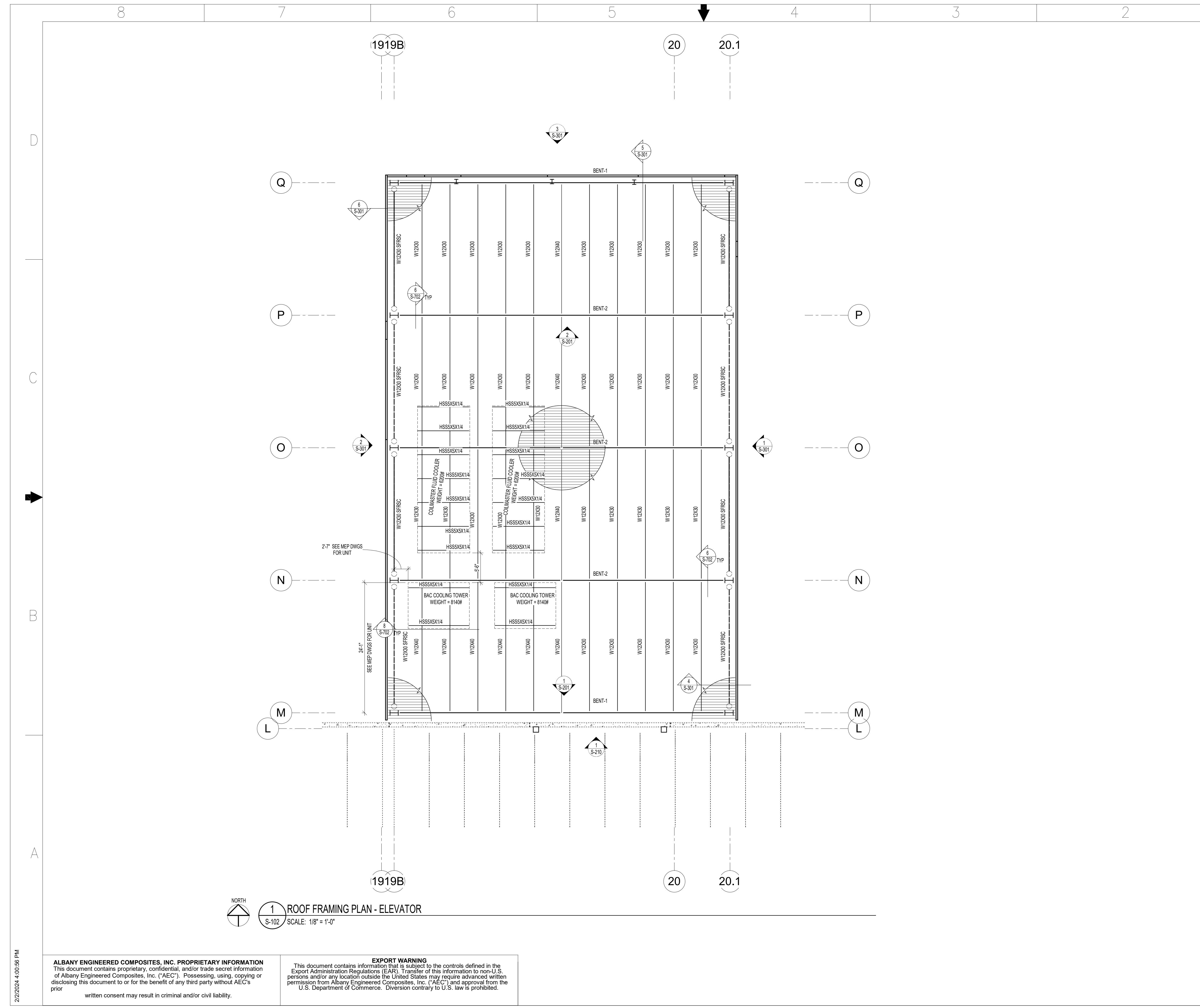
Date
2024.02.02

Project No.
230103

S-102

- FLOOR FRAMING PLAN NOTES:**
1. VERIFY WALL FLOOR OPENINGS FOR MECHANICAL SHAFTS, STAIRS, ETC. WITH ARCHITECTURAL DRAWINGS.
 2. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO COLUMNS, WALLS, SLAB EDGES, SLOPES, ELEVATIONS, CURBS AND DEPRESSIONS.
 3. SEE SCHEDULES ON (S-800) SHEETS FOR:
 - STEEL COLUMNS
 - SINGLE SHEAR BEAM CONNECTIONS, TYPICAL UNLESS NOTED OTHERWISE
 - REINFORCING SPLICE LENGTHS
 4. SEE FLOOR FRAMING DETAILS ON (S-600) SHEETS FOR:
 - FRAMING AROUND MISCELLANEOUS OPENINGS
 - DECK REINFORCEMENT AT OPENINGS
 - DECK BEARING ANGLES AT COLUMNS

- MARKS & SYMBOLS LEGEND**
- SECTION MARK SHEET NUMBER
 - FRAME ELEVATION SHEET NUMBER
 - CONCRETE OVER METAL DECK. SEE GENERAL STRUCTURAL NOTES
 - ADDITIONAL CONC REINF OF (2) #5 BARS x 6'-4" CTR ON CORNERS, MID-HEIGHT OF SLAB. T&B AT SLABS THICKER THAN 7"
 - DEPRESSED SLAB. SEE ARCHITECTURAL PLANS FOR EXACT LOCATION AND ELEVATION
 - CONCRETE WALL
 - (E) CONCRETE WALL
 - STEEL COLUMN
 - (E) STEEL COLUMN
 - LATERAL FRAME MOMENT CONNECTIONS
 - GRAVITY MOMENT CONNECTIONS
 - SFRSC TOP FLANGE CJP WELD CONNECTION. SEE DETS ON (S-200) SHEETS
 - LATERAL BEAM BRACING CONN PER DET (-/-)
 - COMPOSITE STEEL BEAMS. SEE GENERAL STRUCTURAL NOTES
 - NUMBER OF HSA FOR SECTION OF COMPOSITE STEEL BEAM. SEE GENERAL STRUCTURAL NOTES
 - REQUIRED PRE-CAMBER AT MID-SPAN OF BMS. CAMBER TOLERANCE SHALL BE +1/4" -0"
 - DIFFERENCE IN TOP OF BEAM ELEVATION FROM TYPICAL TOP OF FLOOR STEEL
 - SFRSC SEISMIC FORCE RESISTING SYSTEM COLLECTOR. SEE DETS ON SHEET (S-203) FOR CONNECTIONS



1 ROOF FRAMING PLAN - ELEVATOR
SCALE: 1/8" = 1'-0"

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Project Name
ALBANY MEZZANINE

Sheet Title
MOMENT FRAME ELEVATIONS

Scale
**2042.02.02
PROGRESS SET**

Date
2024.02.02

Drawn
JDD

Project No.
230103

Sheet No.
S-201

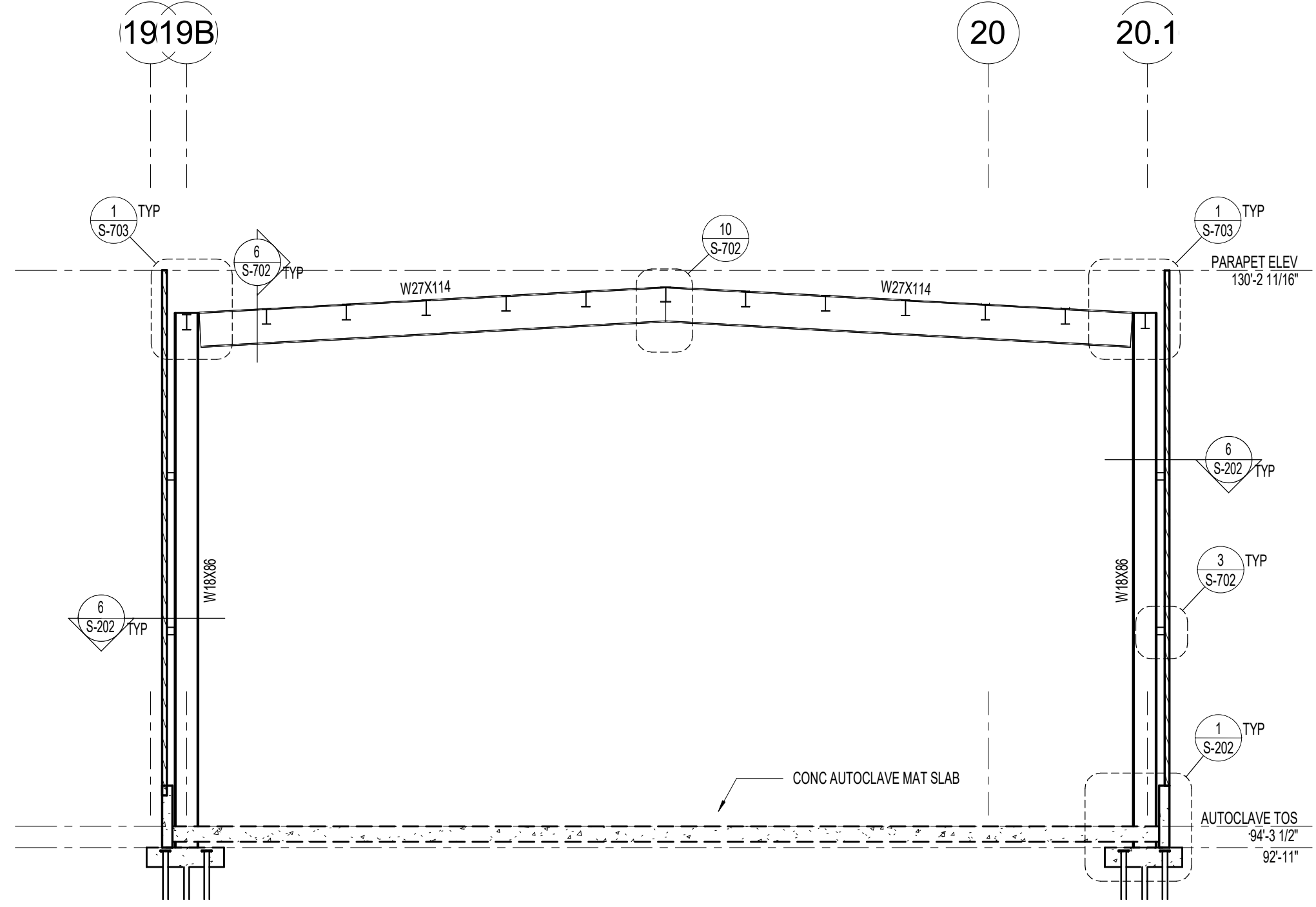
ELEVATION NOTES

1. ALL MEMBERS SIZED ON THIS SHEET ARE PART OF THE MOMENT FRAME. SEE GENERAL STRUCTURAL NOTES AND SPECIFIED DETAILS FOR MORE INFORMATION.
2. WHERE POSSIBLE COORDINATE FULL HEIGHT WEB STIFFENERS WITH PERPENDICULAR BEAM CONNECTIONS. SEE DETAIL (-/-).
3. ALL MOMENT FRAME BEAM TO COLUMN CONNECTIONS ARE REDUCED BEAM CONNECTIONS, SEE SCHEDULE / DETAIL (-/-).
4. PROVIDE 3/4" x 0-5" HSA AT 12" OC TYPICAL AT MOMENT FRAME BEAMS, UNLESS NOTED OTHERWISE.

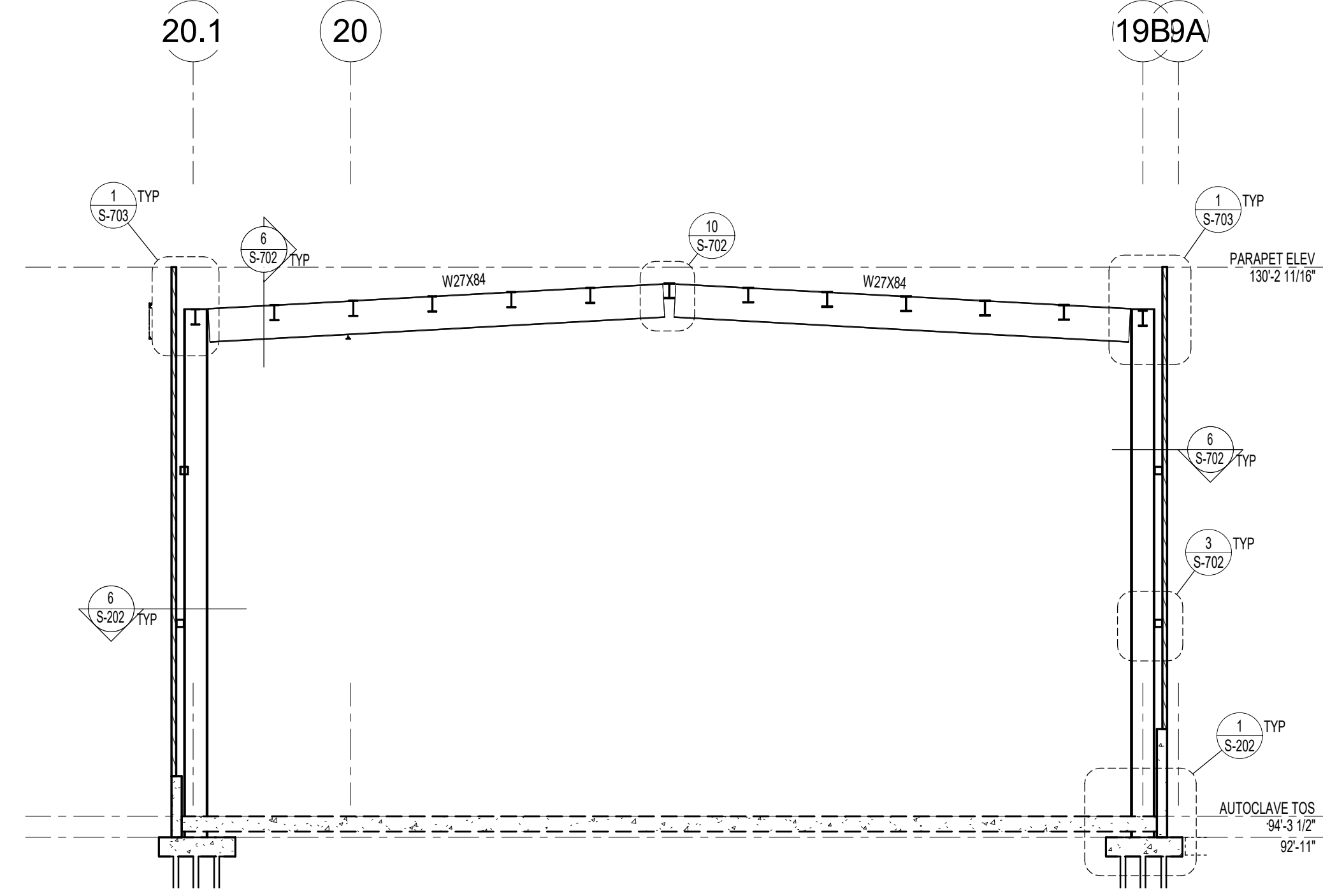
ELEVATION NOTES

SECTION MARK SHEET NUMBER

X SIZE INDICATES CONTINUITY PL THICKNESS
X SIZE INDICATES WEB DOUBLER PL THICKNESS



2 BENT-2 MOMENT FRAME ELEVATION
S-201 NO SCALE



1 BENT-1 MOMENT FRAME ELEVATION
S-201 NO SCALE

8 7 6 5 4 3 2 1

D
C
B
A

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**2024.02.02
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No.	Description	Date

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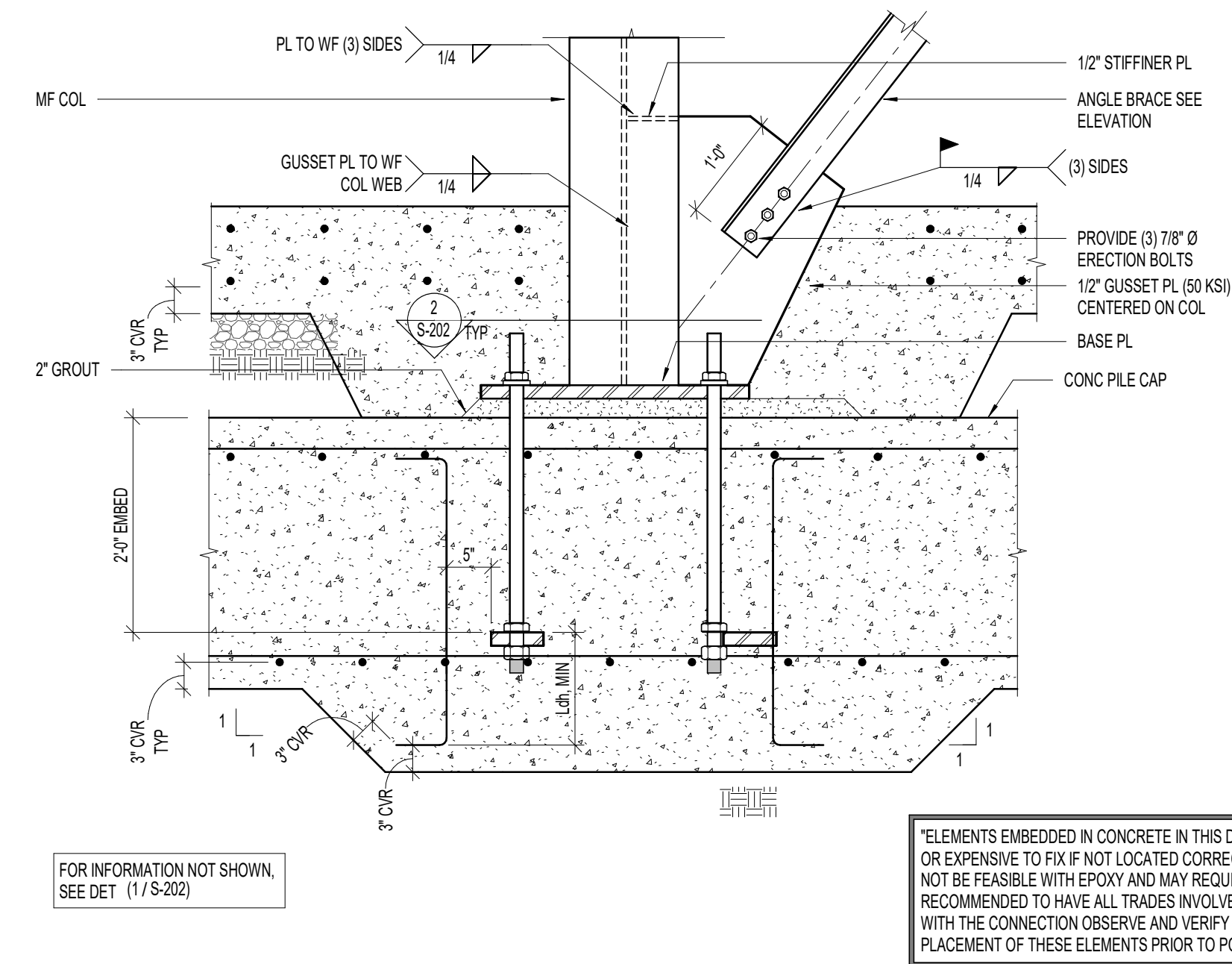
Project Name
ALBANY MEZZANINE

Sheet Title
MOMENT FRAME DETAILS

Scale
2024.02.02
Date
2024.02.02
Drawn
JDD
Project No.
230103

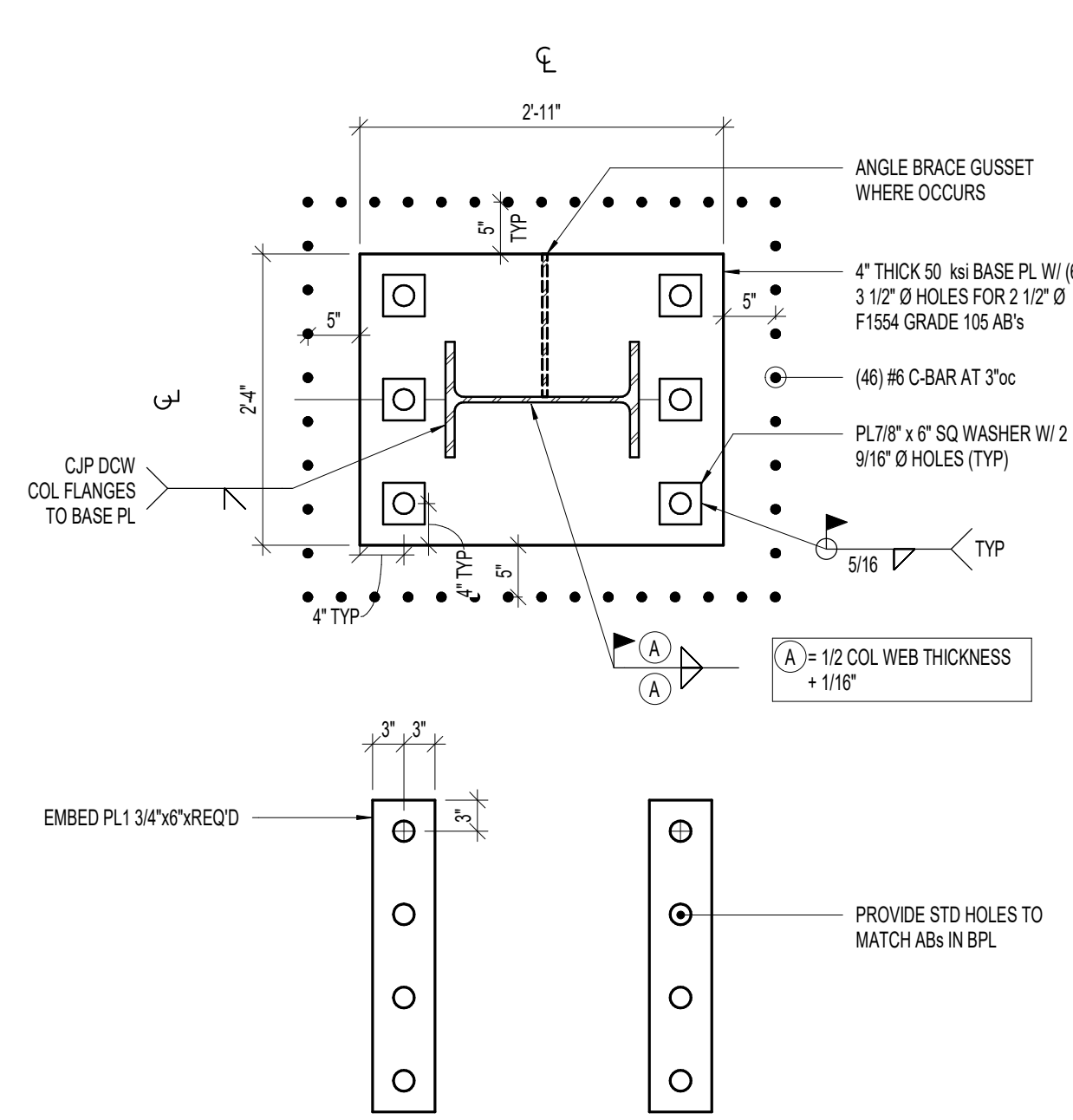
S-202

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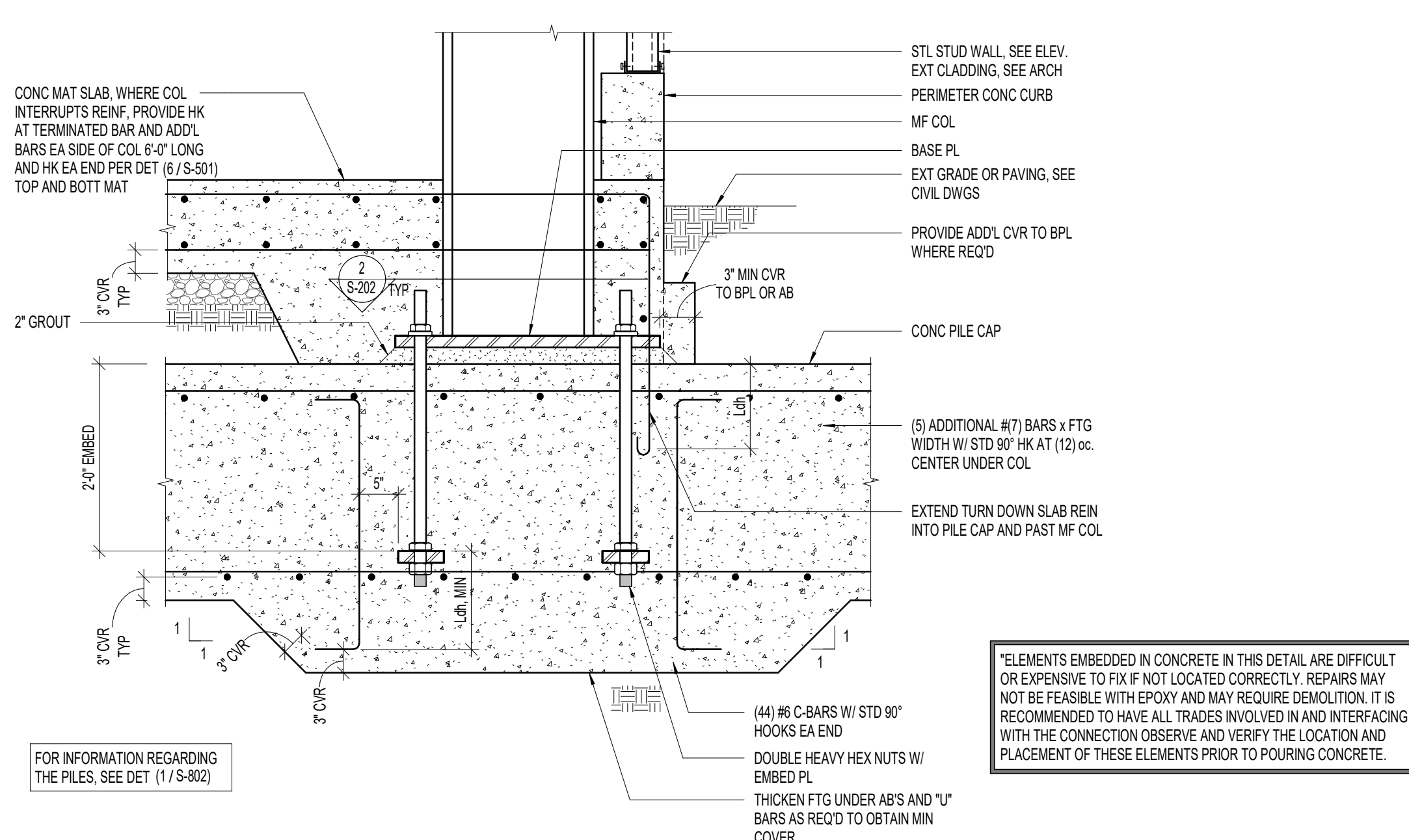
3 TYPICAL ANGLE BRACE TO FOOTING

S-202 NO SCALE: 1/22/24



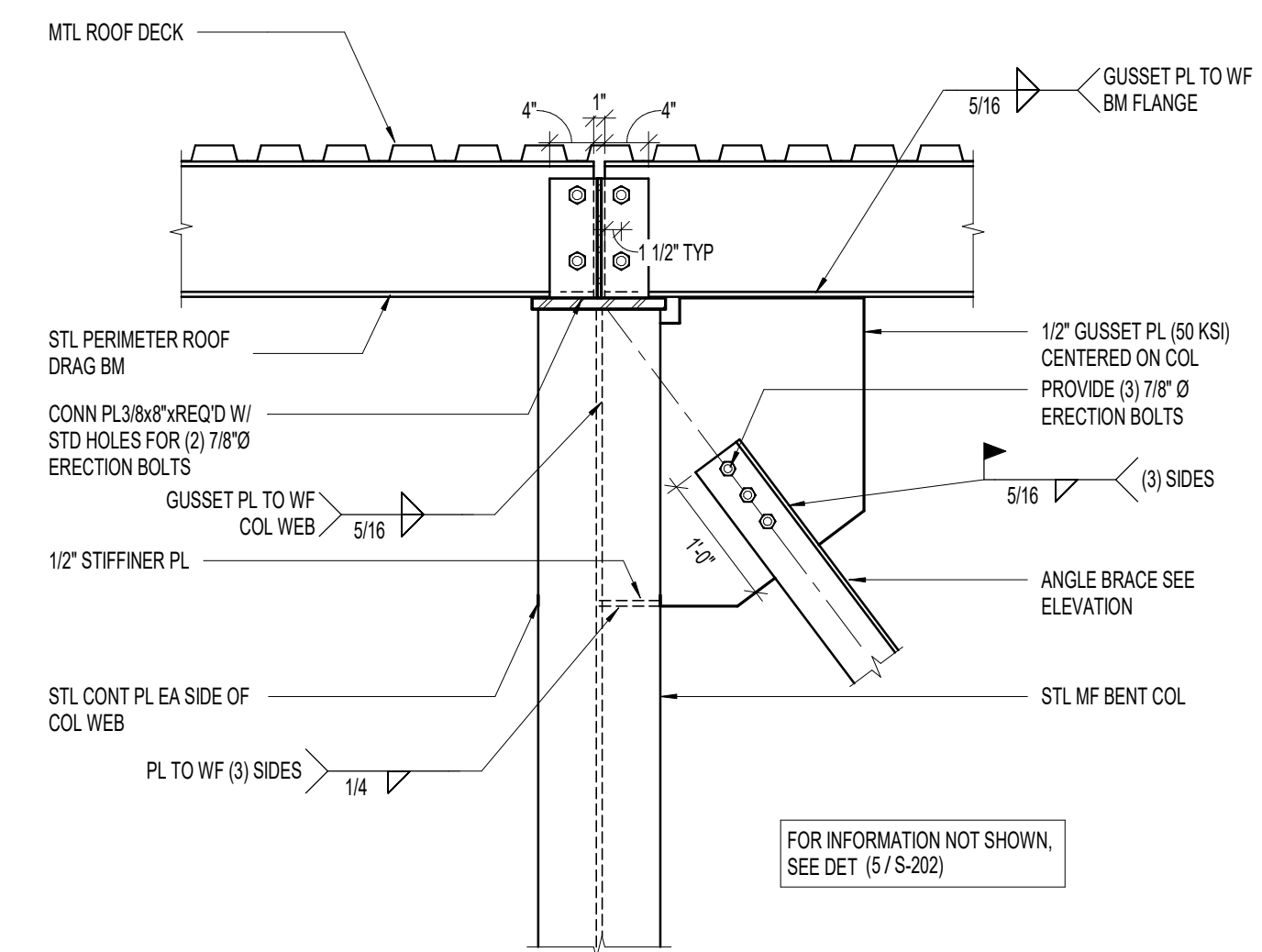
2 MOMENT FRAME COL BASE PLATE AND EMBED TEMPLATE

S-202 NO SCALE: 1/22/24



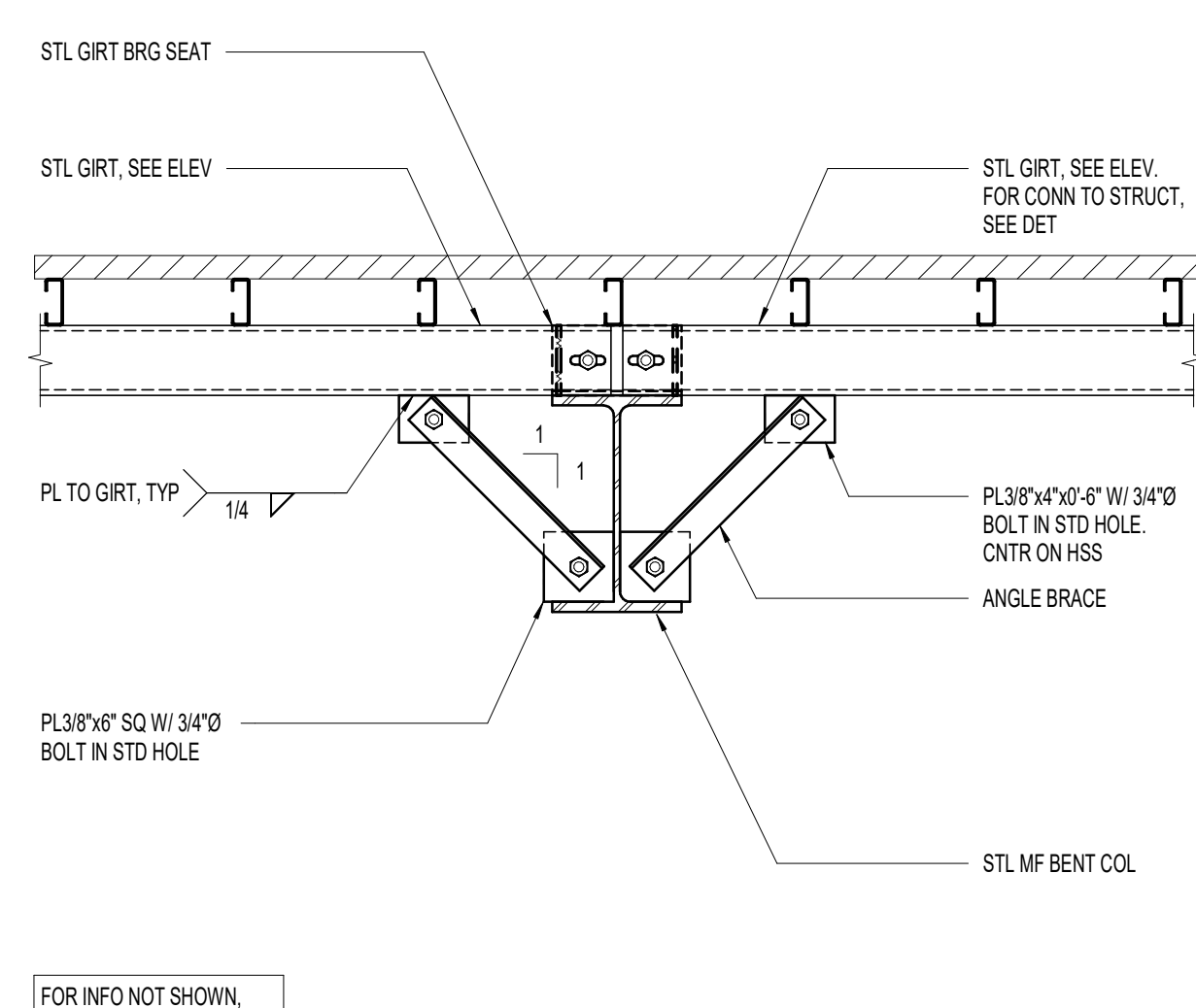
1 MOMENT FRAME BASE TEMPLATE

S-202 NO SCALE: 1/22/24



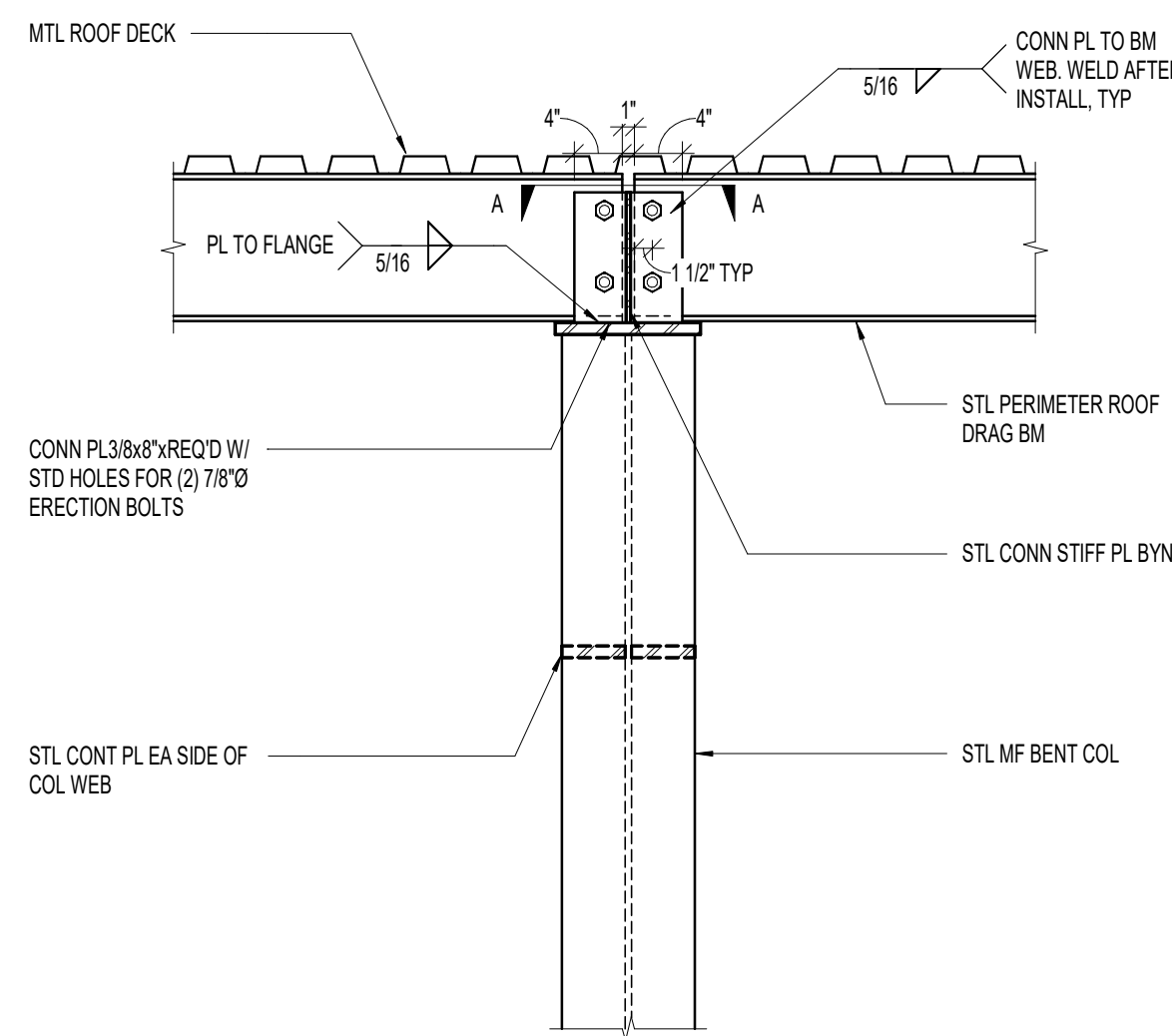
7 TYPICAL PERIMETER ROOF BEAM TO MOMENT FRAME DRAG DETAIL

S-202 NO SCALE: 1/22/24



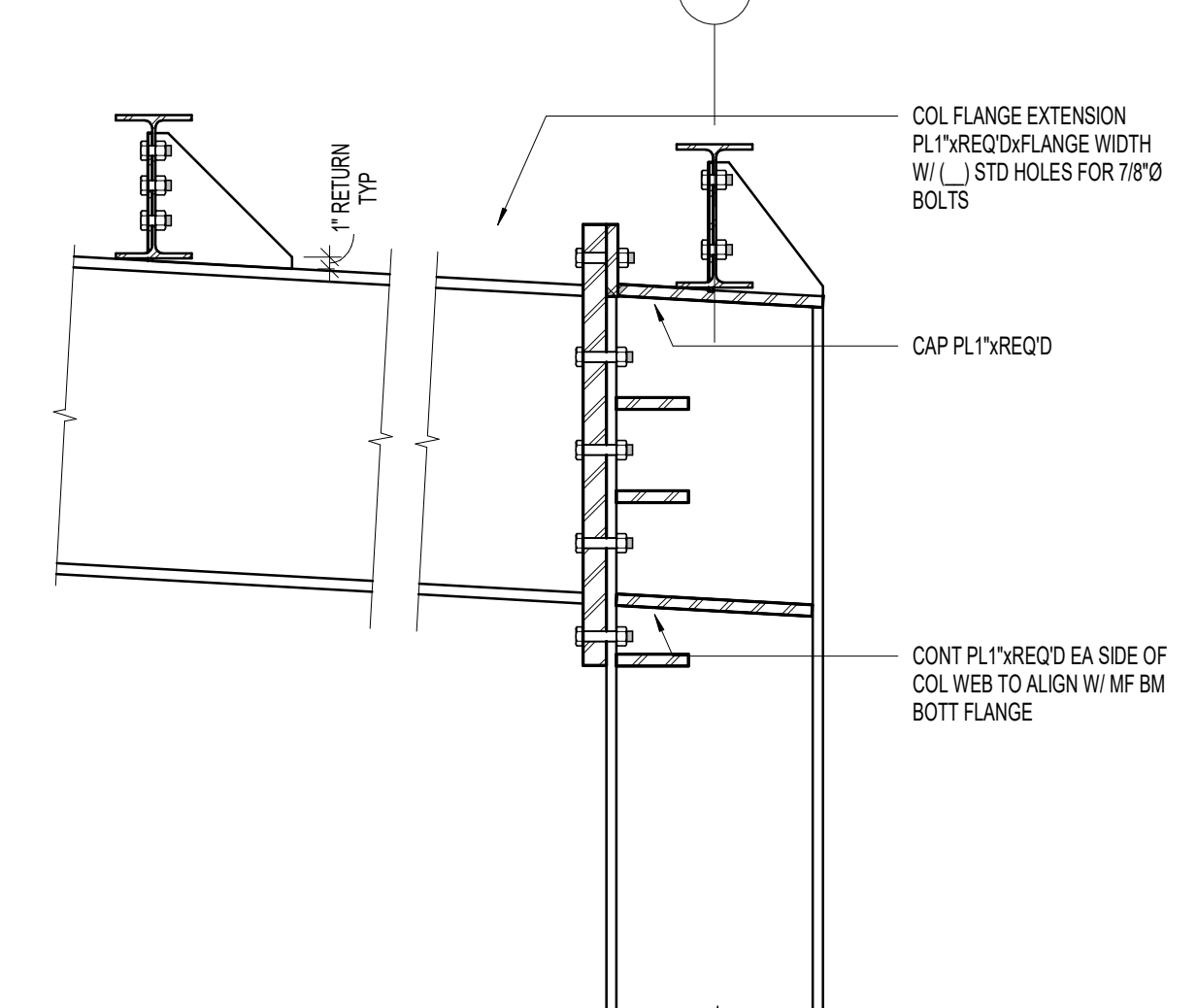
6 TYPICAL COLUMN ANGLE BRACE

S-202 NO SCALE: 1/22/24



5 TYPICAL PERIMETER ROOF BEAM TO MOMENT FRAME DRAG DETAIL

S-202 NO SCALE: 1/22/24



4 TYPICAL MOMENT FRAME BENT TO COLUMN

S-202 NO SCALE: 1/22/24

*ELEMENTS EMBEDDED IN CONCRETE IN THIS DETAIL ARE DIFFICULT OR EXPENSIVE TO FIX IF NOT LOCATED CORRECTLY. REPAIRS MAY NOT BE FEASIBLE WITH EPOXY AND MAY REQUIRE DEMOLITION. IT IS RECOMMENDED TO HAVE ALL TRADES INVOLVED IN AND INTERFACING WITH THE CONNECTION OBSERVE AND VERIFY THE LOCATION AND PLACEMENT OF THESE ELEMENTS PRIOR TO POURING CONCRETE.

*ELEMENTS EMBEDDED IN CONCRETE IN THIS DETAIL ARE DIFFICULT OR EXPENSIVE TO FIX IF NOT LOCATED CORRECTLY. REPAIRS MAY NOT BE FEASIBLE WITH EPOXY AND MAY REQUIRE DEMOLITION. IT IS RECOMMENDED TO HAVE ALL TRADES INVOLVED IN AND INTERFACING WITH THE CONNECTION OBSERVE AND VERIFY THE LOCATION AND PLACEMENT OF THESE ELEMENTS PRIOR TO POURING CONCRETE.

FOR INFORMATION NOT SHOWN, SEE DET (1/S-202)

FOR INFO NOT SHOWN, SEE GIRT CONN DETAILS ON SHEET S702

FOR INFORMATION NOT SHOWN, SEE DET (5/S-202)

Subconsultants
Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Van Boerum & Frank Assoc.
181 E. 5600 S.
Murray, Utah 84107

Hunt Electric, Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

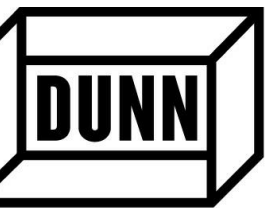
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2024.02.02
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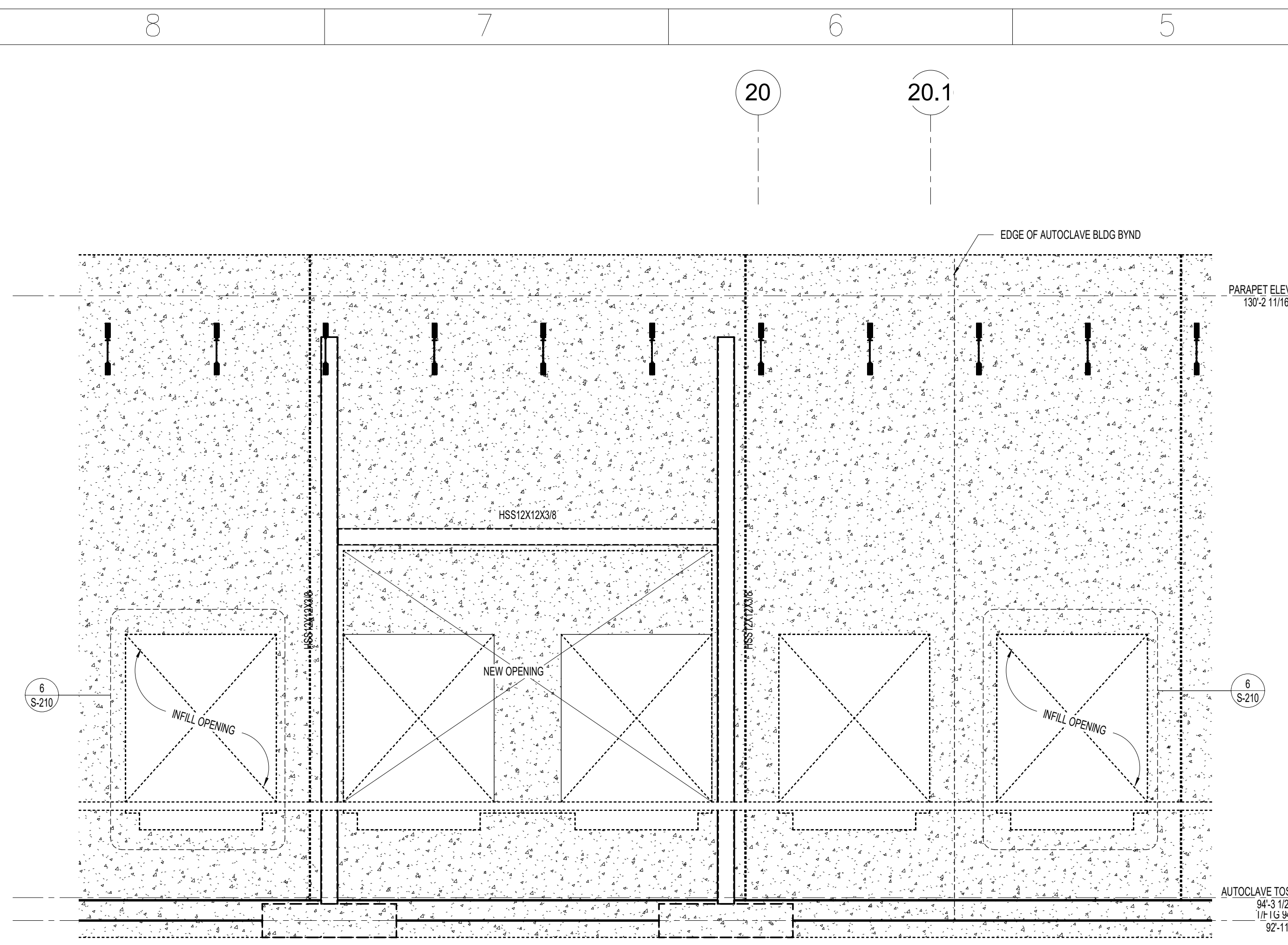
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NEW WALL OPENING ELEVATION AND DETAILS

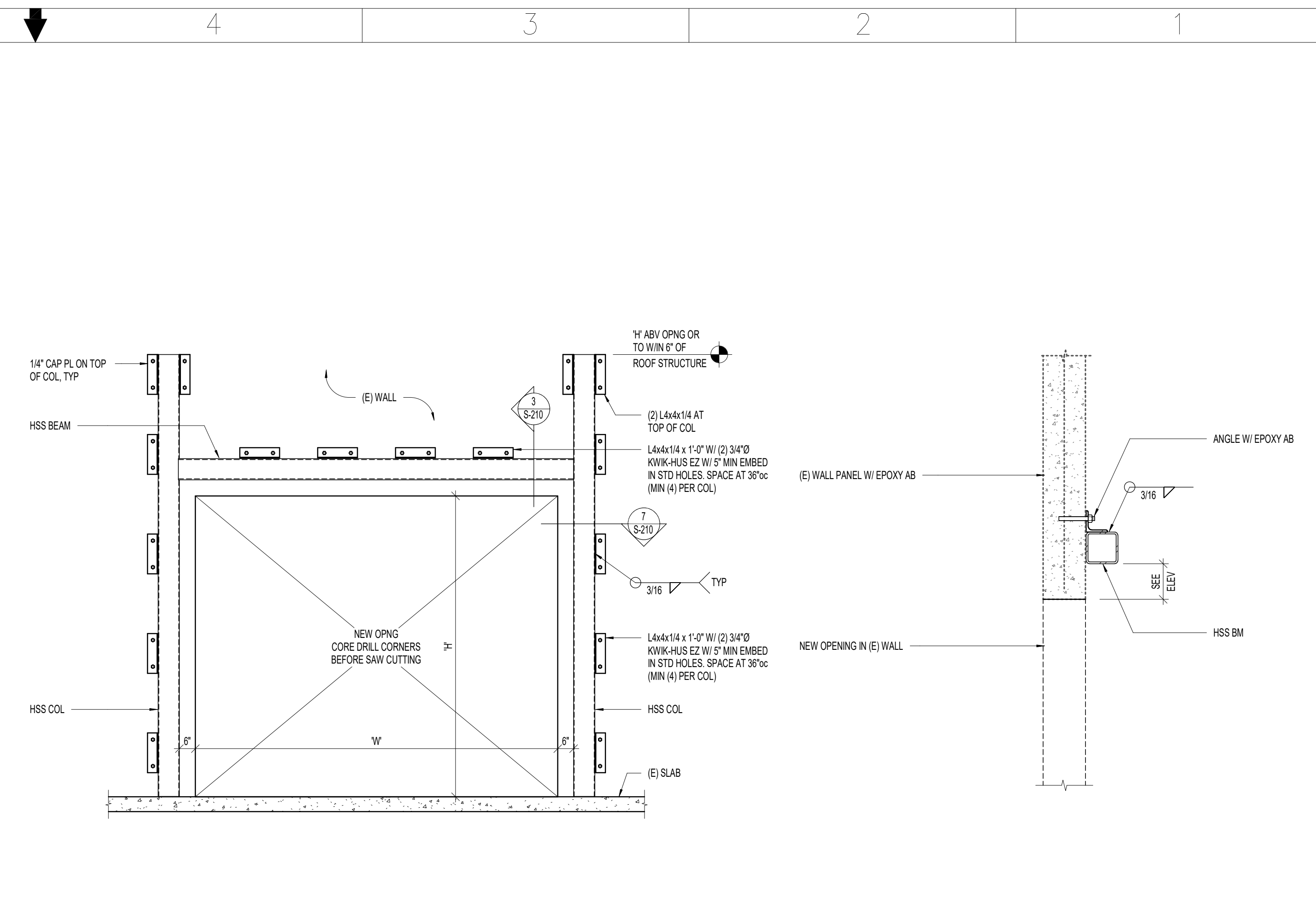
Date: **2024.02.02**
Project No.: **230103**

S-210

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

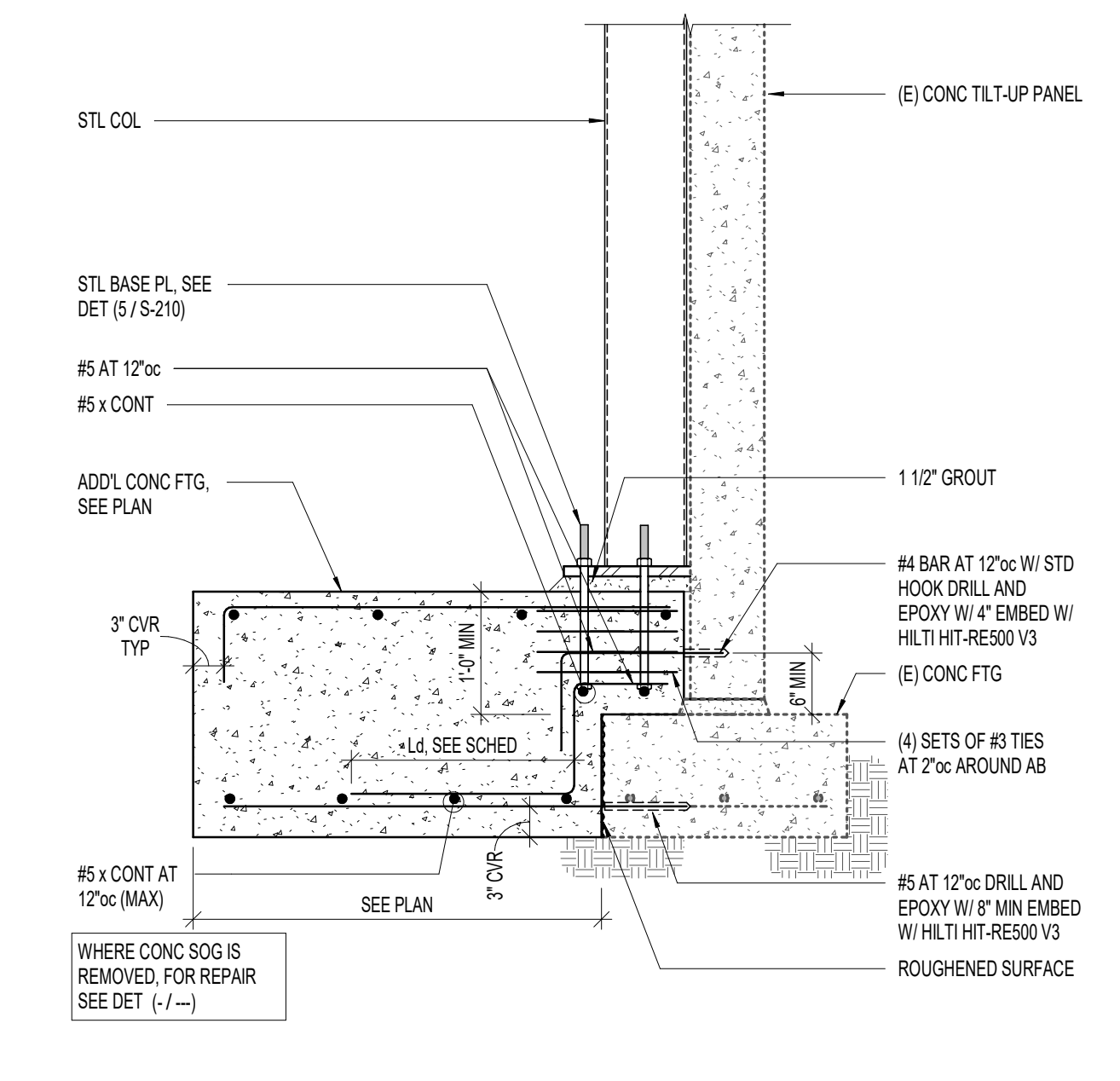


1 NEW OPENING IN EXISTING WALL
S-210 SCALE: 3/16" = 1'-0"

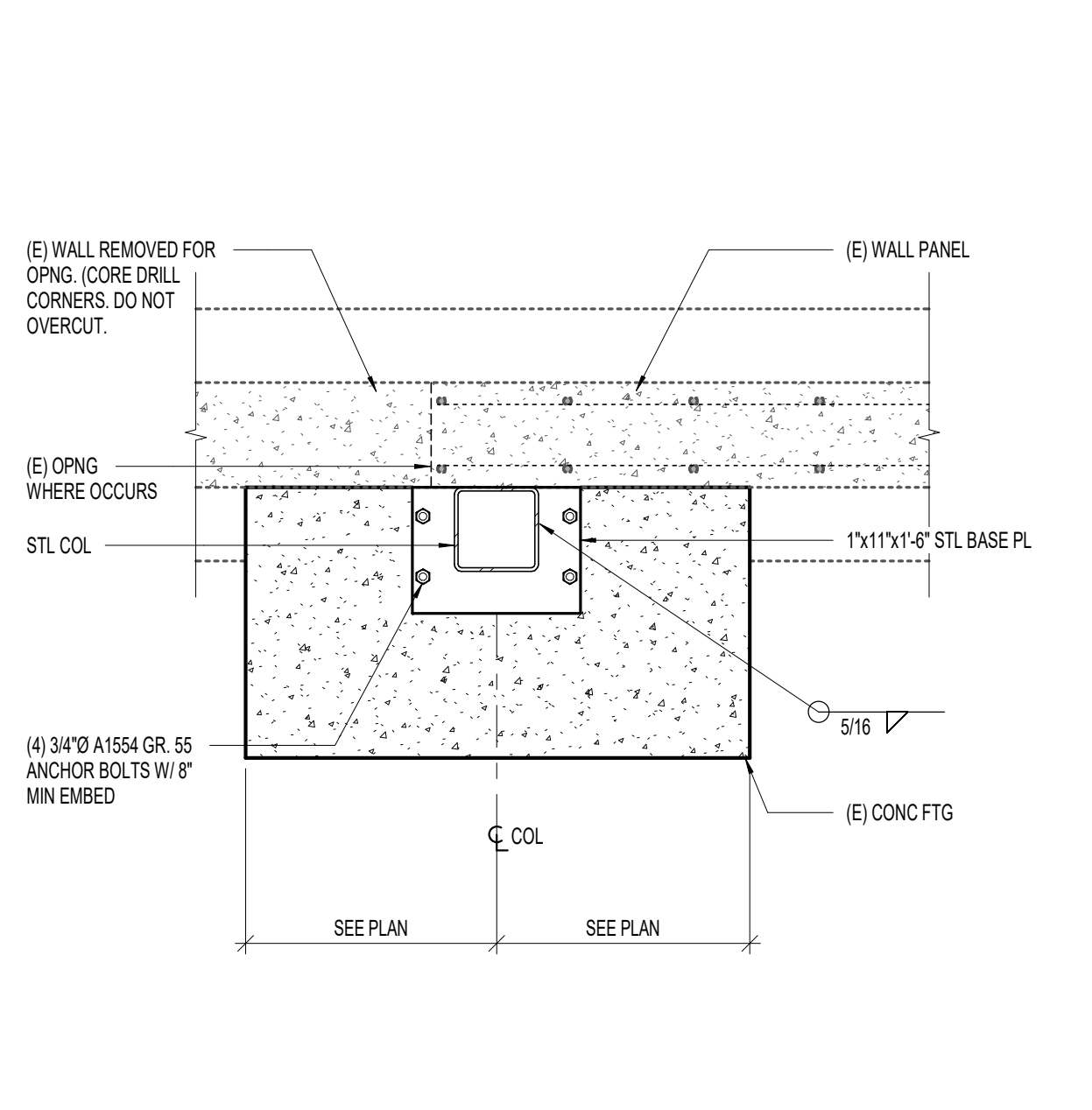


2 TUBE STEEL COLUMN TO SLAB DETAIL
S-210 NO SCALE:

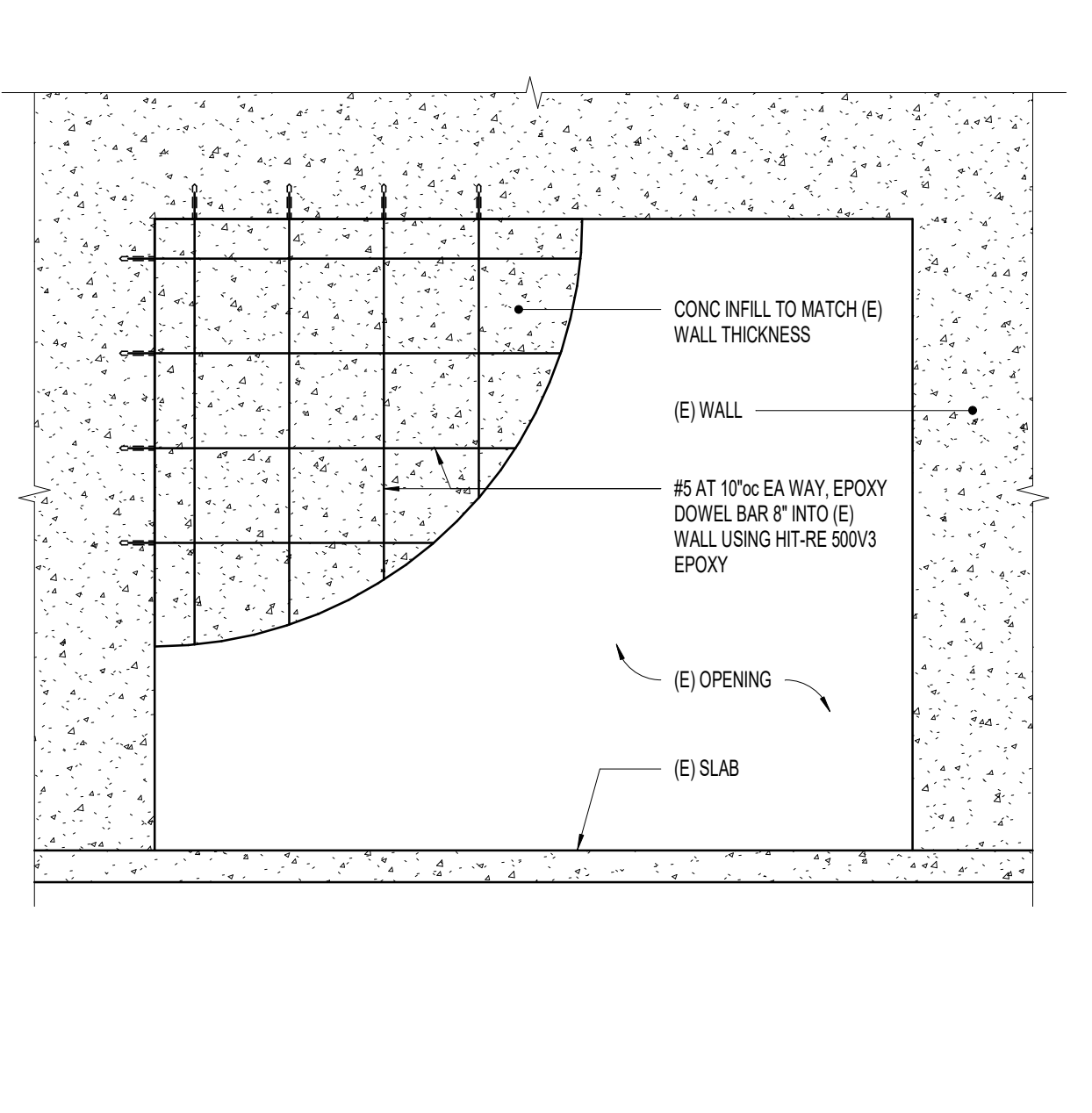
3 TUBE STEEL BEAM TO EXISTING WALL DETAIL
S-210 NO SCALE:



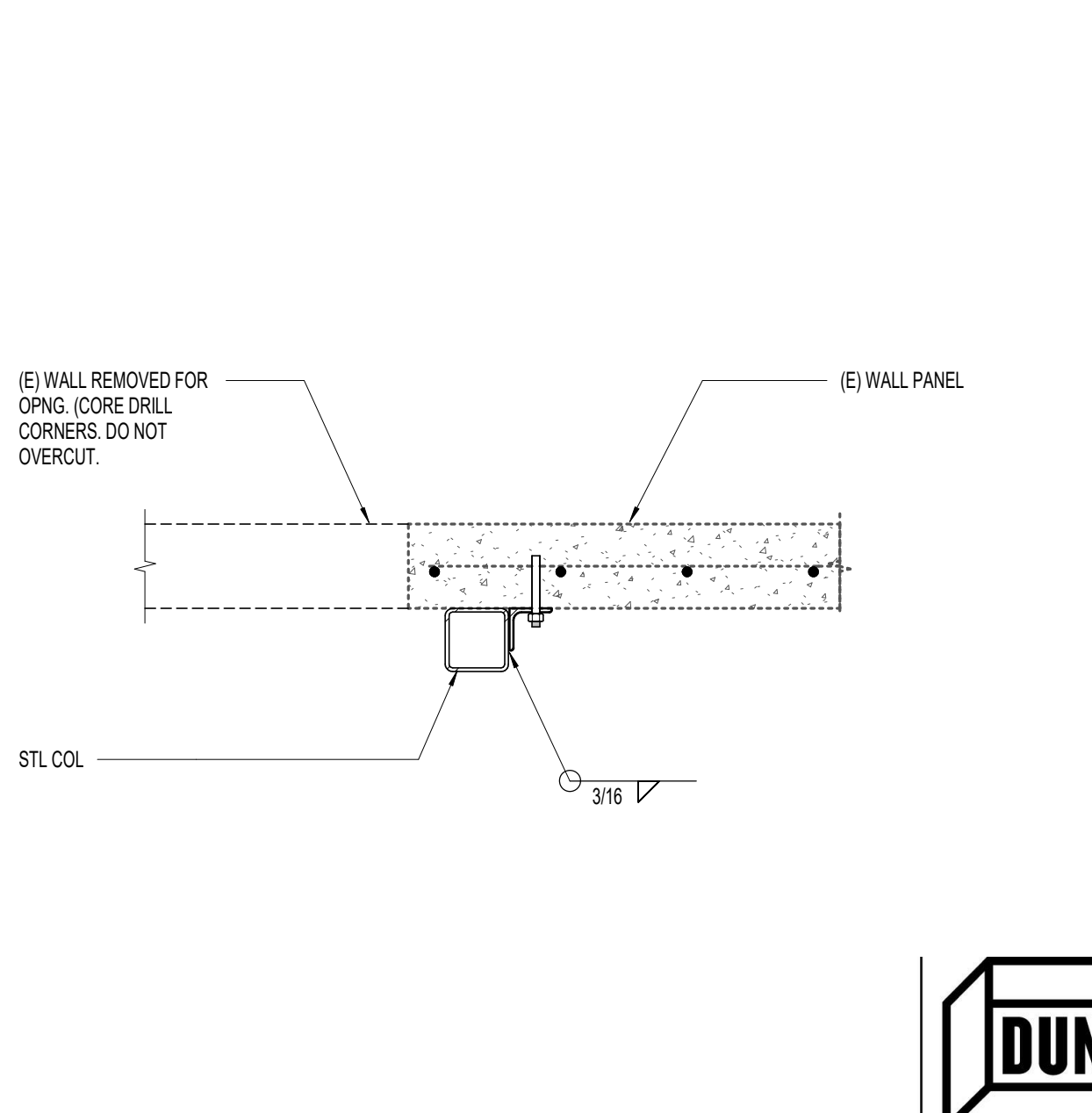
4 NEW TUBE STEEL COLUMN AT EXISTING WALL
S-210 NO SCALE:



5 TUBE STEEL COLUMN TO EXISTING WALL
S-210 NO SCALE:



6 INFILLED OPENINGS ELEVATION
S-210 NO SCALE:



7 TUBE STEEL COLUMN TO EXISTING WALL
S-210 NO SCALE:

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Subconsultants
Dunn & Associates
380 W. 800 S. #100
Salt Lake City, Utah 84101

Van Boerum & Frank Assoc.
181 E. 5600 S.
Murray, Utah 84107

Hunt Electric, Inc.
1863 Alexander Street
Salt Lake City, Utah 84119

Owner / Project Contact

Albany
Engineered
Composites

Tax Parcel ID #:
07-35-252-003-0000

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

Issued/Revisions

No.	Description	Date

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Project Name
ALBANY MEZZANINE

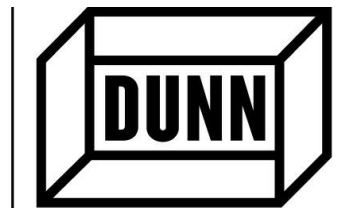
Sheet Title

WIND GIRT ELEVATIONS

Scale	Date
Author	2024.02.02
Project No.	230103

Sheet No.

S-301



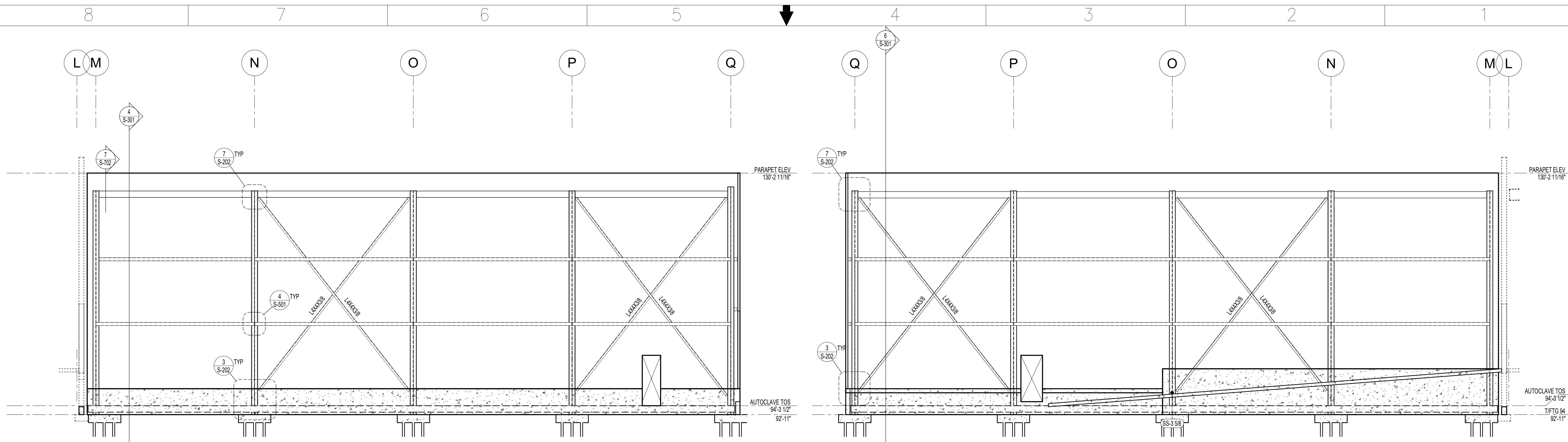
DUNN ASSOCIATES, INC
Consulting Structural Engineers

WWW.DUNN-SE.COM
PH: 801-575-8877 FAX: 801-575-8875

2024.02.02
PROGRESS SET

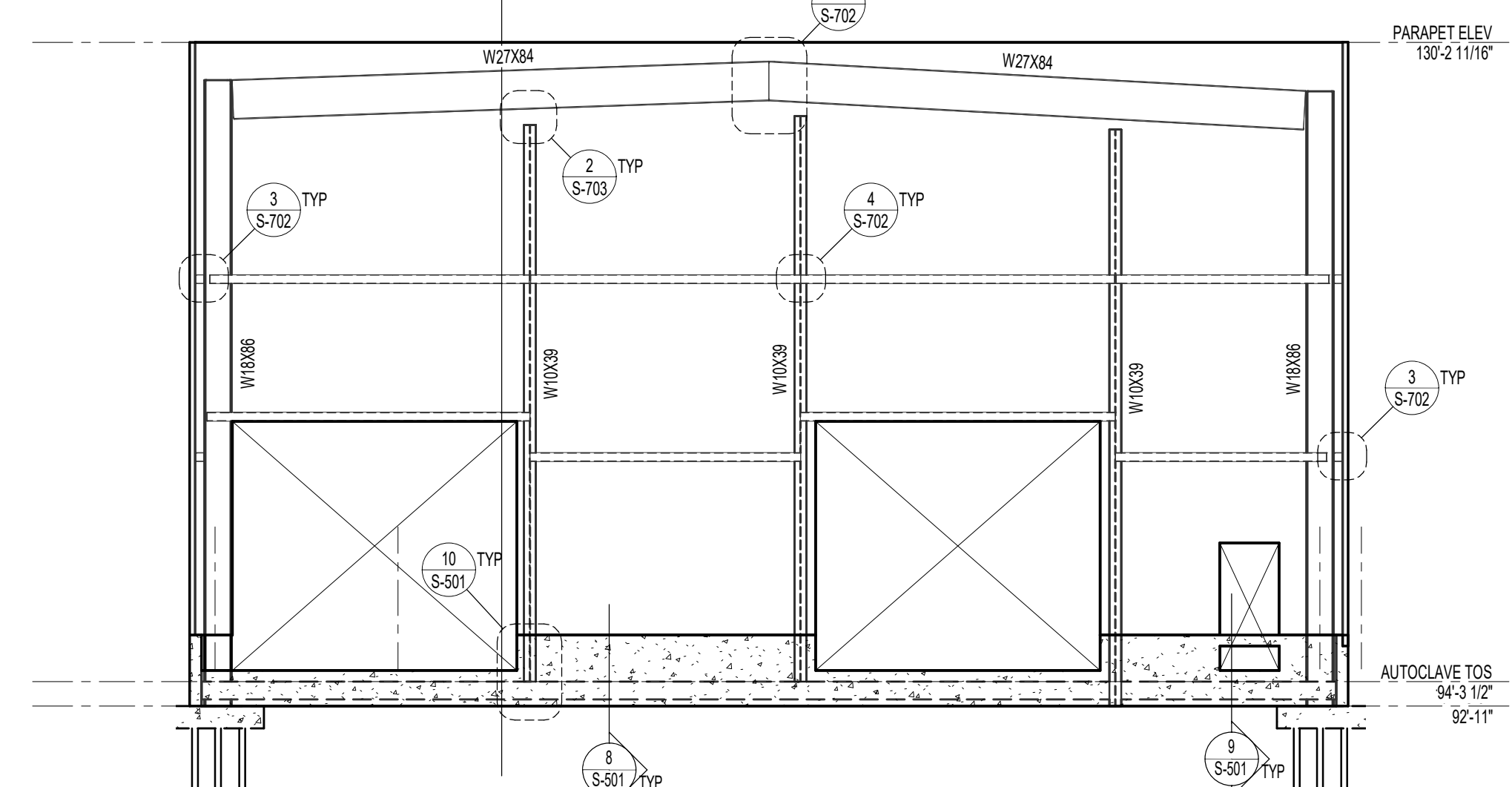
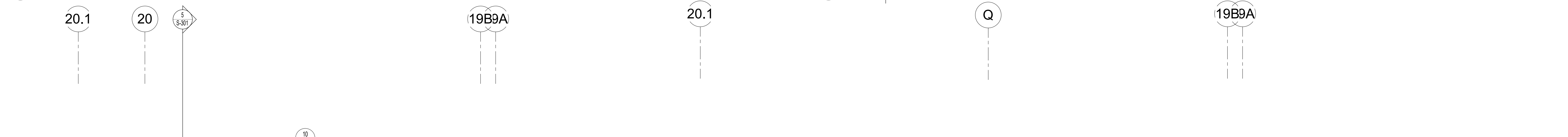
NOTE:
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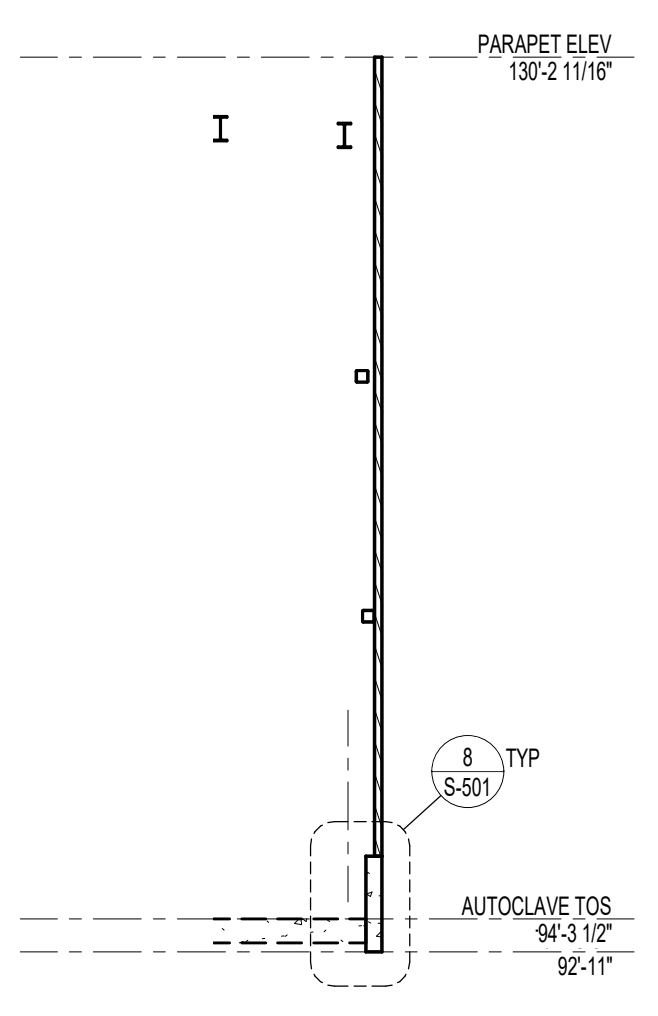


1 WIND GIRT EAST ELEVATION
S-301 SCALE: 1/8" = 1'-0"

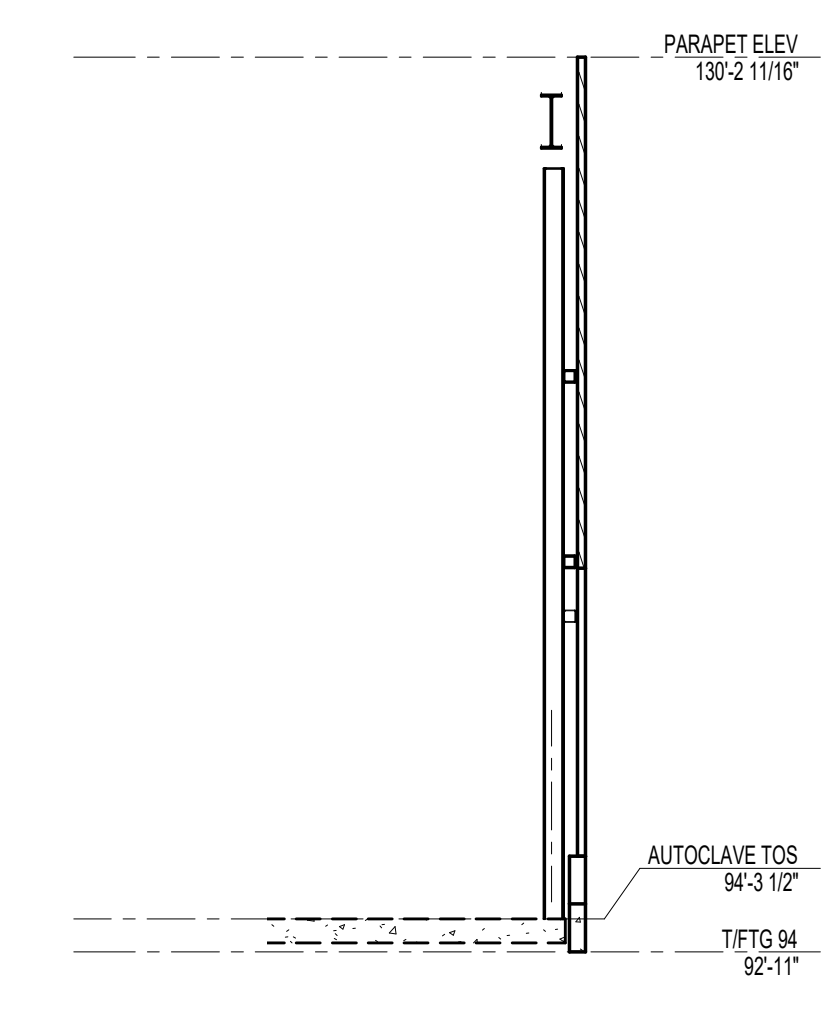
2 WIND GIRT WEST ELEVATION
S-301 SCALE: 1/8" = 1'-0"



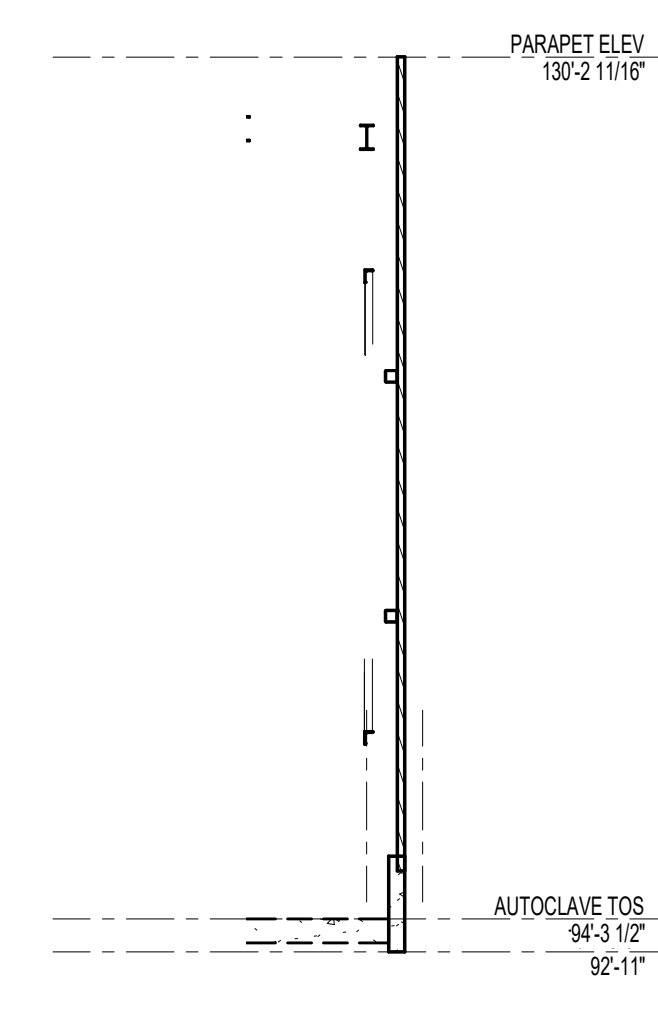
3 BENT-1 AND WIND GIRT NORTH ELEVATION
S-301 SCALE: 1/8" = 1'-0"



4 TYPICAL SIDE WALL GIRT WALL SECTION
S-301 SCALE: 1/8" = 1'-0"



5 TYPICAL FRONT WALL GIRT WALL SECTION
S-301 SCALE: 1/8" = 1'-0"



6 SECTION 13
S-301 SCALE: 1/8" = 1'-0"

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Issued/Revisions

No.	Description	Date

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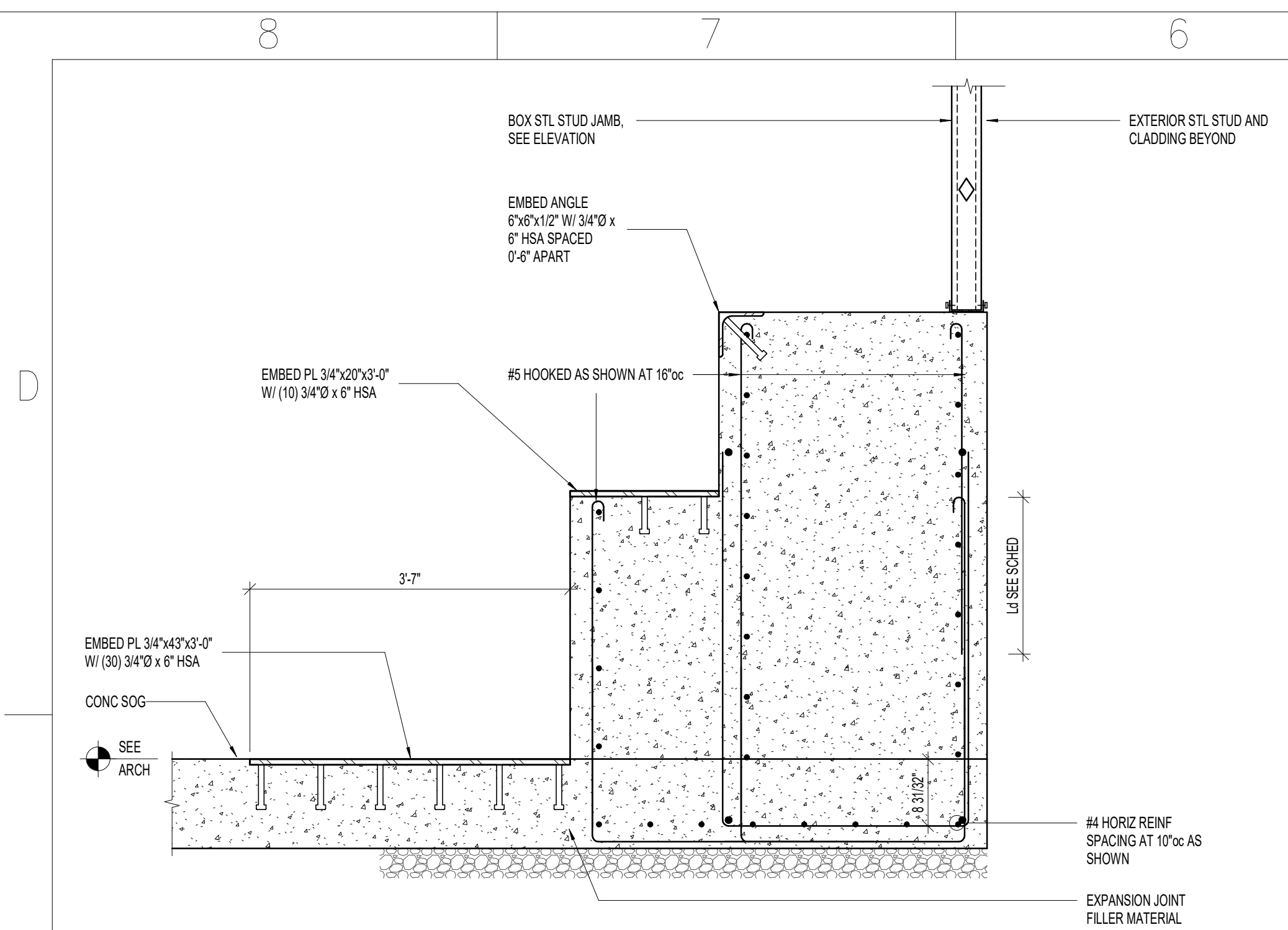
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Project Name
ALBANY MEZZANINE

FOOTING AND FOUNDATION DETAILS

Scale	Date
Drawn	2024.02.02
JDD	Project No.
	230103

S-502



1 TYPICAL CONCRETE WALL AT INTERIOR

S-502 NO SCALE:
1/11/21

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Consulting Structural Engineers
WWW.DUNN-SE.COM
PH: 801-575-8877 FAX: 801-575-8875

2024.02.02
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Project Name
ALBANY MEZZANINE

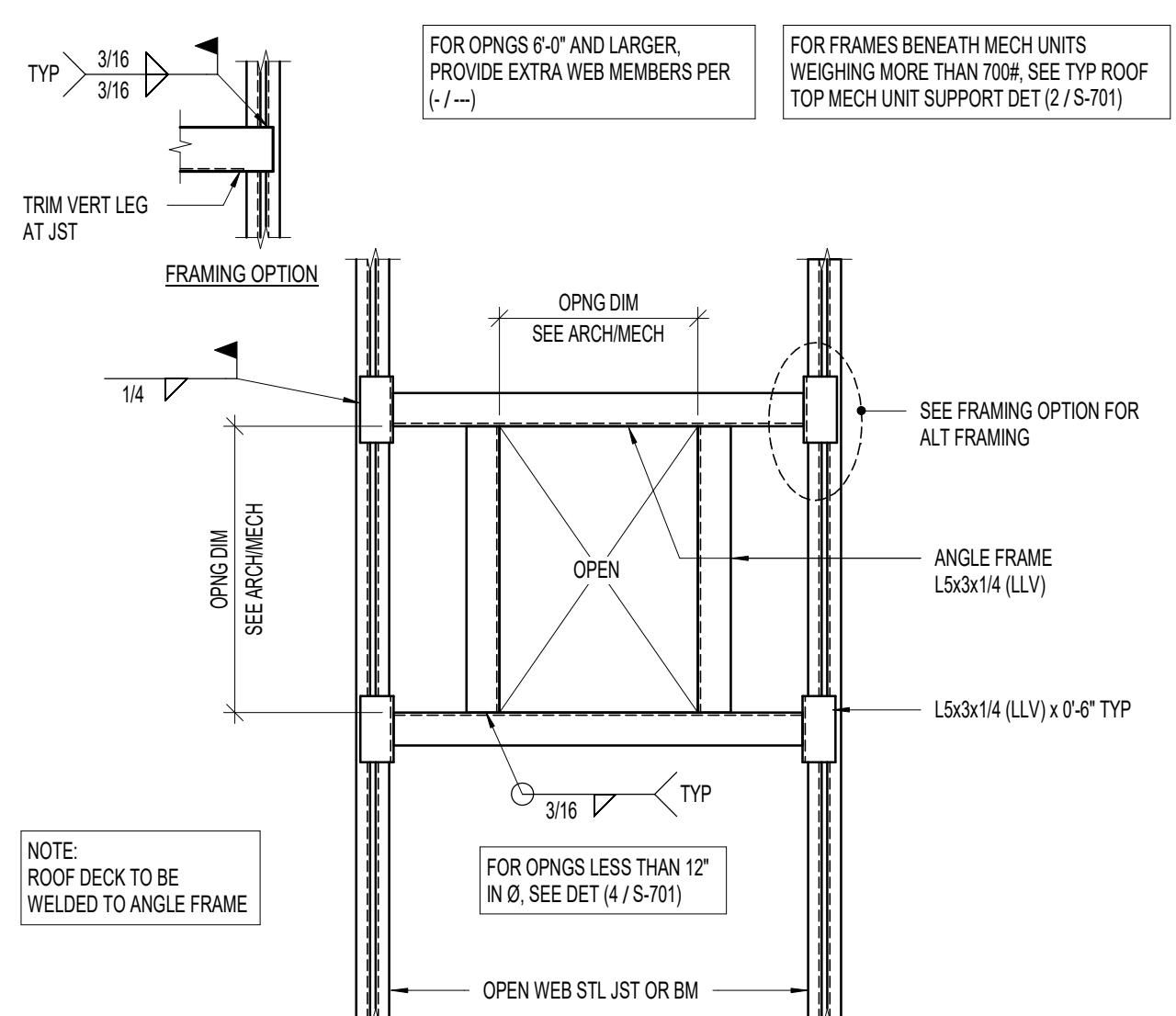
Sheet Title
ROOF FRAMING DETAILS

<small>Scale</small>	<small>Date</small>
<small>Drawn</small>	<small>Project No.</small>
<small>Checked</small>	<small>Issue</small>

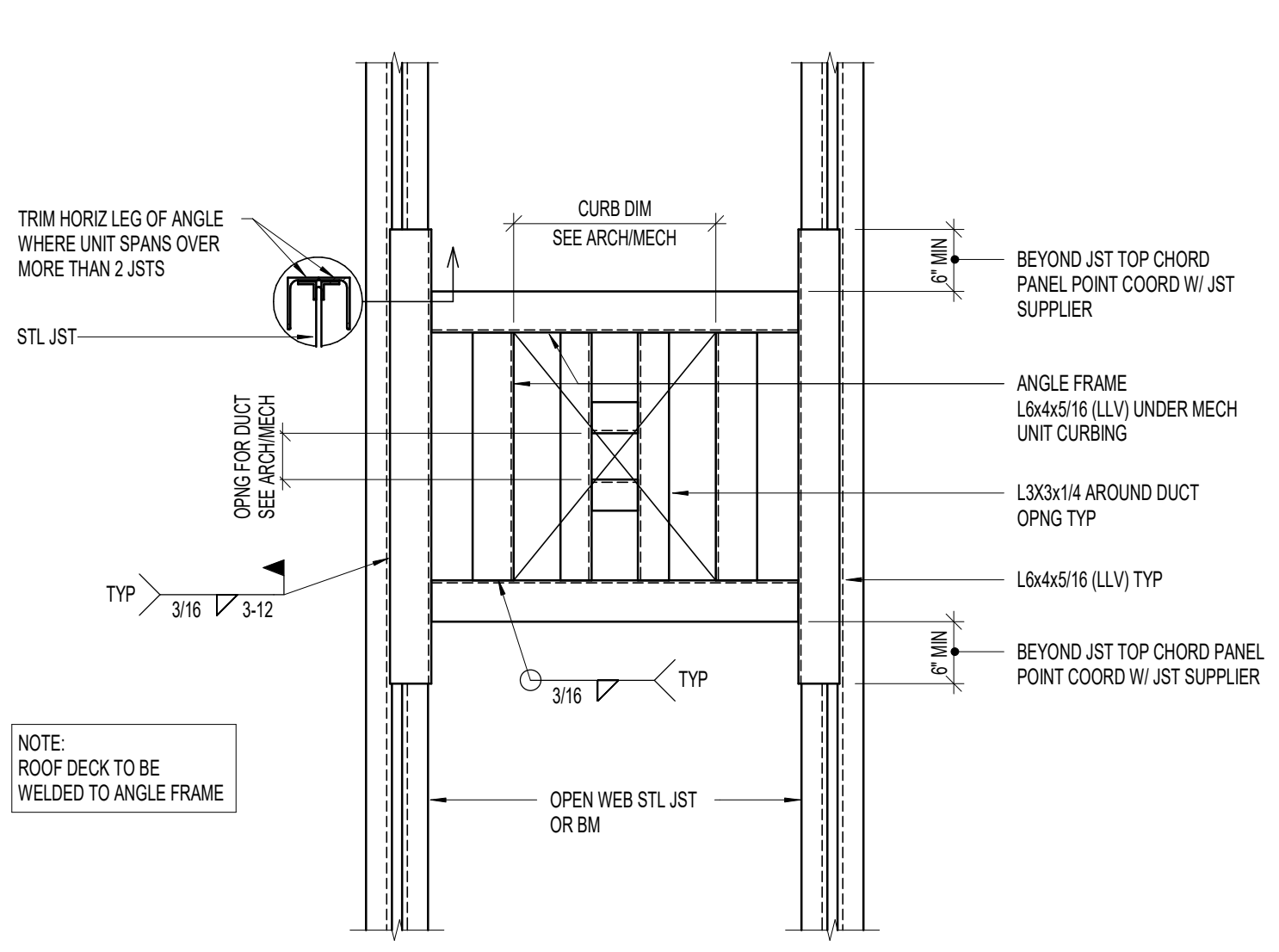
S-701

ALBANY ENGINEERED COMPOSITES - 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116

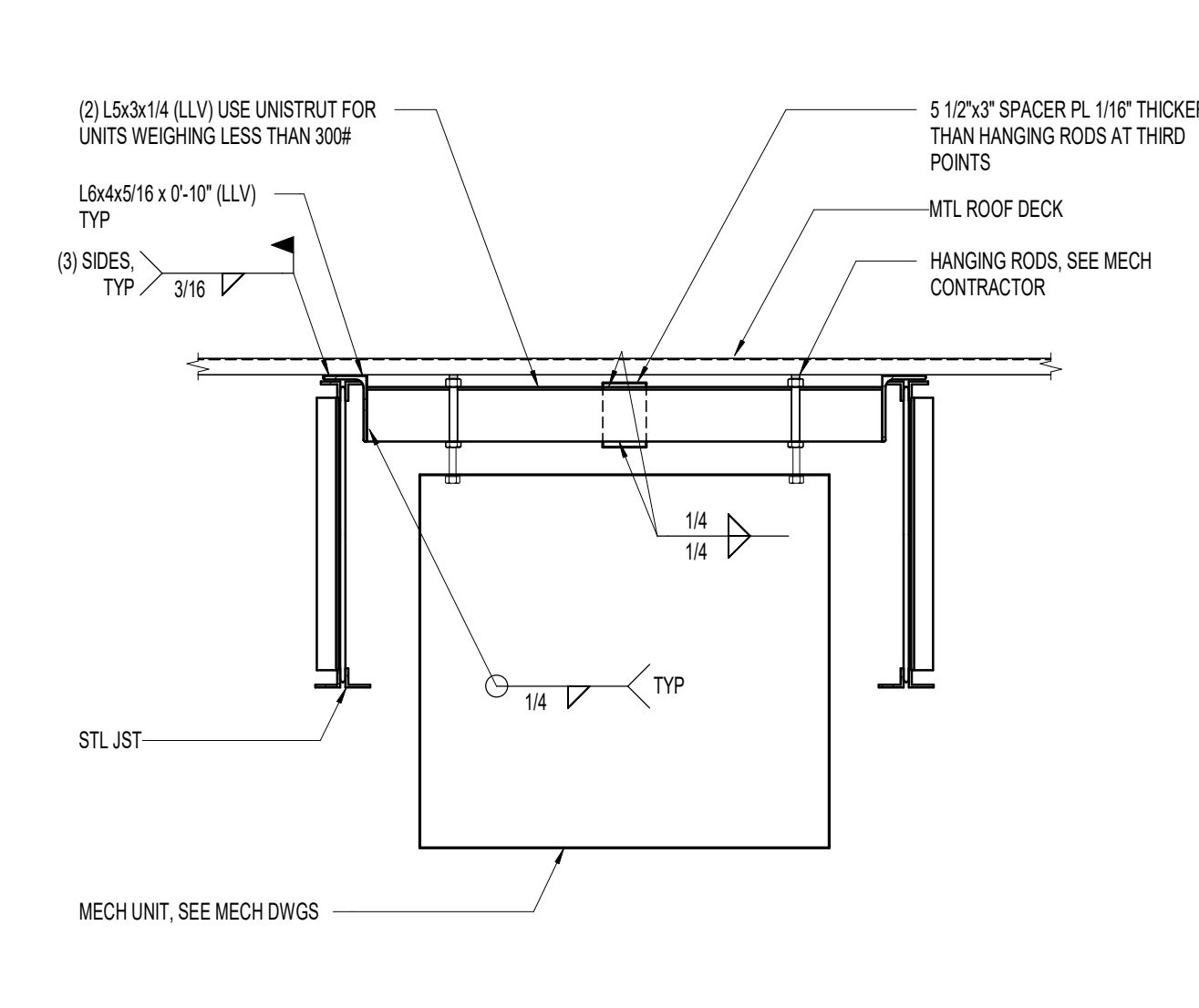
SOLID BLK THE FLUTES OF THE STL DECK BENEATH THE CURB OF THE MECH UNIT W/ HSS1 1/2x1 1/2x3/16x 0'-6", TACK WELDED TO THE STL DECK OR W/ SOLID BLK CONNECTED TO THE STL DECK



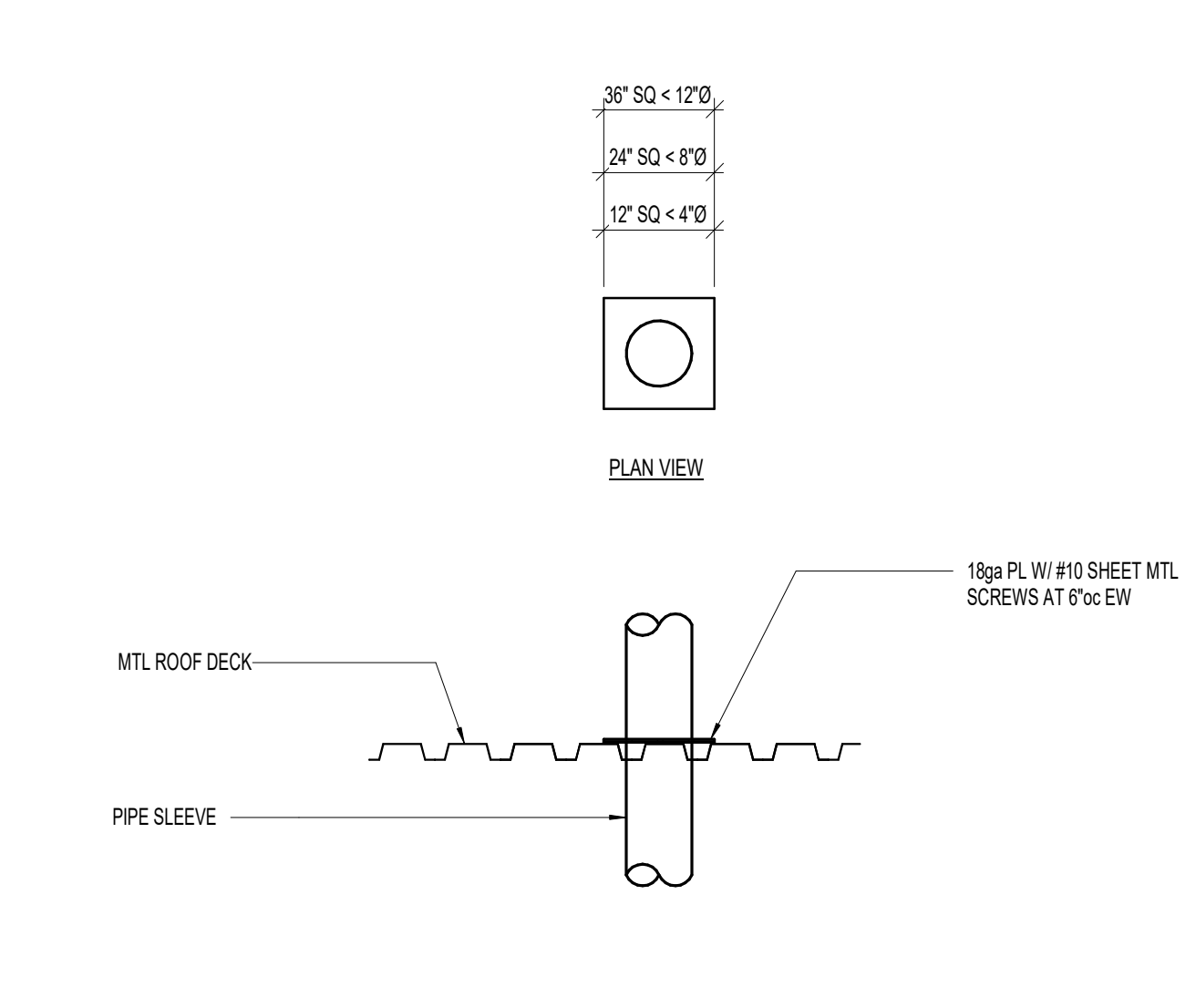
1 TYPICAL ROOF OPENING OR MECHANICAL UNIT WEIGHING LESS THAN 700#
S-701 NO SCALE: 1:20x1



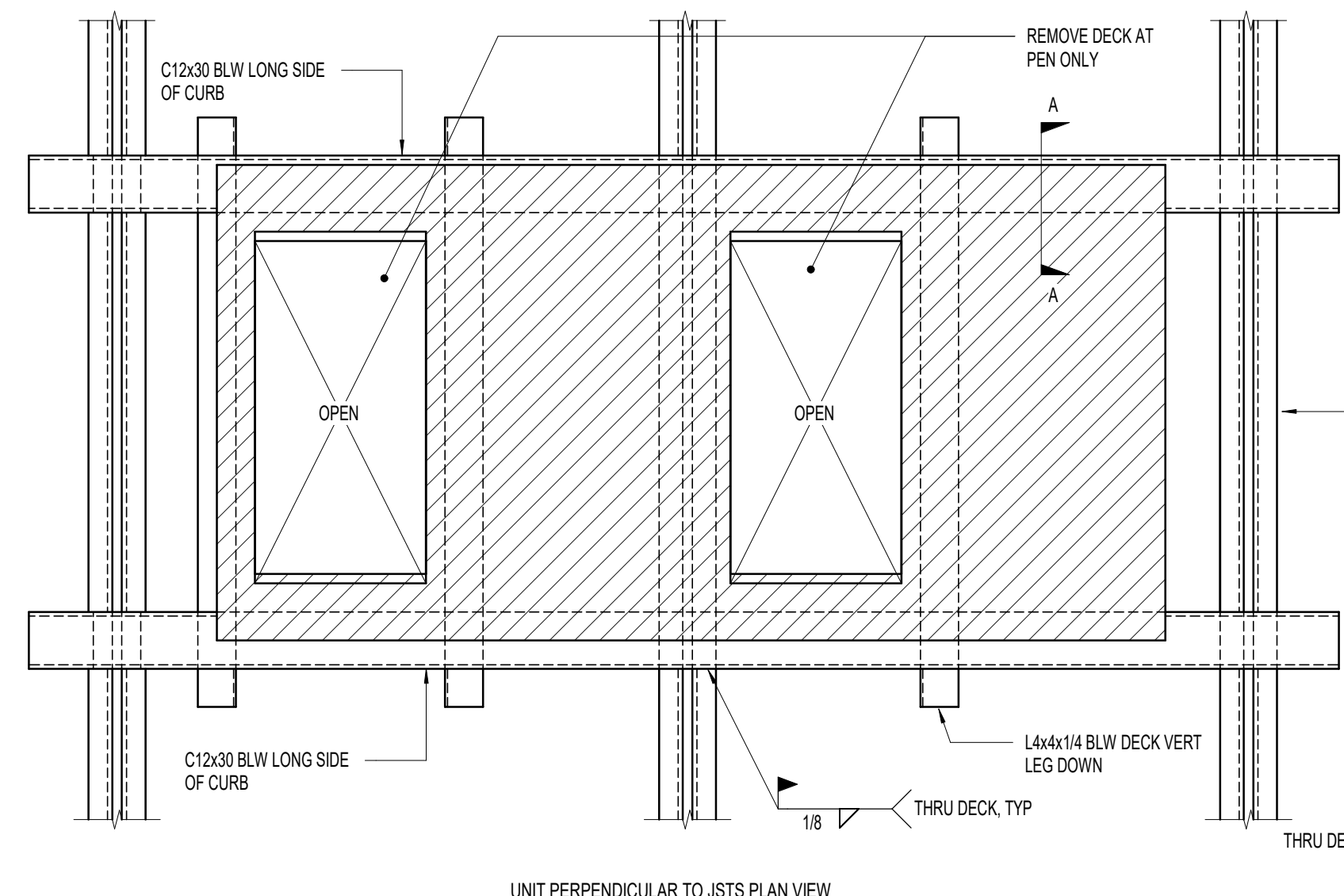
2 TYPICAL ROOF TOP MECHANICAL UNIT WEIGHING MORE THAN 700#
S-701 NO SCALE: 1:20x1



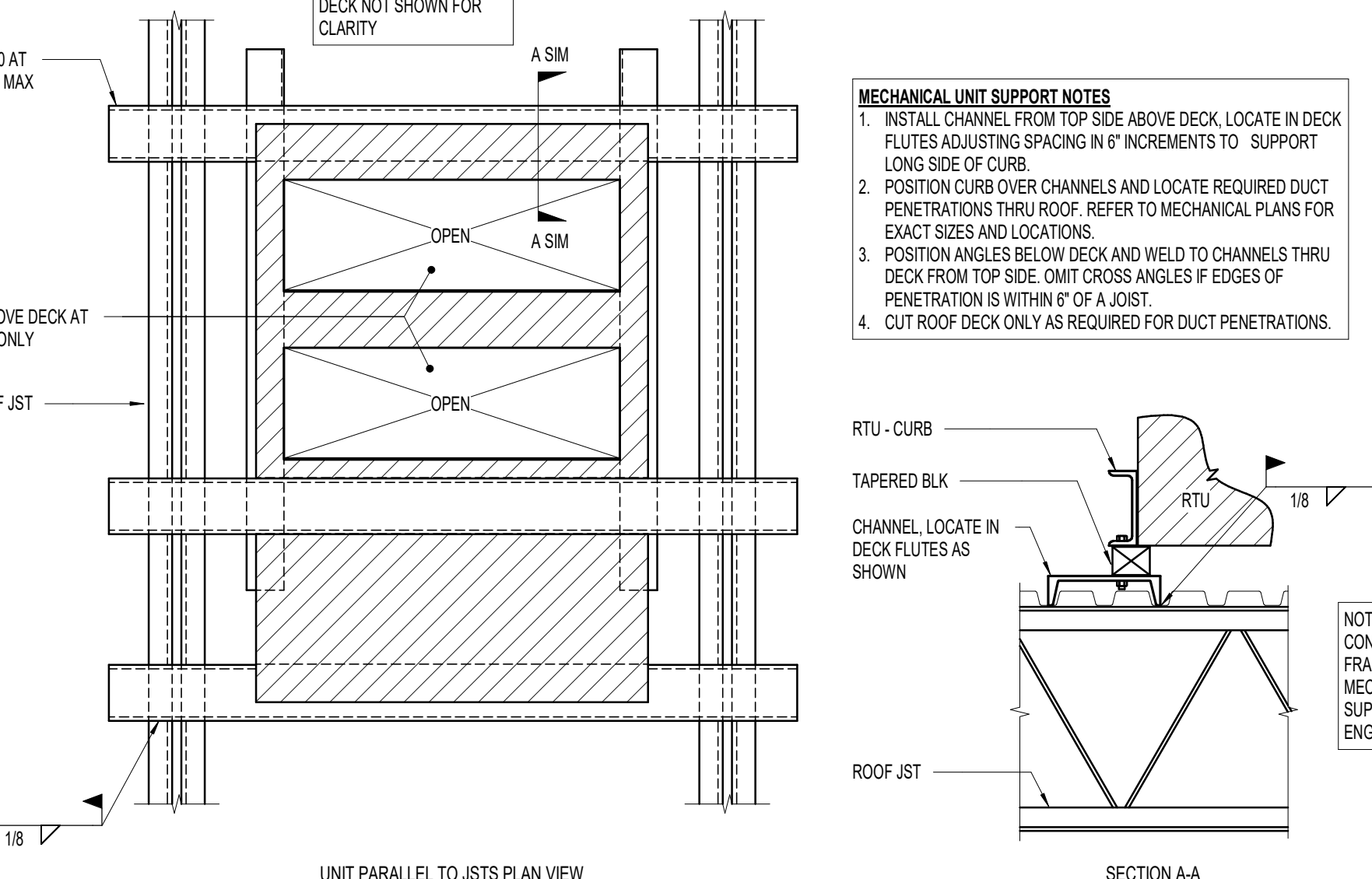
3 HANGING MECHANICAL UNIT
S-701 NO SCALE: 1:20x1



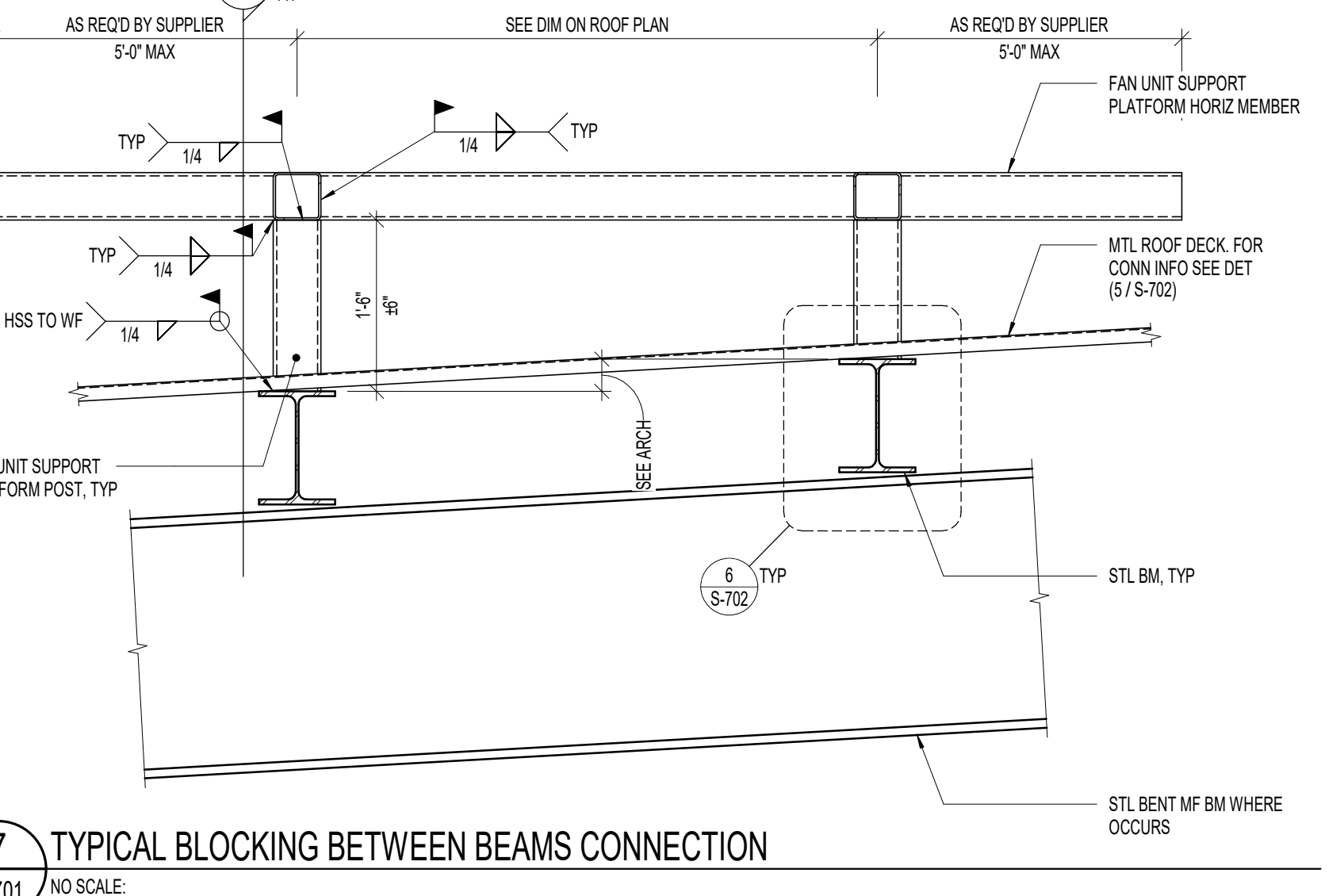
4 TYPICAL PIPE SLEEVE HOLE DETAIL (12"Ø OR LESS) THRU ROOF DECK
S-701 NO SCALE: 1:20x1



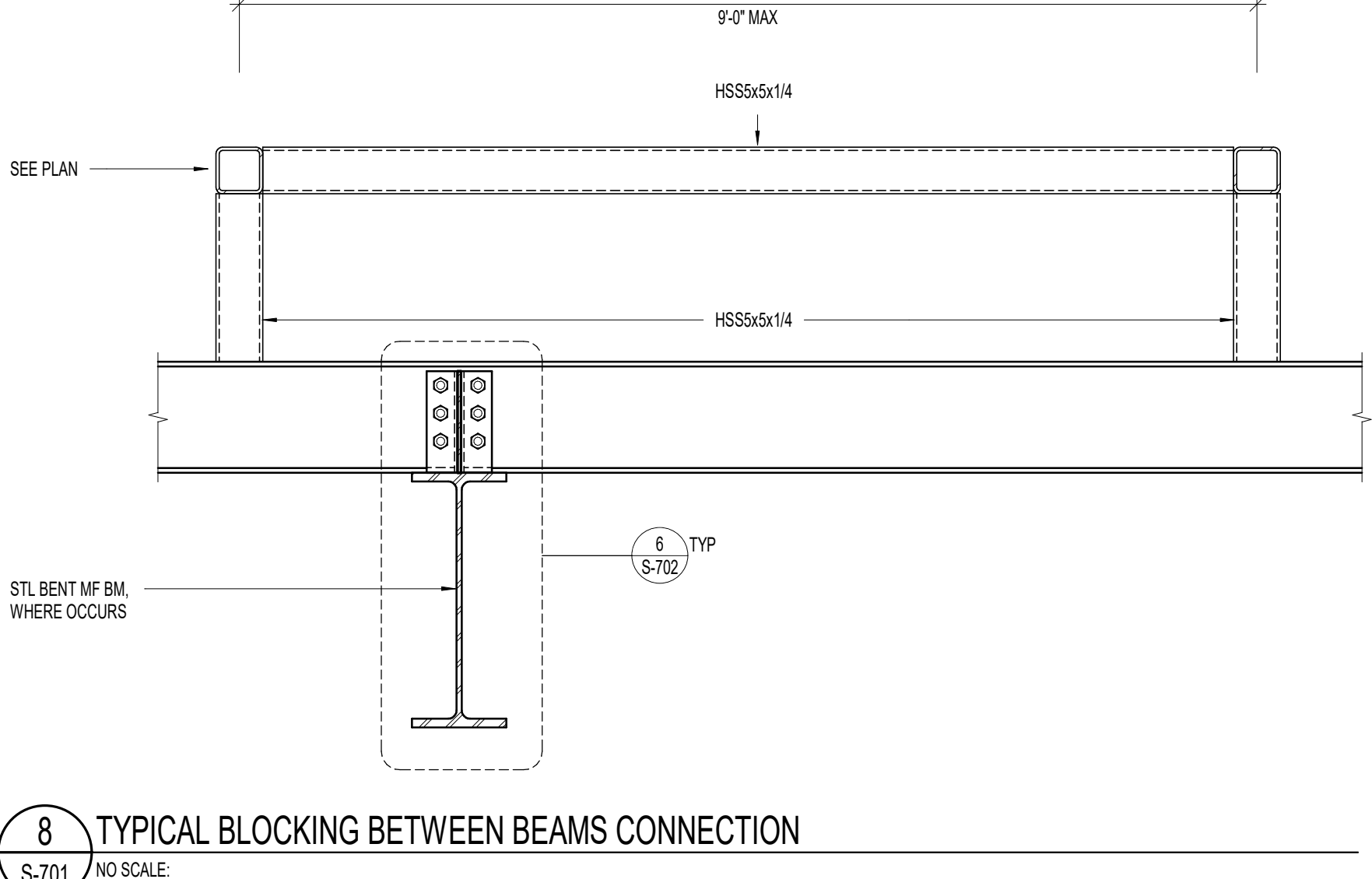
5 TYPICAL ROOF OPENING OR MECHANICAL UNITS (PLAN VIEW)
S-701 NO SCALE: 1:20x1



6 TYPICAL ROOF DRAIN SUPPORT (PLAN VIEW)
S-701 NO SCALE: 1:20x1



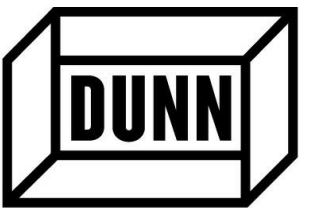
7 TYPICAL BLOCKING BETWEEN BEAMS CONNECTION
S-701 NO SCALE: 1:40x1



8 TYPICAL BLOCKING BETWEEN BEAMS CONNECTION
S-701 NO SCALE: 1:40x1

MECHANICAL UNIT SUPPORT NOTES
1. INSTALL CHANNEL FROM TOP SIDE ABOVE DECK. LOCATE IN DECK FLUTES ADJUSTING SPACING IN 6" INCREMENTS TO SUPPORT LONG SIDE OF CURB.
2. POSITION CURB OVER CHANNELS AND LOCATE REQUIRED DUCT PENETRATIONS THRU ROOF. REFER TO MECHANICAL PLANS FOR EXACT SIZES AND LOCATIONS.
3. POSITION ANGLES BELOW DECK AND WELD TO CHANNELS THRU DECK FROM TOP SIDE. OMIT CROSS ANGLES IF EDGES OF PENETRATION IS WITHIN 6" OF A JOST.
4. CUT ROOF DECK ONLY AS REQUIRED FOR DUCT PENETRATIONS.

NOTE: ACTUAL CONFIGURATION OF FRAME TO SUPPORT MECH UNIT TO BE SUPPLIED BY THE MECH ENGINEER



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Consulting Structural Engineers

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PH: 801-575-8877 FAX: 801-575-8875

2024.02.02
PROGRESS SET

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ALBANY MEZZANINE

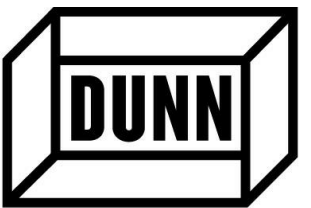
Sheet Title
ROOF FRAMING DETAILS

Scale
**2024.02.02
PROGRESS SET**

Date
2024.02.02

Author
230103

S-702



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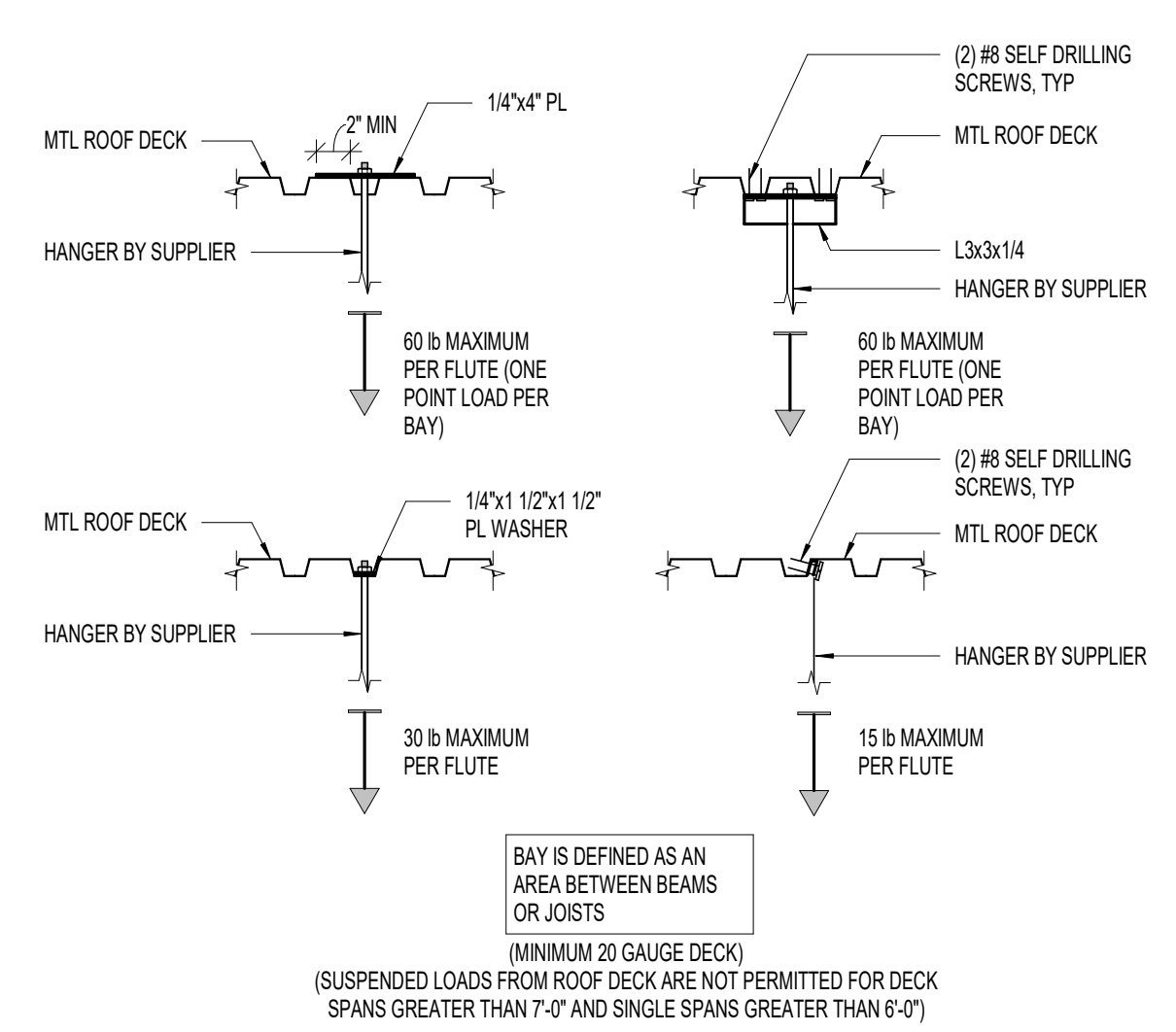
WWW.DUNN-SE.COM
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**2024.02.02
PROGRESS SET**

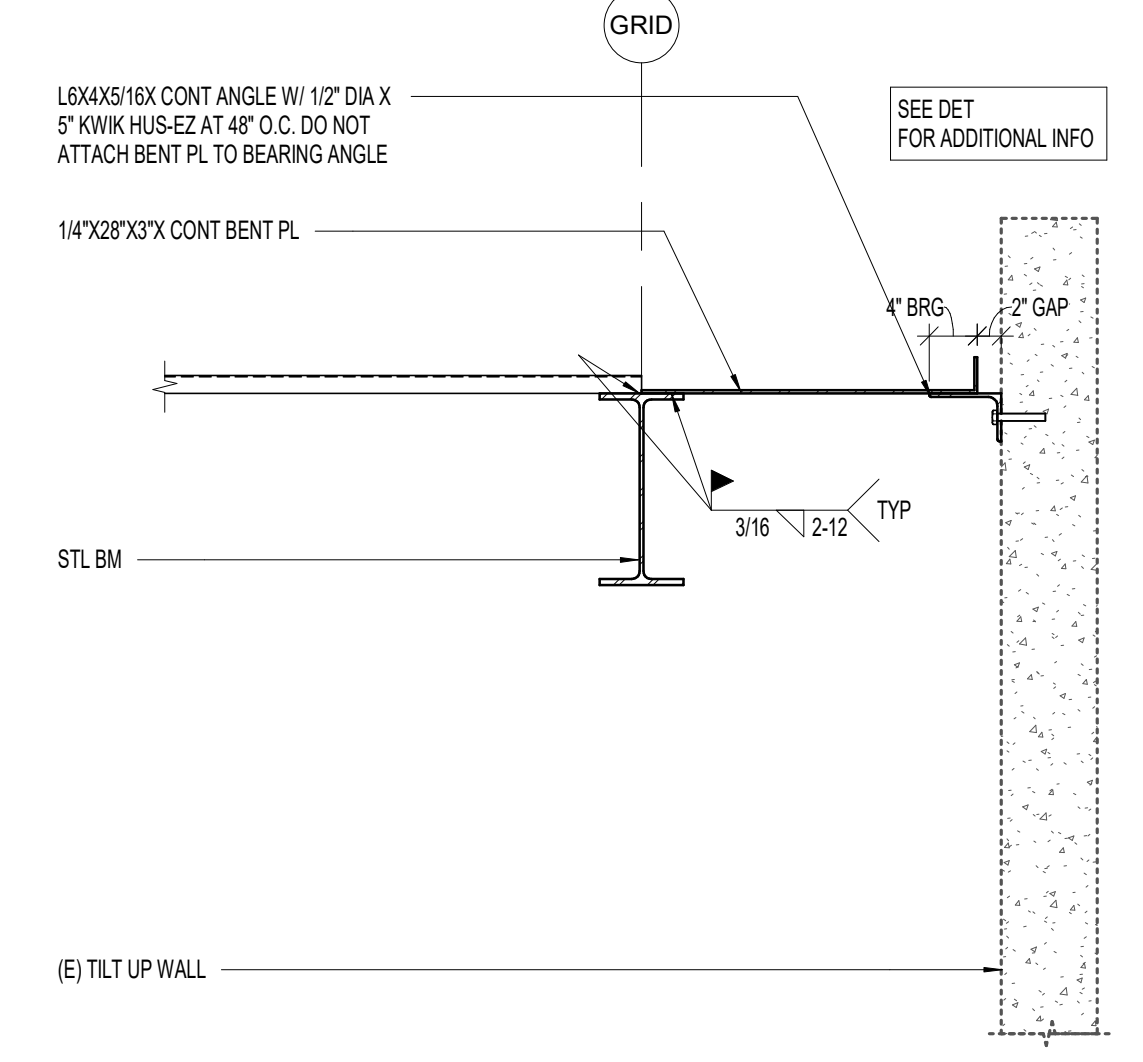
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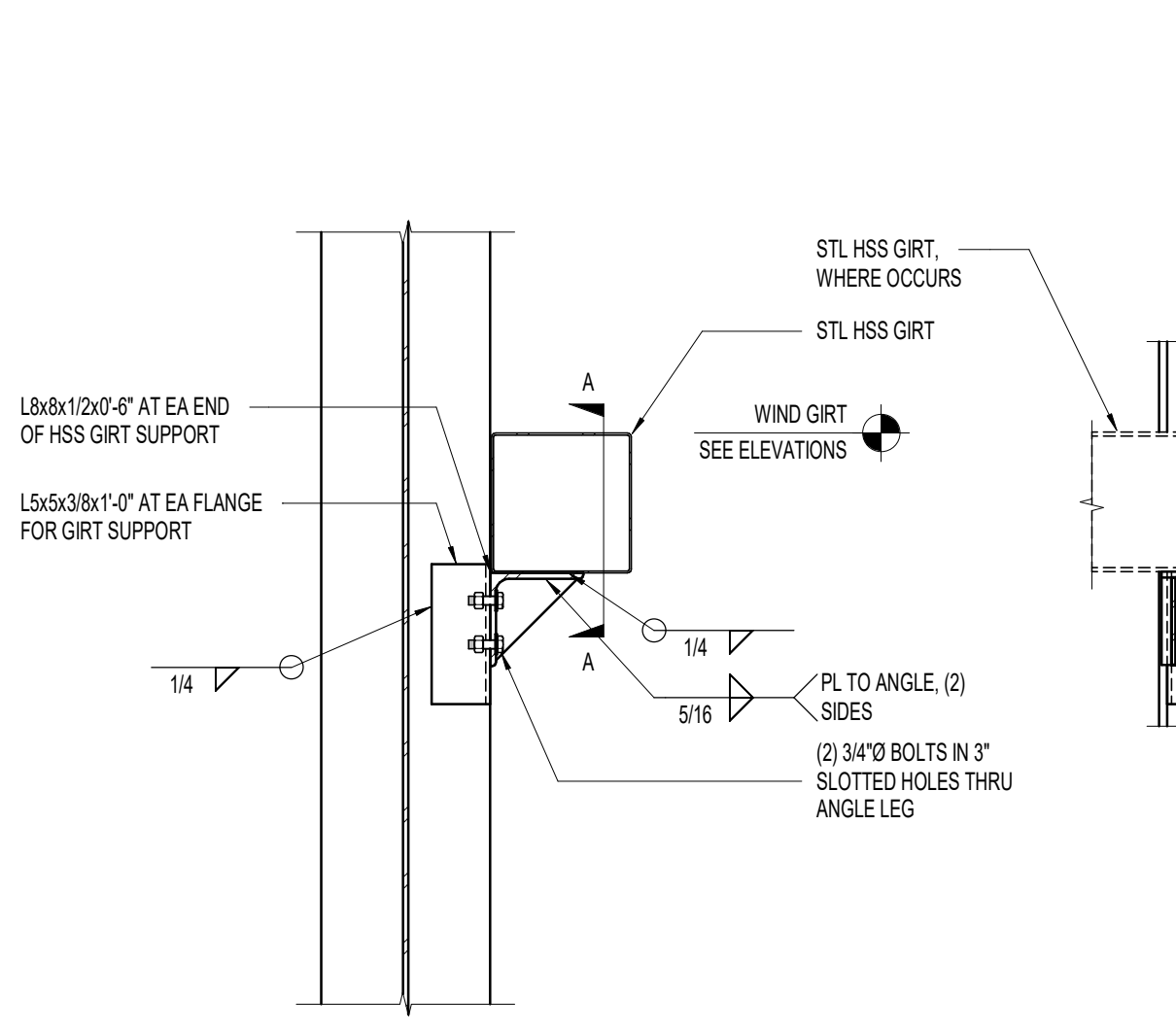
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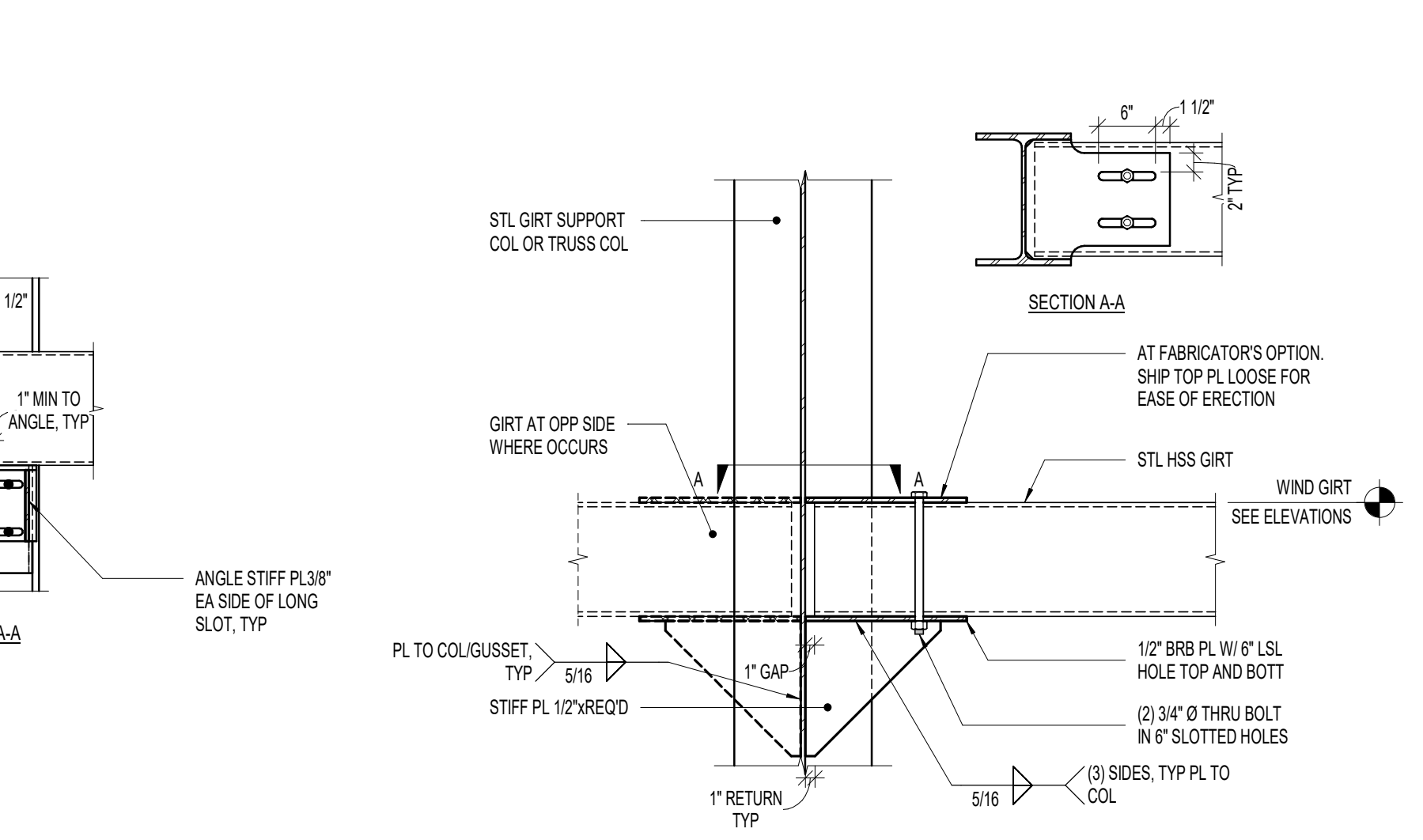
1 SUSPENDED LOADS FROM METAL DECK
S-702 NO SCALE.
1/28/10



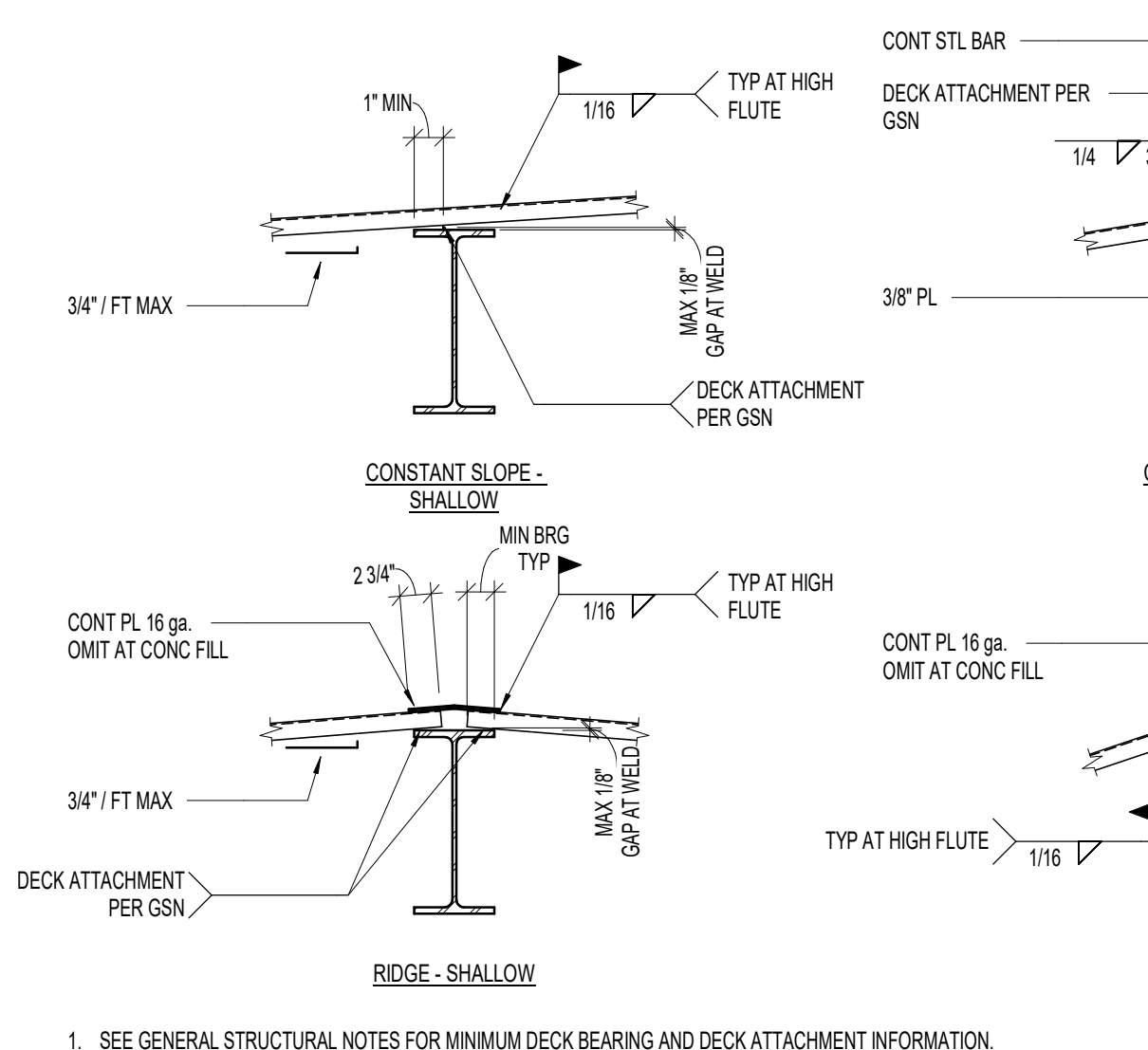
2 JOINT
S-702 NO SCALE.
1/28/10



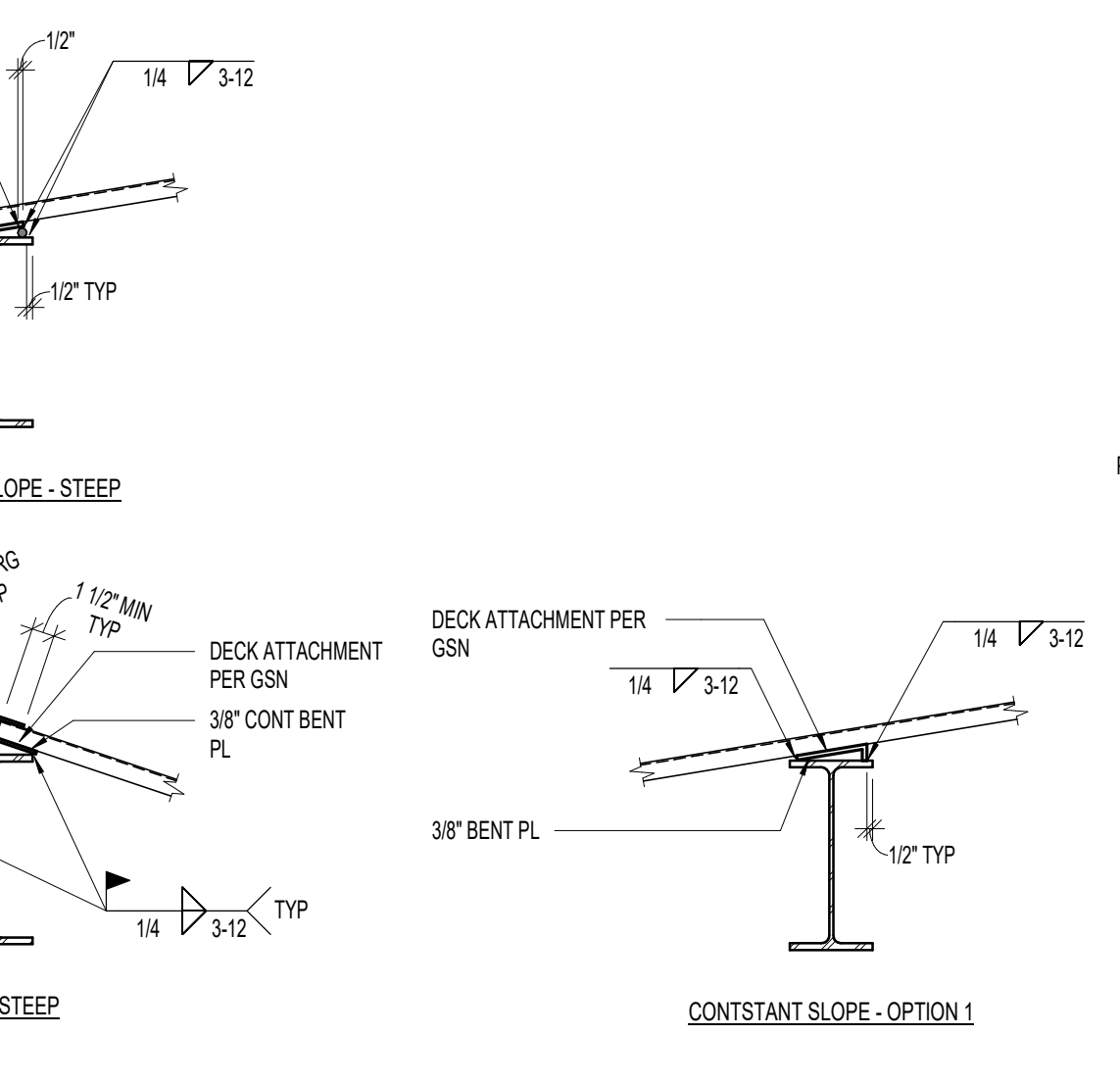
3 TYPICAL HSS GIRT CONNECTION TO WF COLUMN
S-702 NO SCALE.
1/28/10



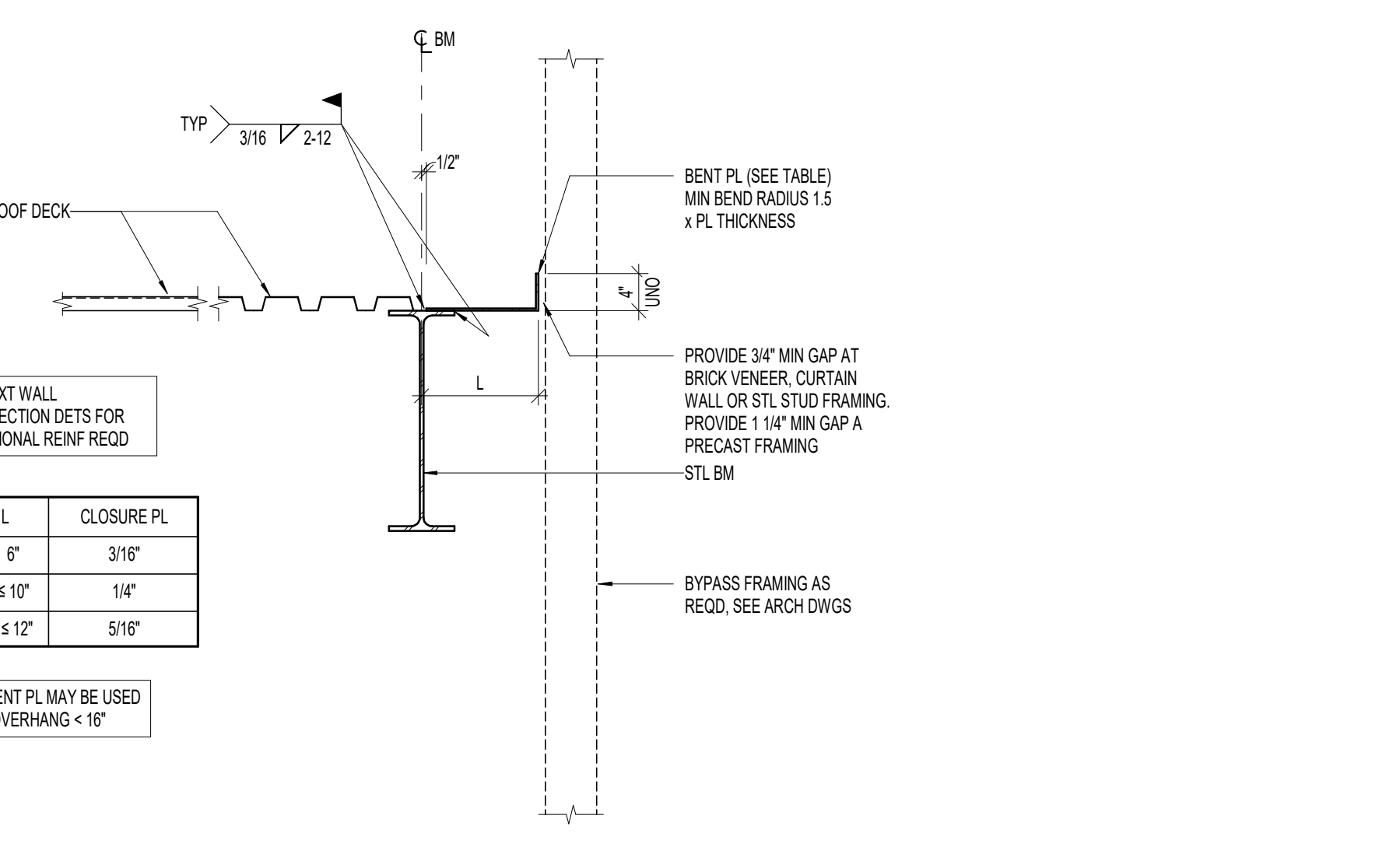
4 TYPICAL HSS GIRT CONNECTION TO WF COLUMN
S-702 NO SCALE.
1/28/10



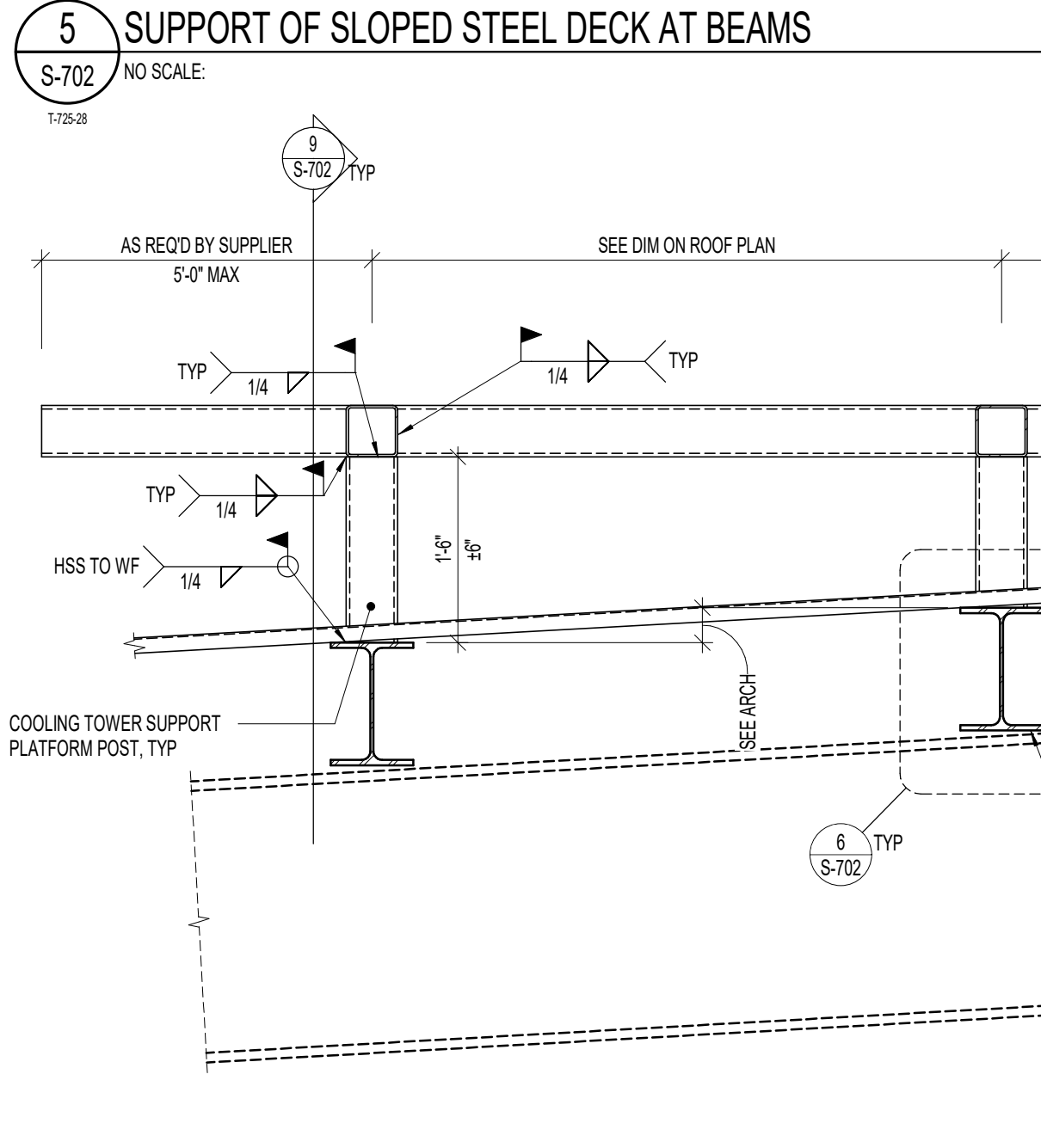
5 SUPPORT OF SLOPED STEEL DECK AT BEAMS
S-702 NO SCALE.
1/28/10



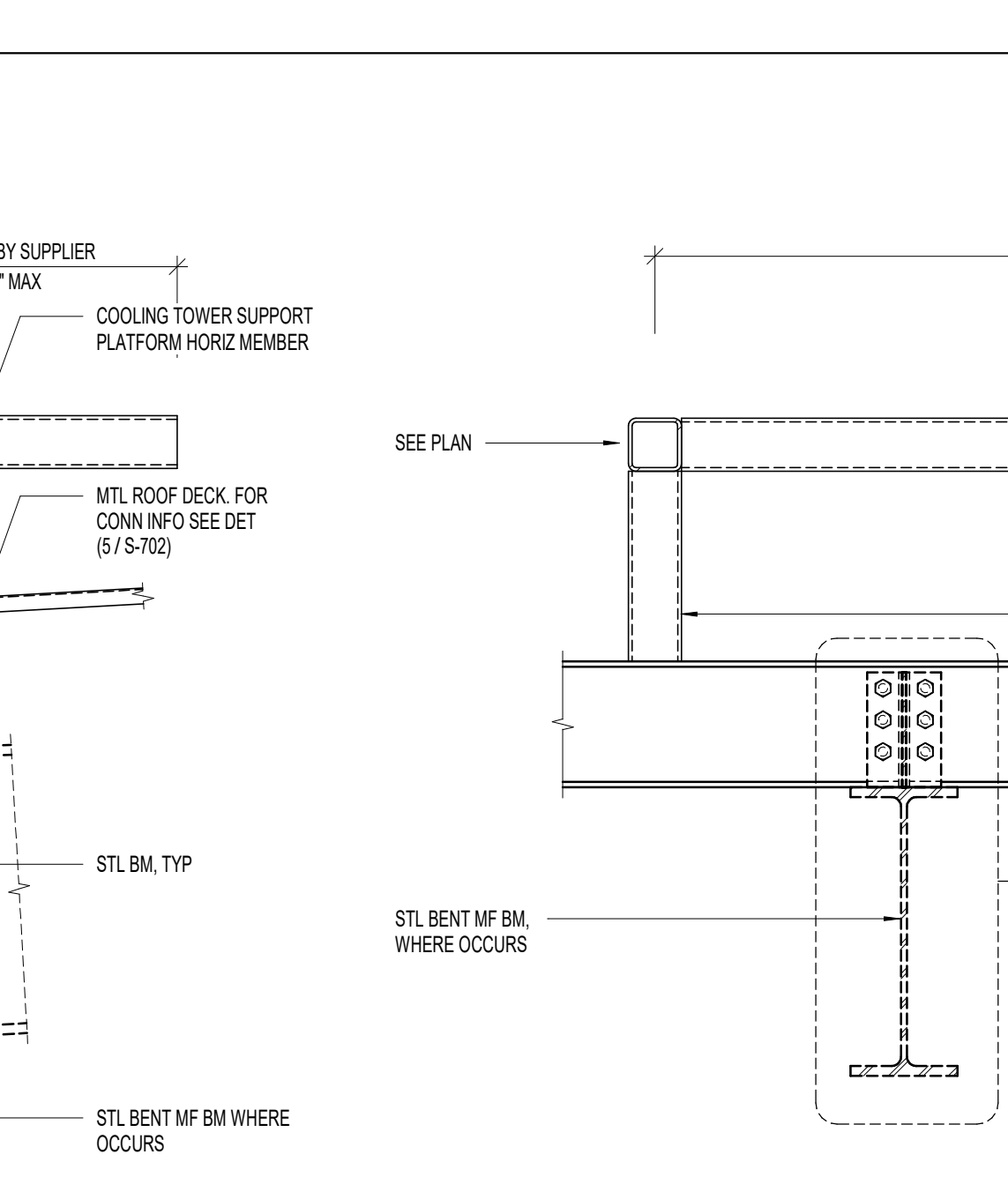
6 TYPICAL ROOF BEAM TO MOMENT FRAME BENT DETAIL
S-702 NO SCALE.
1/28/10



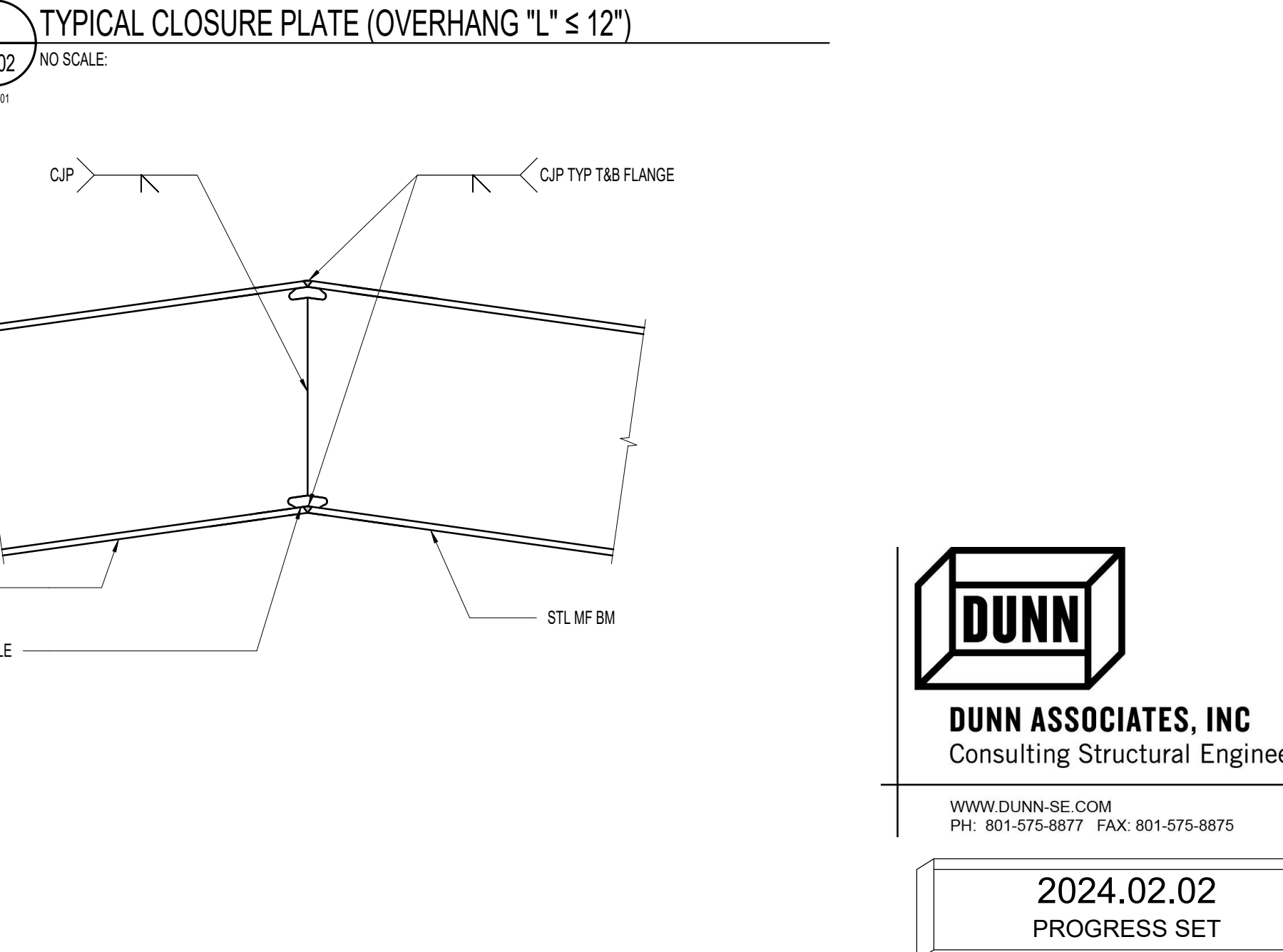
7 TYPICAL CLOSURE PLATE (OVERHANG "L" ≤ 12")
S-702 NO SCALE.
1/28/10



8 TYPICAL BLOCKING BETWEEN BEAMS CONNECTION
S-702 NO SCALE.
1/28/10



9 TYPICAL BLOCKING BETWEEN BEAMS CONNECTION
S-702 NO SCALE.
1/28/10



10 TYPICAL SLOPED BROKEN BACK BEAM DETAIL
S-702 NO SCALE.
1/28/10

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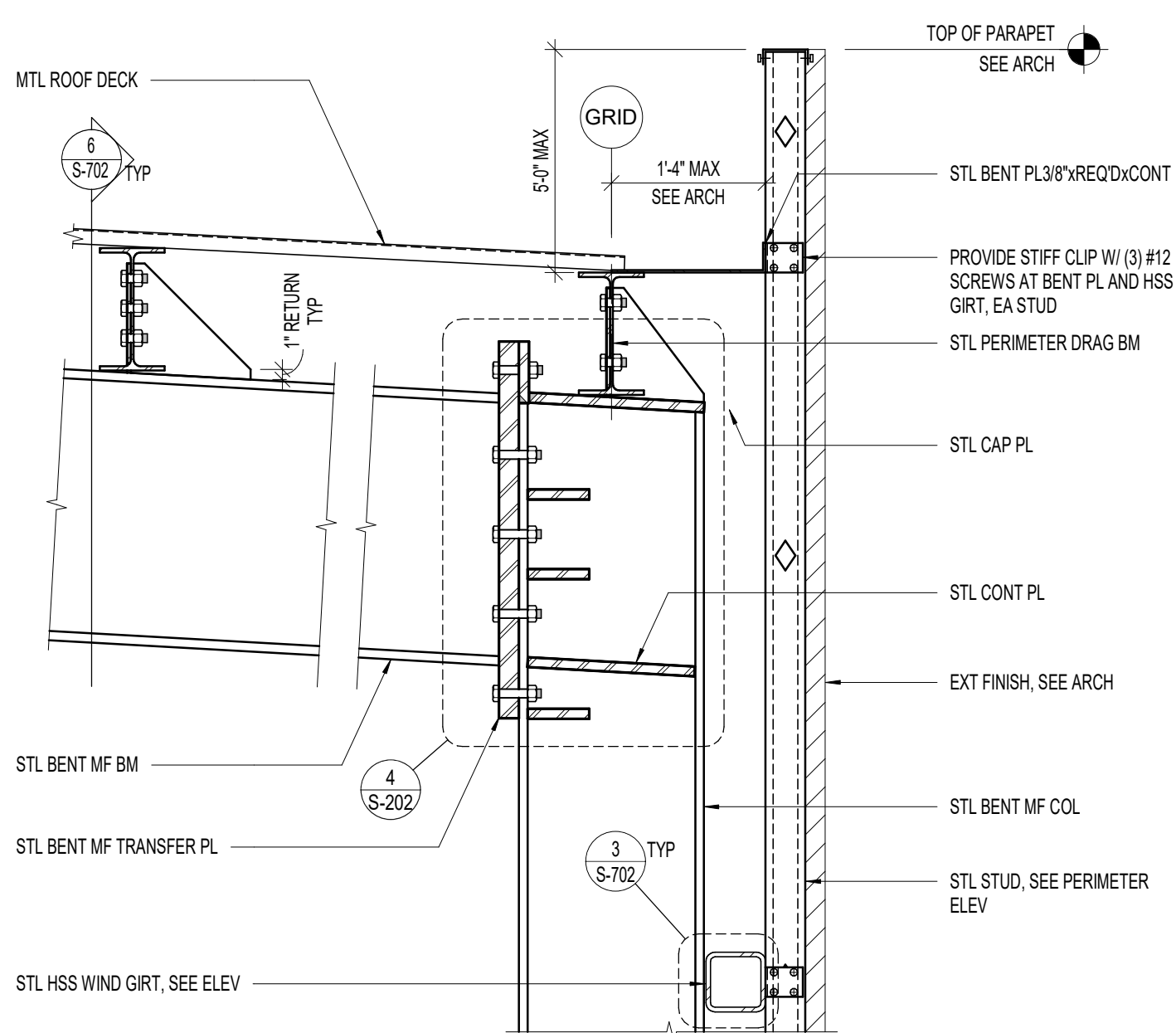
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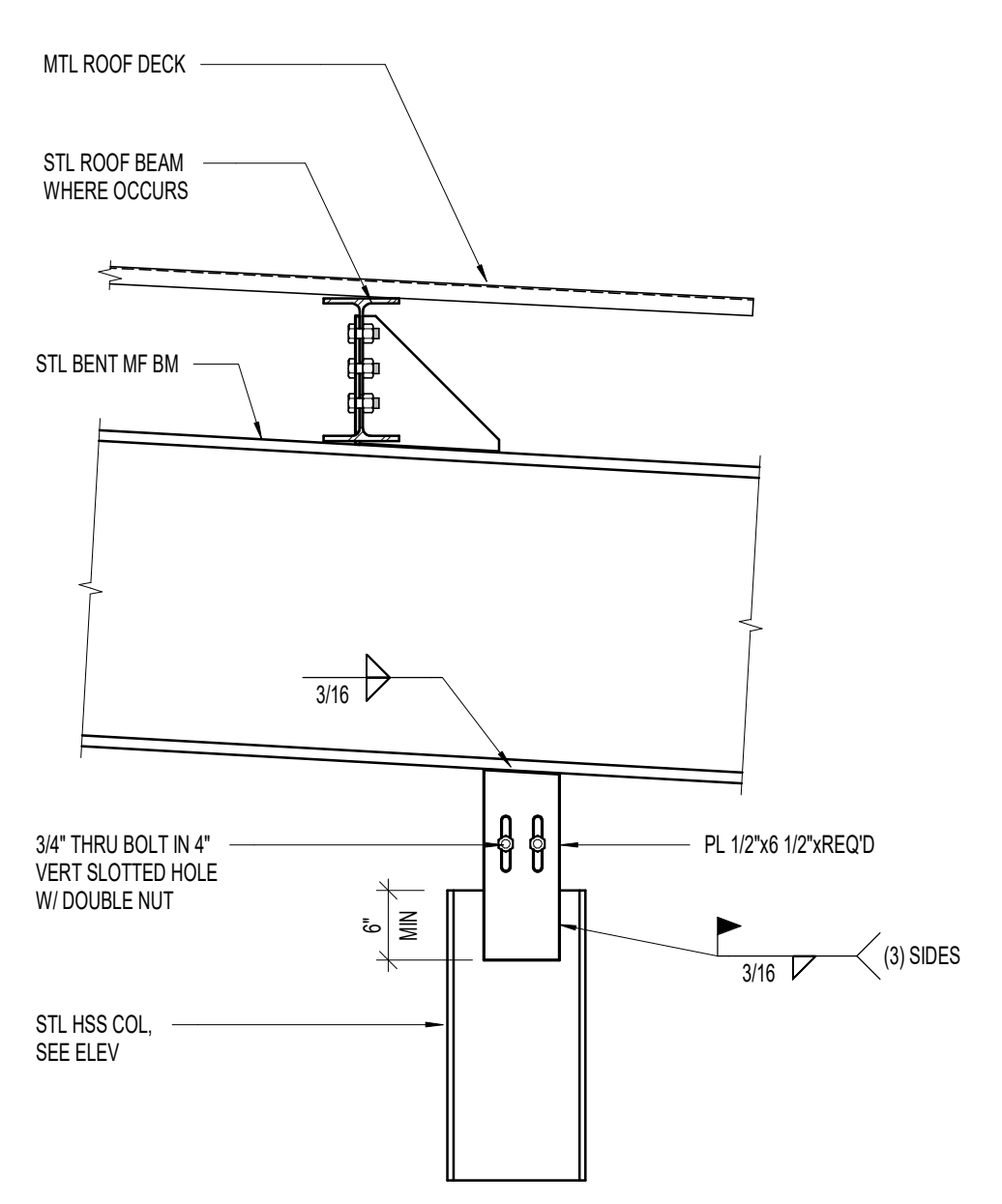
**ROOF FRAMING
DETAILS**

Scale	Date
Author	2024.02.02
Drawn	Project No.
Author	230103

S-703



1 PERIMETER CLADDING TO MOMENT FRAME BENT DETAIL
S-703 NO SCALE



2 EXTERIOR COLUMN TO MOMENT FRAME BENT DETAIL
S-703 NO SCALE

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Project Name
ALBANY MEZZANINE

Sheet Title
SCHEDULES

Scale
Date
2024.02.02

Drawn
Project No.
JDD 230103

Sheet No.
S-801

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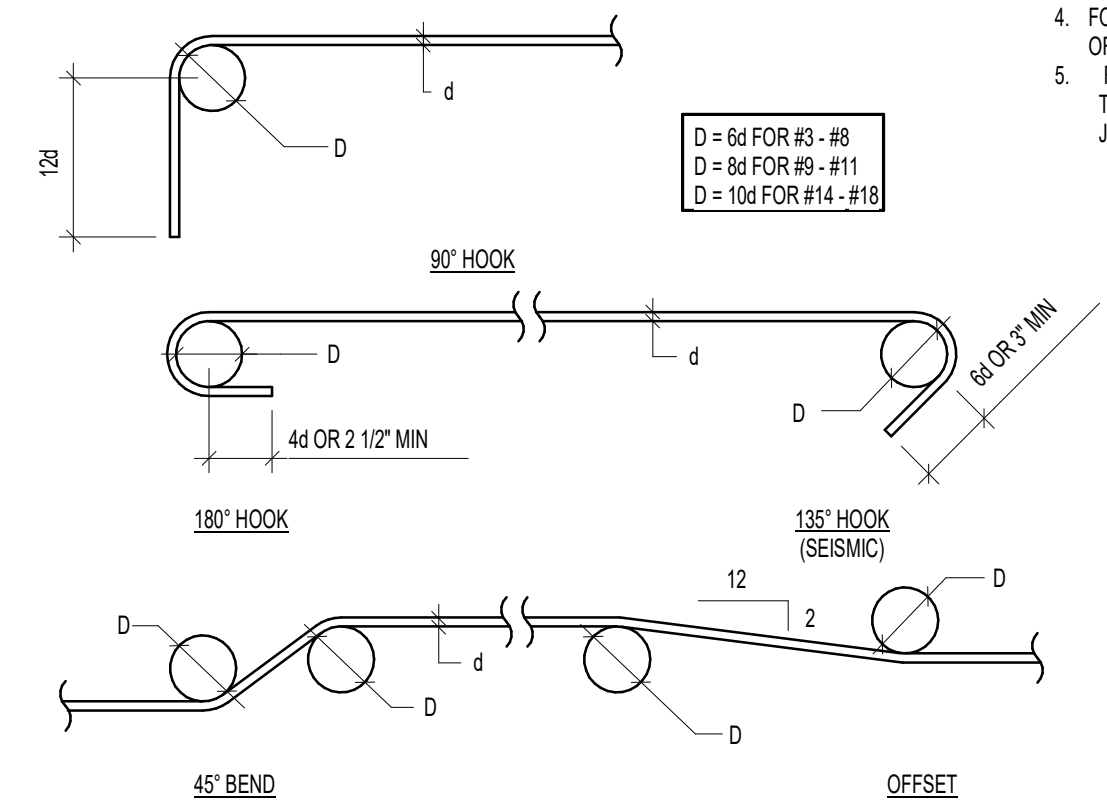
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S-801

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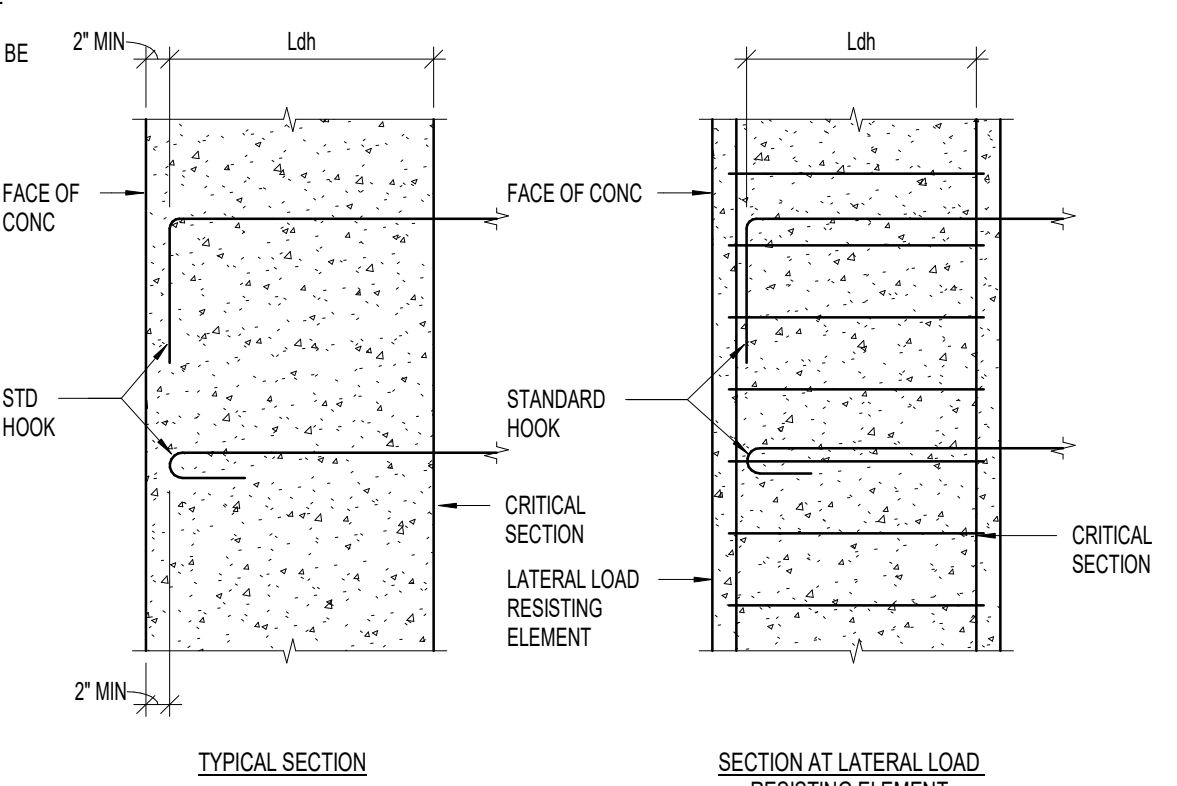
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	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP	
	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	CLASS	
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#4	22"	29"	29"	38"	21"	27"	27"	36"	19"	25"	25"	33"	18"	24"	24"	31"	17"	23"	23"	29"	16"	21"	21"	27"
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#6	33"	43"	43"	56"	31"	40"	40"	52"	29"	37"	37"	49"	27"	35"	35"	46"	26"	34"	34"	44"	24"	31"	31"	40"
#7	48"	63"	63"	81"	45"	59"	59"	75"	42"	54"	54"	71"	40"	51"	51"	67"	38"	49"	49"	63"	34"	45"	45"	58"
#8	55"	72"	72"	93"	51"	67"	67"	82"	48"	62"	62"	81"	45"	59"	59"	76"	43"	56"	56"	72"	39"	51"	51"	66"
#9	62"	81"	81"	105"	58"	75"	75"	98"	54"	70"	70"	91"	51"	66"	66"	86"	48"	63"	63"	81"	44"	57"	57"	74"
#10	70"	91"	91"	118"	65"	85"	85"	110"	61"	79"	79"	102"	57"	74"	74"	96"	54"	71"	71"	92"	50"	64"	64"	84"
#11	78"	101"	101"	131"	73"	94"	94"	122"	67"	87"	87"	114"	64"	82"	82"	107"	60"	78"	78"	102"	55"	71"	71"	93"

- NOTES:**
- THIS SCHEDULE SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE.
 - HORIZONTAL BARS ARE CLASSIFIED AS TOP BARS WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BARS.
 - CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS NOTED OTHERWISE.
 - TIES AND STIRRUPS SHALL NOT BE SPLICED.
 - FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
 - FOR ALL EPOXY COATED BARS, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5 FOR BARS WITH CLEAR COVER LESS THAN 3 BAR DIAMETERS OR CLEAR SPACING LESS THAN 6 BAR DIAMETERS, OTHERWISE MULTIPLY BY 1.2.
 - LAP LENGTHS SHALL BE MULTIPLIED BY 1.25 AT SHEARWALL BOUNDARY ELEMENTS.
 - DEVELOPMENT LENGTH 'L_d' IS EQUAL TO CLASS 'A' SPLICE.
 - IF REINFORCING HAS CLEAR COVER LESS THAN ONE BAR DIAMETER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
 - IF REINFORCING IS NOT ENCLOSED IN TIES OR STIRRUPS AND IS SPACED TIGHTER THAN 2 BAR DIAMETERS ON CENTER, LAP LENGTHS SHALL BE MULTIPLIED BY 1.5.
 - LAP LENGTHS SHALL BE MULTIPLIED BY 1.25 FOR GRADE 75 REBAR.
 - WHERE BARS OF DIFFERENT SIZES ARE LAPPED, THE SPLICE LENGTH SHALL BE THE LARGER OF L_d OF THE LARGER BARS AND THE SPLICE LENGTH OF THE SMALLER BAR.



BAR SIZE	f _c = 3000 PSI	f _c = 4000 PSI	f _c = 4500 PSI	f _c = 5000 PSI	f _c = 6000 PSI
#3	9"	8"	7"	7"	6"
#4	11"	10"	9"	9"	8"
#5	14"	12"	12"	11"	10"
#6	17"	15"	14"	13"	12"
#7	20"	17"	16"	15"	14"
#8	22"	19"	18"	17"	16"
#9	25"	22"	21"	20"	18"
#10	28"	25"	23"	22"	20"
#11	31"	27"	26"	24"	22"

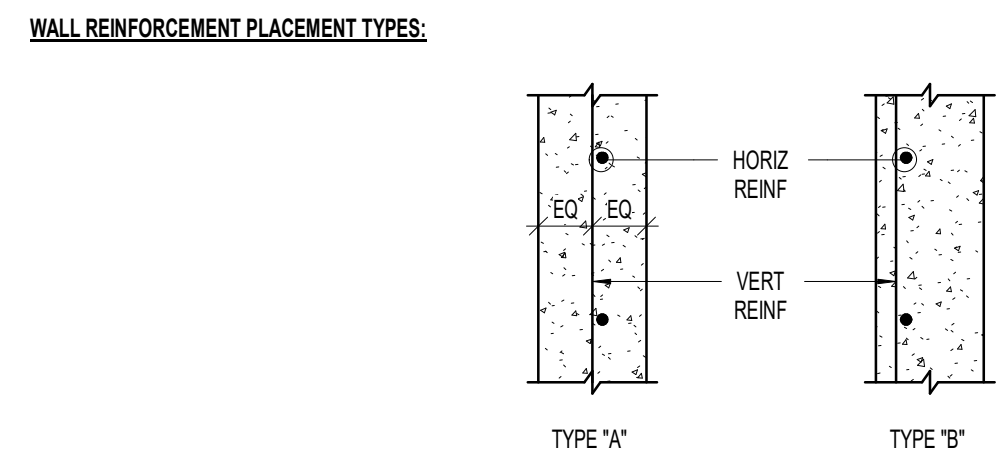
- NOTES:**
- FOR GRADE 75 REBAR, MULTIPLY LENGTHS BY 1.25.
 - FOR LIGHTWEIGHT CONCRETE, MULTIPLY LENGTHS BY 1.3.
 - FOR EPOXY COATED REINFORCEMENT, MULTIPLY LENGTHS BY 1.2.
 - FOR HOOKS WITH 2.5" MINIMUM SIDE COVER PERPENDICULAR TO PLANE OF HOOK, MULTIPLY LENGTHS BY 0.7.
 - FOR LATERAL LOAD RESISTING ELEMENTS, CRITICAL SECTIONS SHALL BE TAKEN AS THE FACE OF THE HOOK AT CONFINED CORES OF COLUMN JOINTS OR SHEAR WALL BOUNDARY ZONE.



1 CONCRETE REINFORCING BAR LAP SCHEDULES AND DIAGRAMS
S-801 NO SCALE

MARK	THICKNESS	REINFORCING			WALL TYPE	COMMENTS
		VERTICAL	HORIZONTAL	TOP AND BOTTOM		
CW-08	8"	#4 AT 18"oc	#4 AT 12"oc	(1) #5	A	—

- CONCRETE WALL NOTES:**
- SEE GENERAL STRUCTURAL NOTES FOR COVER AND OTHER REQUIREMENTS NOT NOTED IN SCHEDULE.
 - CONCRETE WALLS NOT DESIGNATED ON THE PLANS SHALL BE REINFORCED AS FOLLOWS:
- | THICKNESS | VERTICAL REINFORCING | HORIZONTAL REINFORCING |
|-----------|--------------------------|--------------------------|
| 8" | #4 BARS AT 18"oc | #4 BARS AT 18"oc |
| 8" | #4 BARS AT 18"oc | #4 BARS AT 12"oc |
| 10" | #4 BARS AT 18"oc | #5 BARS AT 15"oc |
| 12" | #4 BARS AT 18"oc EA FACE | #4 BARS AT 16"oc EA FACE |
- PLACE STEEL IN THE CENTER OF THE WALL (EXCEPT TYPE 'B' AND RETAINING WALLS). WALLS THICKER THAN 10" SHALL HAVE TWO CURTAINS OF REINFORCEMENT (PLACED NEAR EA FACE OF THE WALL), UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.



3 CONCRETE WALL SCHEDULE
S-801 NO SCALE

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2024.02.02
PROGRESS SET

NOTE:
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Project Name
ALBANY MEZZANINE

Sheet Title
SCHEDULES

Scale
2024.02.02

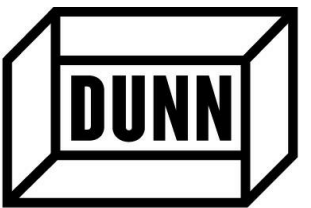
Date
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NOTE:
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Drawn
JDD

Project No.
230103

Sheet No.
S-802



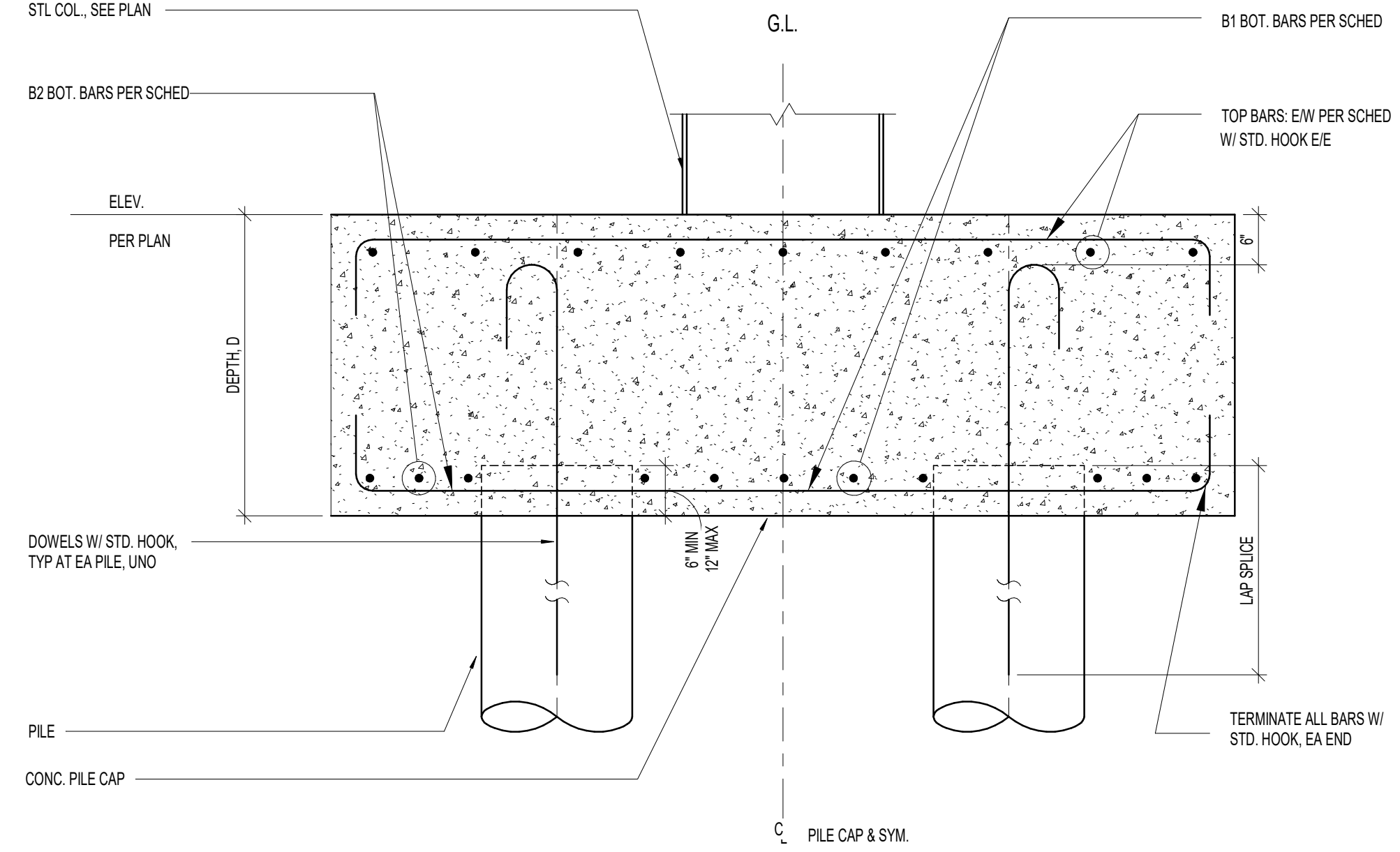
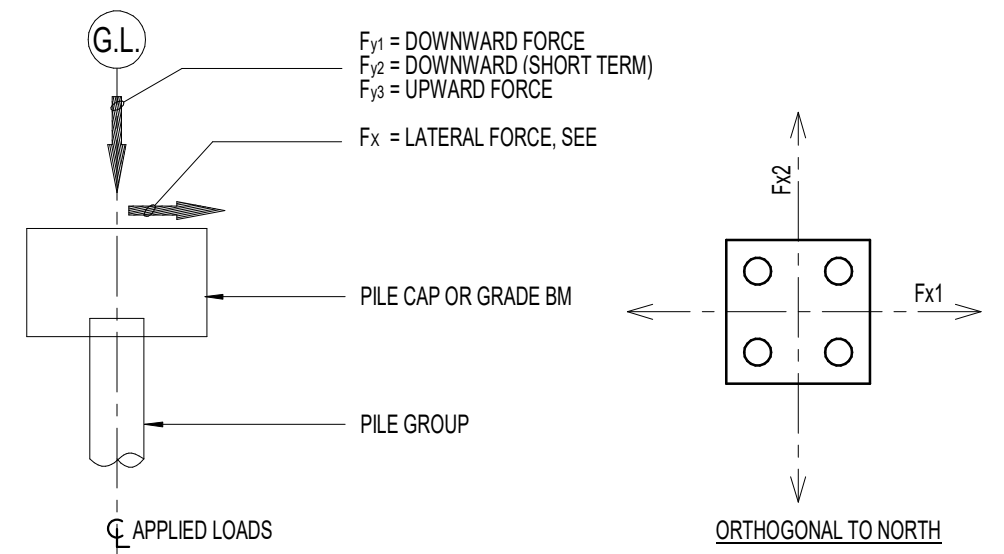
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Consulting Structural Engineers

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PH: 801-575-8877 FAX: 801-575-8875

2024.02.02
PROGRESS SET

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MARK	LENGTH (L)	WIDTH (B)	DEPTH (D)	BOT. BARS (B1)	BOT. BARS (B2)	TOP BARS	DETAIL
PC-5	DET	DET	2'-6"	(5) #8	(5) #8	(5) #8 EW	(3 / S-802)
PC-7	DET	DET	3'-0"	(7) #8	(7) #8	(7) #8 EW	(4 / S-802)

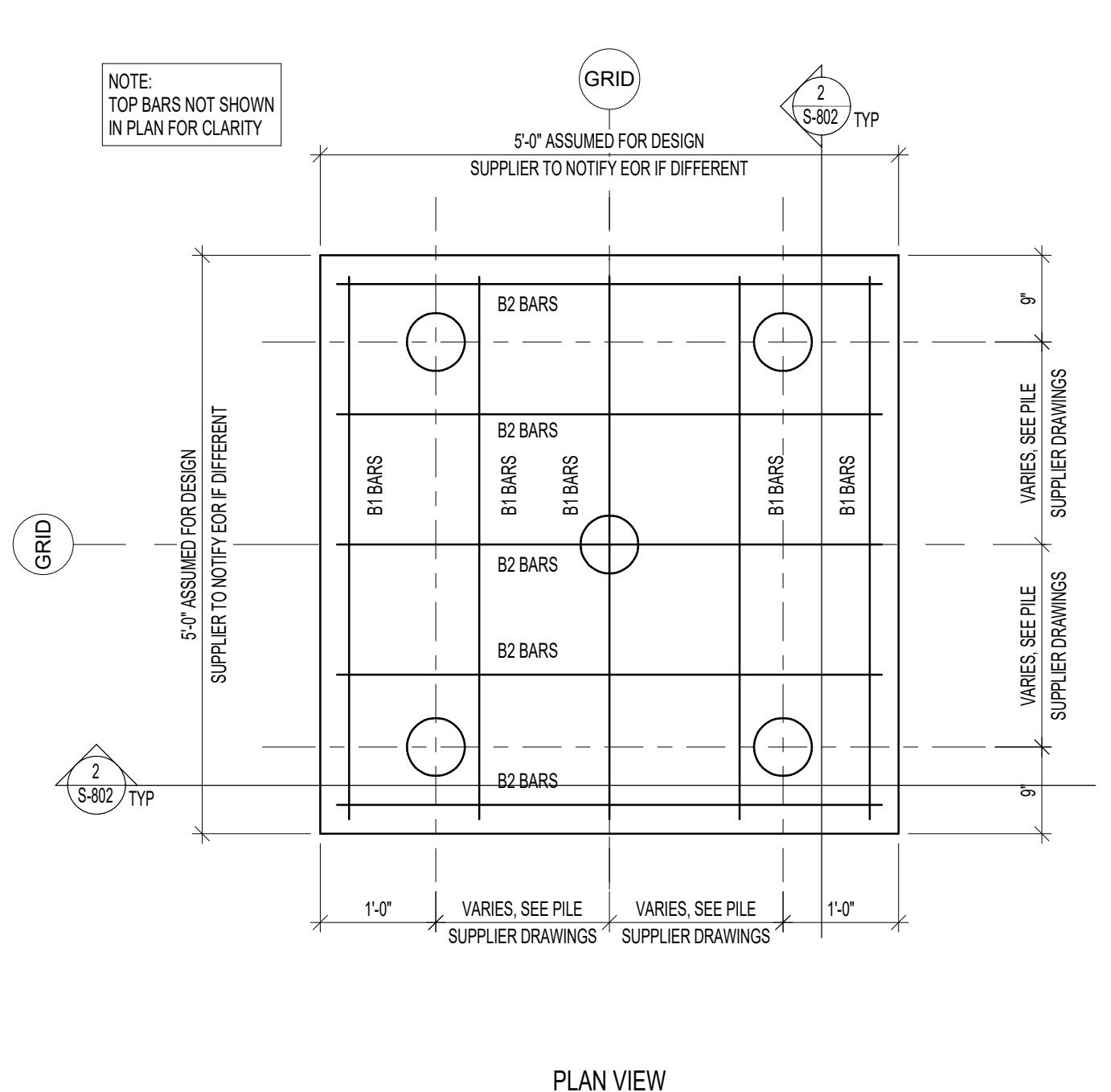


LOCATION / MARK	ASD (kip)			STRENGTH (kip)			LATERAL (kip)	
	Fy1	Fy2	Fy3	Fy1	Fy2	Fy3	Fx (ASD)	Fx (SD)
PC-19B-M	140	-	-	-	-	-	-	-
PC-19B-N	120	-	-	-	-	-	-	-
PC-19B-O	180	-	-	-	-	-	-	-
PC-19B-P	200	285	140	270	380	200	100	140
PC-19B-Q	200	285	140	270	380	200	100	140
PC-20.1-M	200	285	140	270	380	200	100	140
PC-20.1-N	200	285	140	270	380	200	100	140
PC-20.1-O	200	285	140	270	380	200	100	140
PC-20.1-P	200	285	140	270	380	200	100	140
PC-20.1-Q	200	285	140	270	380	200	100	140

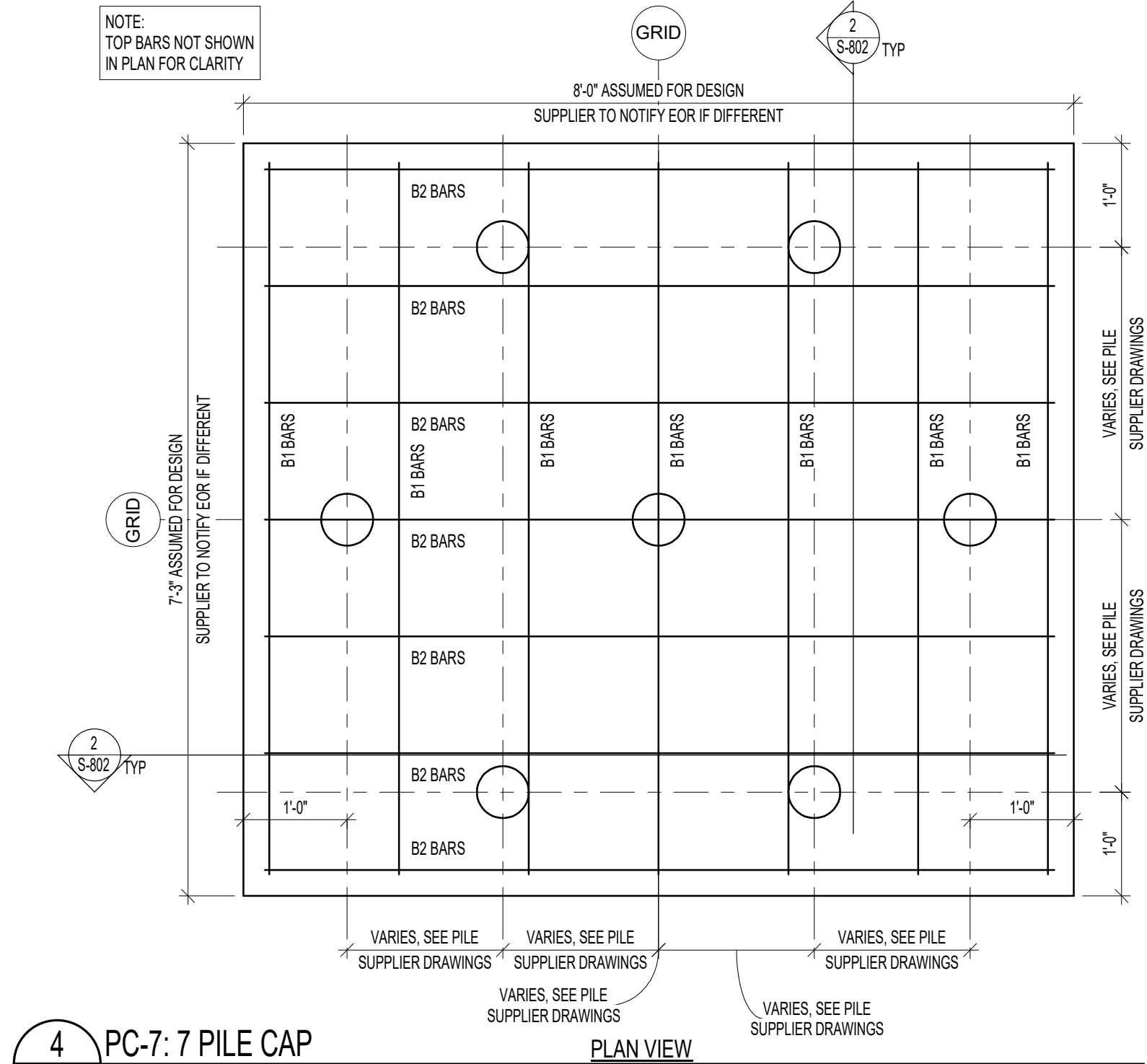
- NOTES:**
- PILES SHALL BE GROUTED MICRO-PILES. THE PILES SHALL BE DESIGNED IN ACCORDANCE WITH THE 2018 INTERNATIONAL BUILDING CODE (IBC) BY AN ENGINEER LICENSED IN THE STATE OF UTAH AND RETAINED BY THE CONTRACTOR.
 - THE CONTRACTOR SHALL SUBMIT PILE SHOP DRAWINGS AND STRUCTURAL CALCULATIONS STAMPED AND SIGNED BY THE CONTRACTOR'S ENGINEER. SHOP DRAWINGS WILL BE REVIEWED BY THE ARCHITECT, STRUCTURAL ENGINEER, AND GEOTECHNICAL ENGINEER AND ARE SUBJECT TO THE APPROVAL OF THE BUILDING DEPARTMENT.
 - PILES SHALL BE DESIGNED TO PROVIDE ADEQUATE RESISTANCE TO THE APPLIED ALLOWABLE STRESS DESIGN (ASD) LOADS LISTED IN THE TABLE FOR EACH PILE GROUP. REFERENCE PROJECT GEOTECHNICAL REPORT.
 - PILES SHALL BE DESIGNED TO PROVIDE STRENGTH ADEQUATE TO RESIST THE INTERNAL FORCES GENERATED BY THE APPLIED STRENGTH DESIGN LOADS LISTED IN THE TABLE FOR EACH PILE GROUP. THESE FORCES SHALL BE CONSIDERED IN COMBINATION WITH THE LATERAL LOADS (SEE NOTE #6). SEE DETAIL (B3/S802) FOR THE DIRECTION OF THE APPLIED LOAD.
 - PILES SHALL BE DESIGNED TO PROVIDE ADEQUATE RESISTANCE AND STRENGTH TO RESIST APPLIED LATERAL LOAD LISTED IN THE TABLE FOR EACH PILE GROUP. THE APPLIED LATERAL LOAD IS REPORTED AT THE ALLOWABLE STRESS DESIGN (ASD) FORCE LEVEL. SEE DETAIL (B3/S802) FOR THE DIRECTION OF THE APPLIED LOAD.
 - PILES SHALL BE DESIGNED AS PINNED HEAD OR FIXED HEAD AS REQUIRED TO MEET DEFLECTION LIMIT. LATERAL DEFLECTION SHALL BE LIMITED TO 1" UNDER ALLOWABLE STRESS DESIGN LEVEL LOADS.
 - THE NUMBER OF PILES SHOWN IN EACH GROUP ON PLAN CAN BE ADJUSTED BY THE CONTRACTOR PROVIDED THAT THE ADEQUACY OF THE MODIFIED DESIGN IS DEMONSTRATED. THE MODIFIED DESIGN SHALL USE ONE OF THE STANDARD CONFIGURATIONS SHOWN AND SHALL MAINTAIN A MINIMUM PILE SPACING OF 3 PILE DIAMETERS.
 - PILE INSTALLATION SHALL BE PERFORMED UNDER THE OBSERVATION OF THE GEOTECHNICAL ENGINEER.
 - PILE DESIGNER TO COORD PILE HEAD DETAILING WITH ANCHOR BOLT SETTING TO AVOID CONFLICTS. PILE EMBEDMENT MAY INCREASE AS REQ'D AT THICKENED REGION AT MOMENT FRAME COLUMN (MFC) PILE CAPS AS REQUIRED.

1 PILE GROUP LOAD SCHEDULE
S-802 NO SCALE.

2 PILE CAP SCHEDULE
S-802 NO SCALE.



3 PC-5: 5 PILE CAP
S-802 NO SCALE.



4 PC-7: 7 PILE CAP
S-802 NO SCALE.

No.	Description	Date

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Project Name
ALBANY MEZZANINE

Sheet Title
SCHEDULES

Scale
2024.02.02
Date
PROGRESS SET

NOTE:
THESE STRUCTURAL DRAWINGS ARE BASED ON ARCHITECTURAL MODEL DATED 03.16.2023

Drawn
JDD
Project No.
230103

Sheet No.
S-803

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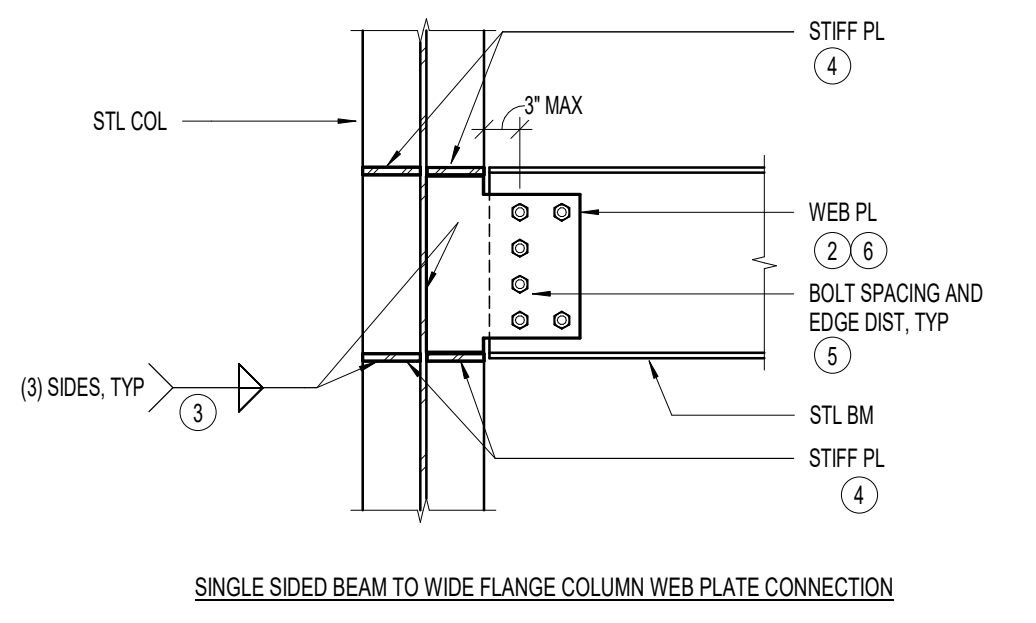
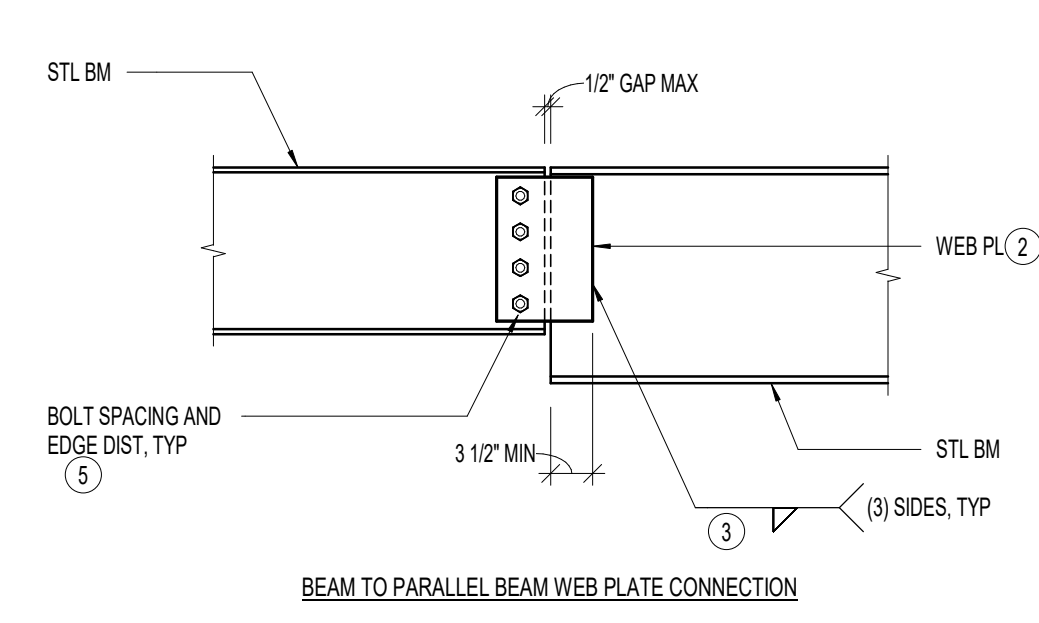
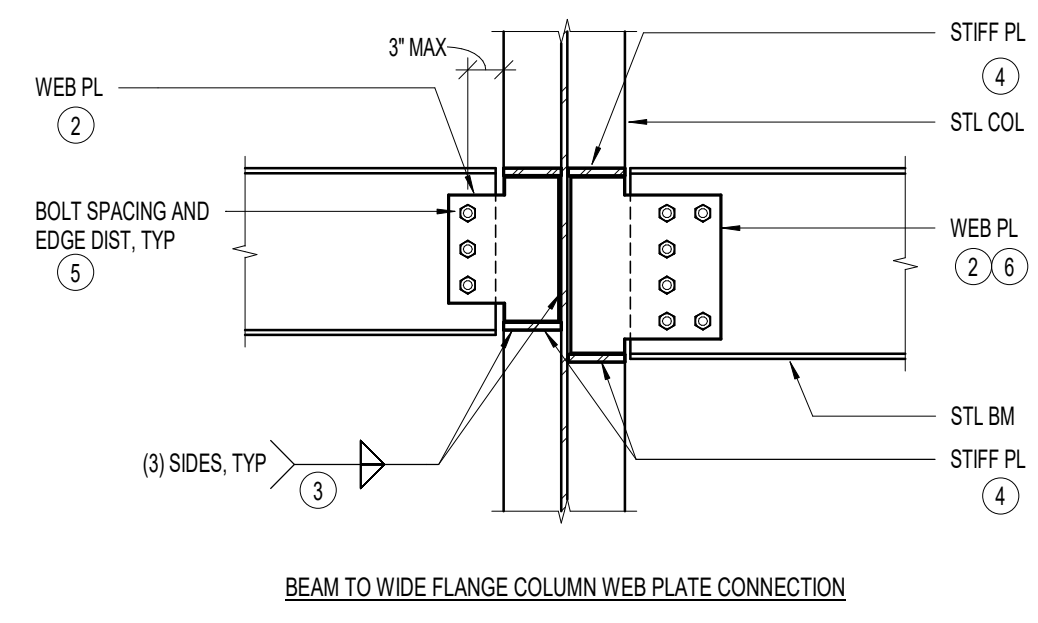
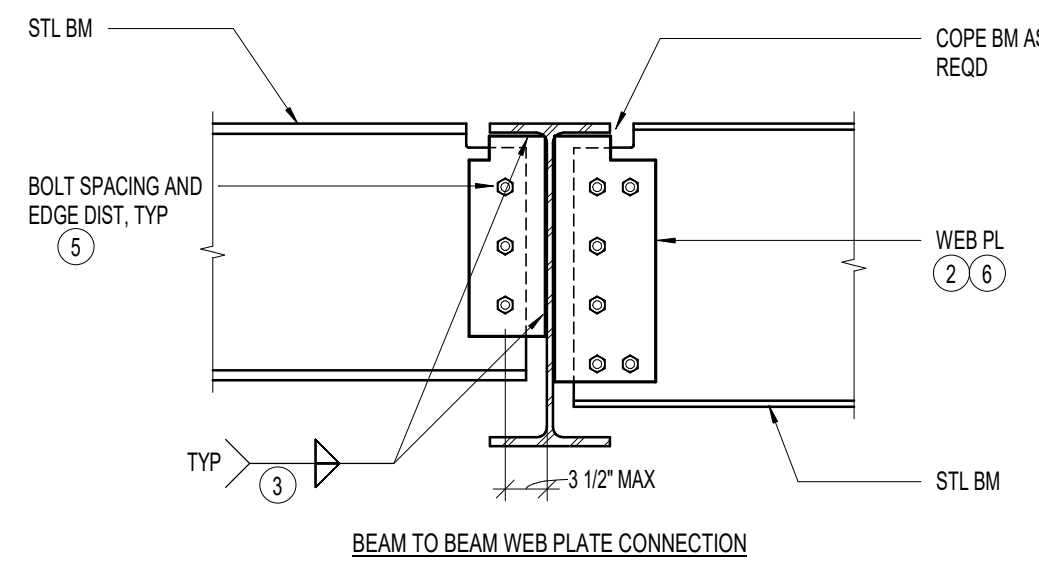
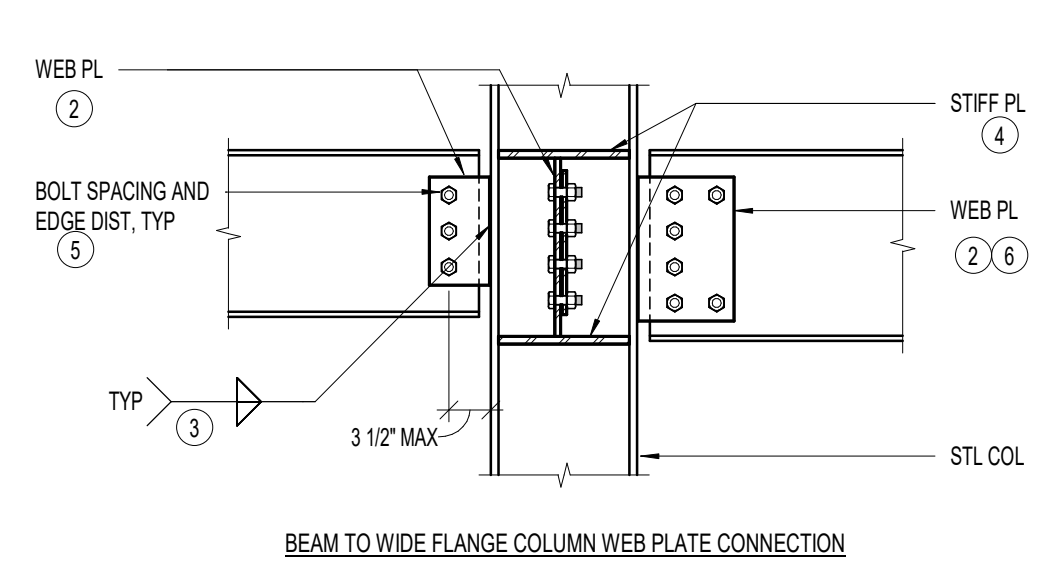
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2024.02.02
PROGRESS SET

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A-325N BOLT SCHEDULE		
MAXIMUM BEAM SIZE IN EACH BEAM DEPTH GROUP	A-325N BOLTS	
	No. PER BEAM	SIZE
W8	2	7/8"Ø
W10	2	7/8"Ø
W12	3	7/8"Ø
W14	3	7/8"Ø
W16	4	7/8"Ø
W18	5	7/8"Ø
W21	6	7/8"Ø
W24	6	7/8"Ø
W27	7	7/8"Ø
W30	8	7/8"Ø

- CLIP ANGLES: L5x3 1/2. THICKNESS SHALL BE EQUAL TO ONE HALF THE BEAM WEB THICKNESS PLUS 1/16" (1/16" MIN). FOR TWO ROWS OF BOLTS OR SKEWED CONNECTIONS, USE BENT PLATES, WHERE COLUMN WIDTH IS SMALLER THAN THE CONNECTING CLIP ANGLES. ANGLE LEGS SHALL BE REDUCED TO MATCH WIDTH OF COLUMN.
- BEAM WEB CONNECTION PLATE THICKNESS EQUALS 3/8" MINIMUM THICK FOR W18 BEAMS OR SMALLER 1/2" MINIMUM THICK FOR W21 BEAMS OR LARGER 3/4" MINIMUM THICK FOR BEAMS WITH WEB GREATER THAN 1" THICK
- FILLET WELDS SHALL BE AS FOLLOWS:
1/4" FOR 3/8" PLATES
5/16" FOR 1/2" PLATES
7/16" FOR 3/4" PLATES
- THICKNESS EQUALS BEAM FLANGE THICKNESS OF BEAM FRAMING INTO COLUMN WEB (3/8" MINIMUM).
- BOLT EDGE DISTANCE SHALL BE 1 1/2" MINIMUM AT ALL EDGES. BOLT SPACING SHALL BE AT 3". BOLT SPACING MAY BE REDUCED TO 3x THE BOLT DIAMETER IF IT IS REQUIRED FOR A SINGLE ROW OF BOLTS. A SINGLE ROW OF BOLTS IS PREFERRED.
- WHEN MORE THAN ONE COLUMN OF BOLTS IS NEEDED, THE FIRST COLUMN SHALL BE COMPLETE WITH THE REMAINDER OF THE BOLTS PLACED IN THE SECOND COLUMN.
- 1/2" PLATE THICKNESS + 5/16"



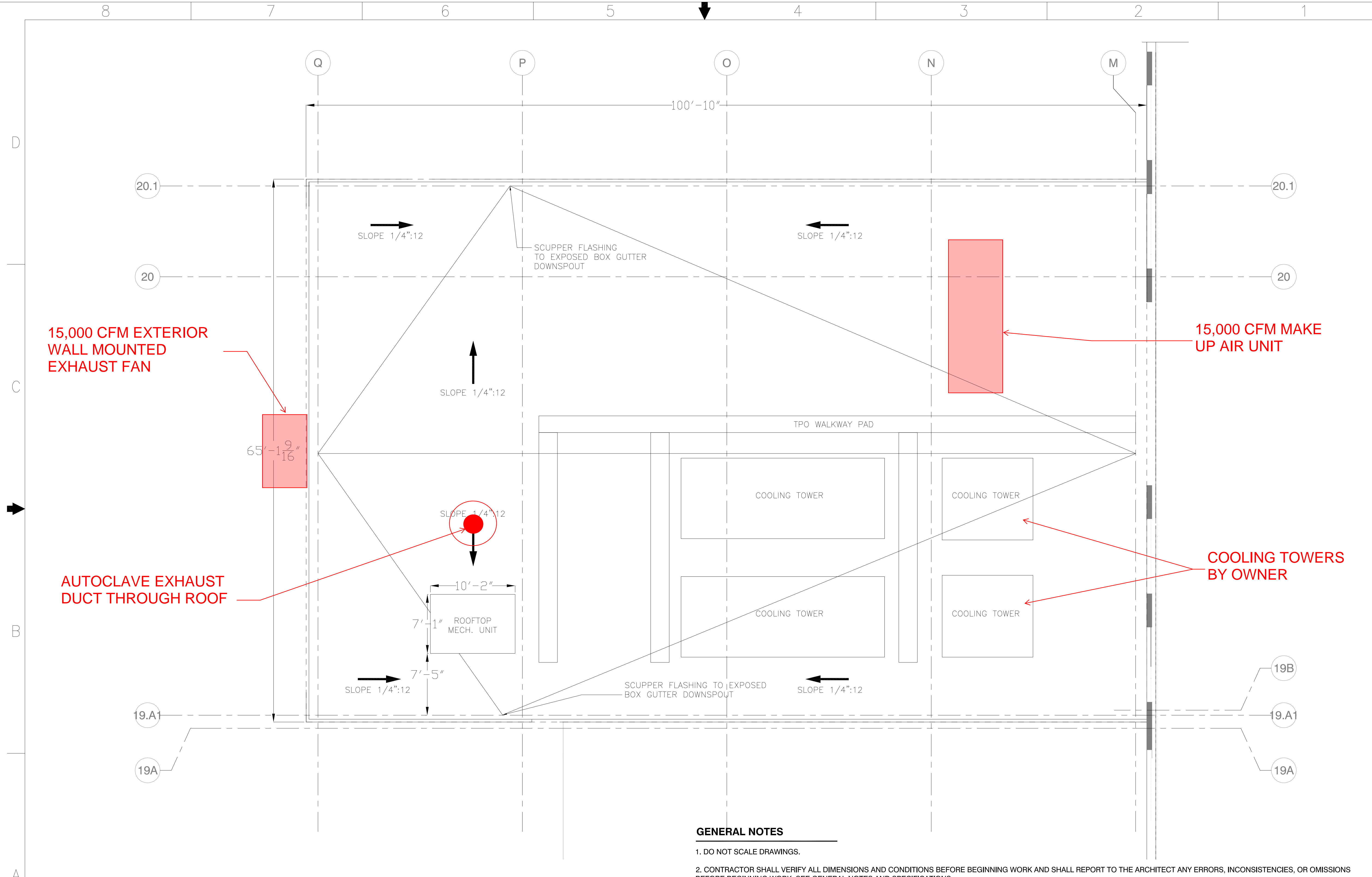
1 TYPICAL BOLTED WEB PLATE CONNECTIONS WITH BOLT SCHEDULE (SINGLE SHEAR)
S-803 NO SCALE

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No.	Description	Date
1	CLIENT REVIEW - PRICING	02/06/24
2		
3		
4		
5		
6		

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GENERAL NOTES

- DO NOT SCALE DRAWINGS.
- CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE BEGINNING WORK AND SHALL REPORT TO THE ARCHITECT ANY ERRORS, INCONSISTENCIES, OR OMISSIONS BEFORE BEGINNING WORK. SEE GENERAL NOTES AND SPECIFICATIONS.
- CONTRACTOR TO TAKE NECESSARY MEASURES TO PROTECT THE EXISTING BUILDING FROM DAMAGE
- CONTRACTOR TO PROVIDE SHOP DRAWINGS AND SUBMITTALS TO ARCHITECT / OWNER FOR REVIEW AND APPROVAL.
- REFER TO STRUCTURAL DRAWINGS AND CALCULATIONS TO CONFIRM ALL SIZES AND ATTACHMENTS.
- CONTRACTOR TO COMPLY WITH ALL REQUIRED SPECIAL INSPECTIONS IMPOSED BY TOWN OF ALTA.
- CONTRACTOR TO LEAVE JOB SITE CLEAN OF ALL DEBRIS AT ALL TIMES. COORDINATE WITH ALBANY FACILITIES DIRECTOR ON CONSTRUCTION STAGING AREA, MITIGATION PLAN AND DUMPSTER LOCATION AS NECESSARY. BUILDING OWNER TO PROVIDE RESTROOM FOR ANY CONSTRUCTION WORKERS.
- CONTRACTOR TO COORDINATE WITH LOCAL FIRE MARSHALL AS REQUIRED.
- COORDINATE ADDITIONAL DEFERRED FIRE SPRINKLER SUBMITTAL AS REQUIRED WITH FIRE MARSHALL AND AUTHORITY HAVING JURISDICTION.

1 ROOF PLAN
SCALE: 3/16" = 1'-0"



PLAN NORTH

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No.	Description	Date
1	OWNER REVIEW - PRICING	02/06/24
2		
3		
4		
5		
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Project Name
ALBANY NORTH ADD.

Sheet Title
LEVEL ONE FLOOR PLAN

Scale
3/16"=1'-0" Date
02.06.2024

Drawn
GxA Project No.
24-002

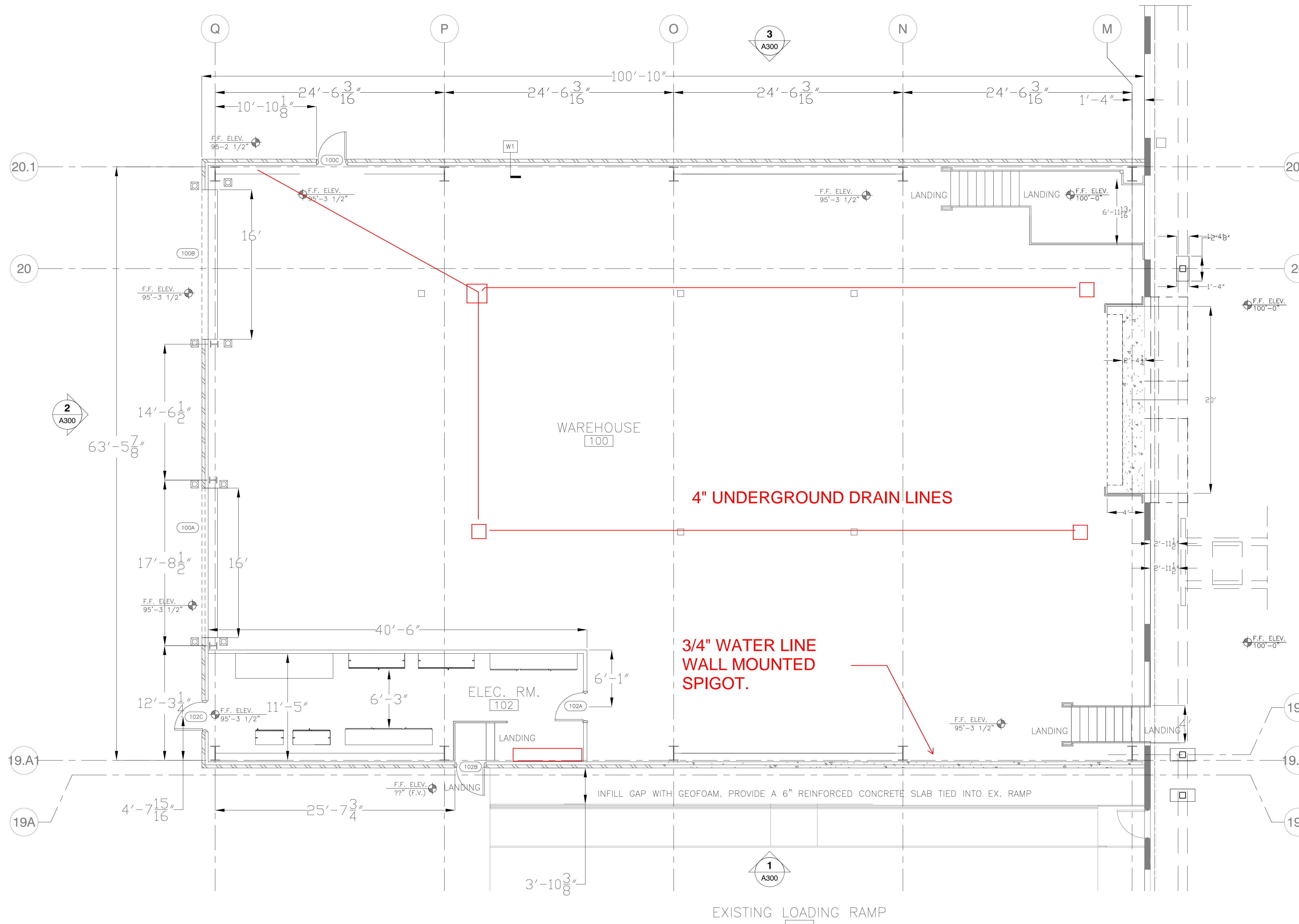
Sheet No.

P201



NORTH

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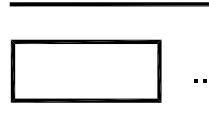


1 LEVEL ONE FLOOR PLAN
A201 SCALE: 3/16" = 1'-0"

GENERAL NOTES

- DO NOT SCALE DRAWINGS.
- NOT FOR CONSTRUCTION. FOR PRICING ONLY.
- CONTRACTOR TO VERIFY ALL QUANTITIES FOR TAKE OFF AND COST ESTIMATING.

GRAPHIC LEGEND



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ABBREVIATIONS	
ABBREV.	DESCRIPTION
A	AMP OR AMPS
AC	ABOVE COUNTER
AFF	ABOVE FINISHED FLOOR
AHJ	AUTHORITY HAVING JURISDICTION
AL	ALUMINUM
C	CONDUIT
CB	CIRCUIT BREAKER
CKT	CIRCUIT
CLG	CEILING
CORR	CORRIDOR
CU	COPPER
D	DRYER
DISP	DISPOSAL
DW	DISHWASHER
EM	EMERGENCY
EMT	ELECTRIC METALLIC TUBING
EWC	ELECTRIC WATER COOLER
E, EX	EXISTING
FA	FIRE ALARM
FACP	FIRE ALARM CONTROL PANEL
FLA	FULL LOAD AMPS
FMC	FLEXIBLE METAL CONDUIT
GND	GROUND CONDUCTOR
HP	HORSE POWER
IG	ISOLATED GROUND
IMC	INTERMEDIATE METAL CONDUIT
INS	INSULATED
ISO	ISOLATED
KVA	KILO VOLT AMPERES
KW	KILOWATTS
LFMC	LIQUID TIGHT METAL CONDUIT
LTG	LIGHTING
LVL	LEVEL
MCM	MINIMUM CIRCUIT AMPS
MCB	MAIN CIRCUIT BREAKER
MDU	MEDIA DISTRIBUTION UNIT
MLO	MAIN LUGS ONLY
MW	MICROWAVE
NIC	NOT IN CONTRACT
NL	NIGHT LIGHT
OC	ON CENTER(S)
OCP	OVER CURRENT PROTECTION
PFR	PHASE FAILURE RELAY
RCPT	RECEPTACLES
REQ	REQUIREMENTS
RELT	REDUCED ENERGY LET THROUGH DEVICE
RMC	RIGID METAL CONDUIT
RMP	ROCKY MOUNTAIN POWER
RNC	RIGID NONMETALLIC CONDUIT
SPD	SURGE PROTECTION DEVICE
SS	SURGE SUPPRESSION
TR	TAMPER RESISTANT
TYP	TYPICAL
TTB	TELEPHONE TERMINAL BOARD
UG	UNDERGROUND
W	WASHER
WP	WEATHERPROOF
XFMR	TRANSFORMER

LIGHTING SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	LED LIGHT FIXTURE
	LED LIGHT FIXTURE - EMERGENCY
	RECESSED LED DOWN LIGHT
	RECESSED LED DOWNLIGHT - EMERGENCY
	RECESSED LED WALL WASH OR SPOT FIXTURE
	LED STRIP LIGHT
	LED STRIP LIGHT - EMERGENCY
	LED LINEAR LIGHT
	LED LINEAR LIGHT - EMERGENCY
	SURFACE OR PENDANT MOUNTED LED LIGHT
	SURFACE OR PENDANT MOUNTED LED LIGHT - EMERGENCY
	RECESSED LED WAFER LIGHT
	RECESSED LED WAFER LIGHT - EMERGENCY
	WALL MOUNTED LED LIGHT FIXTURE
	WALL MOUNTED LED LIGHT FIXTURE - EMERGENCY
	LED TRACK LIGHT HEAD
	FAN
	LED WALL MOUNTED EXIT SIGN - SINGLE SIDED - ARROWS INDICATE DIRECTION
	LED EXIT SIGN - SINGLE SIDED - ARROWS INDICATE DIRECTION
	LED EXIT SIGN - DOUBLE SIDED
	LED EXIT SIGN WITH INTEGRAL EMERGENCY LIGHT
	LED EMERGENCY LIGHT WITH INTEGRAL BATTERY
	SINGLE-POLE SWITCH
	SWITCH - LOWER CASE LETTER INDICATES ZONE
	SWITCH - LOW VOLTAGE
	SWITCH - 3 WAY
	SWITCH - 4 WAY
	SWITCH - DIMMER
	OCCUPANCY SENSOR - CEILING MOUNTED
	OCCUPANCY SENSOR - WALL MOUNTED
	OCCUPANCY SENSOR - CEILING MOUNTED DUAL CIRCUIT
	PHOTOCELL
	MANUAL OVERRIDE SWITCH

CALLOUTS/NOTES LEGEND	
SYMBOL	DESCRIPTION
	MECHANICAL EQUIPMENT CALLOUT
	REVISION CALLOUT
	LIGHT FIXTURE CALLOUT, TOP = TYPE, MIDDLE = CKT #, BOTTOM = SWITCH
	OWNER PROVIDED EQUIPMENT CALLOUT
	KEYED NOTE
	WIRE CONDUIT - ALUMINUM
	WIRE CONDUIT - COPPER
	DETAIL CALLOUT
	ELEVATION CALLOUT

FIRE ALARM SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	FIRE ALARM CONTROL PANEL
	NAC PANEL
	FIRE/SMOKE DAMPER
	SMOKE DETECTOR WITH VISUAL - CEILING MOUNTED
	SMOKE DETECTOR WITH VISUAL - WALL MOUNTED
	SMOKE DETECTOR
	COMBINATION SMOKE/CARBON DETECTOR

POWER/DATA SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	SIMPLEX RECEPTACLE
	TWIST-LOCK RECEPTACLE
	DUPLEX RECEPTACLE
	DUPLEX RECEPTACLE - GFCI
	DROP DUPLEX RECEPTACLE - GFCI
	HALF-SWITCHED DUPLEX RECEPTACLE
	FLOOR BOX WITH DUPLEX 120V RECEPTACLE
	FLOOR BOX WITH 4-PLEX RECEPTACLE AND VOICE/DATA OUTLET
	FOURPLEX RECEPTACLE
	FOURPLEX RECEPTACLE - GFCI
	BLANK FACE - GFCI
	DROP FOURPLEX RECEPTACLE - GFCI
	SPECIAL PURPOSE RECEPTACLE - THREE PHASE
	SPECIAL PURPOSE RECEPTACLE - SINGLE PHASE
	NON-FUSED DISCONNECT SWITCH
	FUSED DISCONNECT SWITCH
	COMBINATION STARTER/FUSED DISCONNECT
	STARTER
	ELECTRICAL PANEL
	TELEPHONE TERMINAL BOARD W/GROUND BUSS BAR
	DOOR CONTACTOR
	ELECTRIC STRIKE
	EMERGENCY POWER OFF
	MAGNETIC LOCK
	PUSH BUTTON
	POWER PACK
	ROOM CONTROLLER
	REQUEST TO EXIT
	THERMAL SWITCH
	ELECTRIC MOTOR
	J-BOX
	J-BOX FOR DATA/VOICE
	METER
	VARIABLE FREQUENCY DRIVE
	TV OUTLET, REFER TO AV / DATA DRAWINGS FOR CABLE TYPE
	POWER COMMUNICATIONS POLE FOR WORKSTATION FURNITURE
	VOICE RECEPTACLE
	DATA RECEPTACLE
	COMBINATION VOICE/DATA RECEPTACLE
	CEILING MOUNT SECURITY CAMERA ("TYPE" INDICATES TYPE OF CAMERA)
	WALL MOUNT SECURITY CAMERA ("TYPE" INDICATES TYPE OF CAMERA)
	CARD READER
	SECURITY SYSTEM KEYPAD
	DOORBELL - EXTERIOR
	DOORBELL - INTERIOR
	ADA PUSHBUTTON
	HOME RUN TO PANELBOARD
	CIRCUIT BREAKER
	ELECTRONIC TRIP CIRCUIT BREAKER
	FUSE

SITE LIGHTING SYMBOLS LEGEND	
SYMBOL	DESCRIPTION
	POLE LIGHT
	POLE LIGHT - TWIN HEAD
	BOLLARD LIGHT

GENERAL NOTES	
1.	REFER TO THE MECHANICAL SHEETS FOR THE EXACT LOCATION OF THE MECHANICAL EQUIPMENT.
2.	ALL METALLIC CONDUITS, JOINTS, FITTINGS, ETC., IN CONTACT WITH THE GROUNDS SHALL BE SPIRALLY WRAPPED WITH 3M SCOTCHWRAP-51, 20 MIL TAPE (OR APPROVED EQUAL).
3.	ALL UNDERGROUND CONDUIT SHALL BE BURIED 24" MINIMUM UNDER THE GROUND.
4.	FLEXIBLE CONDUITS CAN ONLY BE USED FOR SHORT RUNS (6' MAXIMUM).
5.	NO CONDUITS SHALL RUN IN DUCT WORK.
6.	ALL DUPLEX OUTLETS AND SWITCHES SHALL BE 20A, 120V SPEC GRADE, HUBBELL AND PASS & SEYMOUR AND LEVITON ARE APPROVED MANUFACTURERS.
7.	INSTALL EXIT SIGNS ON THE WALL IF POSSIBLE.
8.	SWITCHBOARDS, PANELBOARDS, AND MOTOR CONTROL CENTERS SHALL BE FIELD MARKED TO WARN QUALIFIED PERSONS OF POTENTIAL ARC FLASH HAZARDS. THE MARKING SHALL BE LOCATED SO AS TO BE CLEARLY VISIBLE TO QUALIFIED PERSONS BEFORE EXAMINATION, ADJUSTMENT, SERVICING, OR MAINTENANCE OF THE EQUIPMENT. (NEC 110-16).
9.	FOR 20 AMP CIRCUITS, USE NO. 10 THHN CONDUCTORS FOR CONDUCTOR LENGTH OVER 100 FEET, NO. 8 THHN OVER 200 FEET, NO. 6 THHN OVER 300 FEET AND NO. 4 THHN OVER 400 FEET.
10.	IF CONDUITS ARE TO BE PLACED WITHIN THE PT SLAB, CONDUITS ARE TO BE SPACED NO CLOSER THAN 4 CONDUIT DIAMETERS OR 4" O.C. WHICHEVER IS GREATER. LIMIT CONDUIT OUTER DIAMETER TO ONE FOURTH OF THE SLAB THICKNESS AND PLACE WITHIN THE CENTER THIRD OF SLAB THICKNESS. CONDUIT IS NOT TO BE TIED TO PARALLEL REBAR AND/OR TENDONS, NOR SHOULD REINFORCEMENT BE MOVED, BENT OR CUT TO ACCOMMODATE CONDUIT.
11.	AIC RATINGS OF ALL OVERCURRENT PROTECTIVE DEVICES SHALL BE EQUAL TO OR GREATER THAN THE AIC RATING SHOWN IN THE PLANS.
12.	SERVICE EQUIPMENT SHALL BE FIELD MARKED WITH CALCULATED MAXIMUM AVAILABLE FAULT CURRENT AND THE DATE IT WAS CALCULATED (NEC 110-24).
13.	IN OTHER THAN DWELLING UNITS, IN ADDITION TO THE REQUIREMENTS IN 1.10.16(A), A PERMANENT LABEL SHALL BE FIELD OR FACTORY APPLIED TO SERVICE EQUIPMENT RATED 1200 AMPS OR MORE. THE LABEL SHALL MEET THE REQUIREMENTS OF 110.21(B) AND CONTAIN THE FOLLOWING INFORMATION: (1) NOMINAL SYSTEM VOLTAGE. (2) AVAILABLE FAULT CURRENT AT THE SERVICE OVERCURRENT PROTECTIVE DEVICES. (3) THE CLEARING TIME OF SERVICE OVERCURRENT PROTECTIVE DEVICES BASED ON THE AVAILABLE FAULT CURRENT AT THE SERVICE EQUIPMENT. (4) THE DATE THE LABEL WAS APPLIED.

SEISMIC BRACING REQUIREMENTS	
HUNT ELECTRIC IS RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL REQUIRED SEISMIC BRACING BY THE 2018 INTERNATIONAL BUILDING CODE SECTION 1613 EARTHQUAKE LOADS PARAGRAPH 1613.1 SCOPE. RESTRAINT MUST BE PROVIDED FOR THE FOLLOWING CONDITIONS UNLESS OTHERWISE EXCLUDED BY CHAPTER 13 OF ASCE 7-10: (1) $l_p > 1.0$, (2) MEP COMPONENTS > 400 POUNDS AND SUPPORTED BY A FLOOR OR ROOF, (3) MEP COMPONENTS > 20 POUNDS AND SUPPORTED BY A CEILING OR WALL, OR (4) MEP DISTRIBUTION SYSTEMS WEIGHING > 5 PLF.	
HUNT ELECTRIC WILL PROVIDE A COMPLETE SUBMITTAL FOR ALL ELECTRICAL EQUIPMENT TO INCLUDE LOCATION OF EACH SEISMIC BRACE, TYPE AND DESIGN OF THE BRACING, AND A DETAIL OF THE SEISMIC BRACING. THE SUBMITTAL SHALL BE ON 36" X 24" SHEETS AND AT A SCALE OF 1/4" = 1'-0" ALONG WITH A COMPLETE SET OF CALCULATIONS.	
THE SUBMITTAL WILL CLEARLY INDICATE WHICH ITEMS ARE REQUIRED TO BE BRACED AND THE MINIMUM BRACING REQUIREMENTS (E.G. PER IBC 103.1 AND CHAPTER 13 OF ASCE 7-10). IN ADDITION THE SUBMITTAL SHALL BE PROVIDED BY A LICENSED PROFESSIONAL ENGINEER LICENSED IN UTAH.	

DEFERRED SUBMITTAL	
FIRE ALARM SHOP DRAWINGS, SUBMITTALS, BATTERY CALCULATIONS AND VOLTAGE DROP CALCULATIONS ARE A DEFERRED SUBMITTAL. THESE ITEMS SHALL BE SUBMITTED TO FIRE MARSHAL AND APPROVED BY FIRE MARSHAL PRIOR TO BEGINNING ANY WORK ON THE FIRE ALARM SYSTEM.	

ELECTRICAL SHEET INDEX	
SHEET	NAME
E001	NOTES, LEGENDS, SCHEDULES
E002	POWER SINGLE LINE DIAGRAM - SERVICE A PHASE 2
E003	ELECTRICAL DETAILS
E004	ELECTRICAL SCHEDULES
E005	ELECTRICAL SCHEDULES
E101	LEVEL 1 OVERALL ELECTRICAL POWER PLAN
E301	ENLARGED ELECTRICAL PLANS
E302	ENLARGED ELECTRICAL PLANS
E303	ENLARGED ELECTRICAL PLANS

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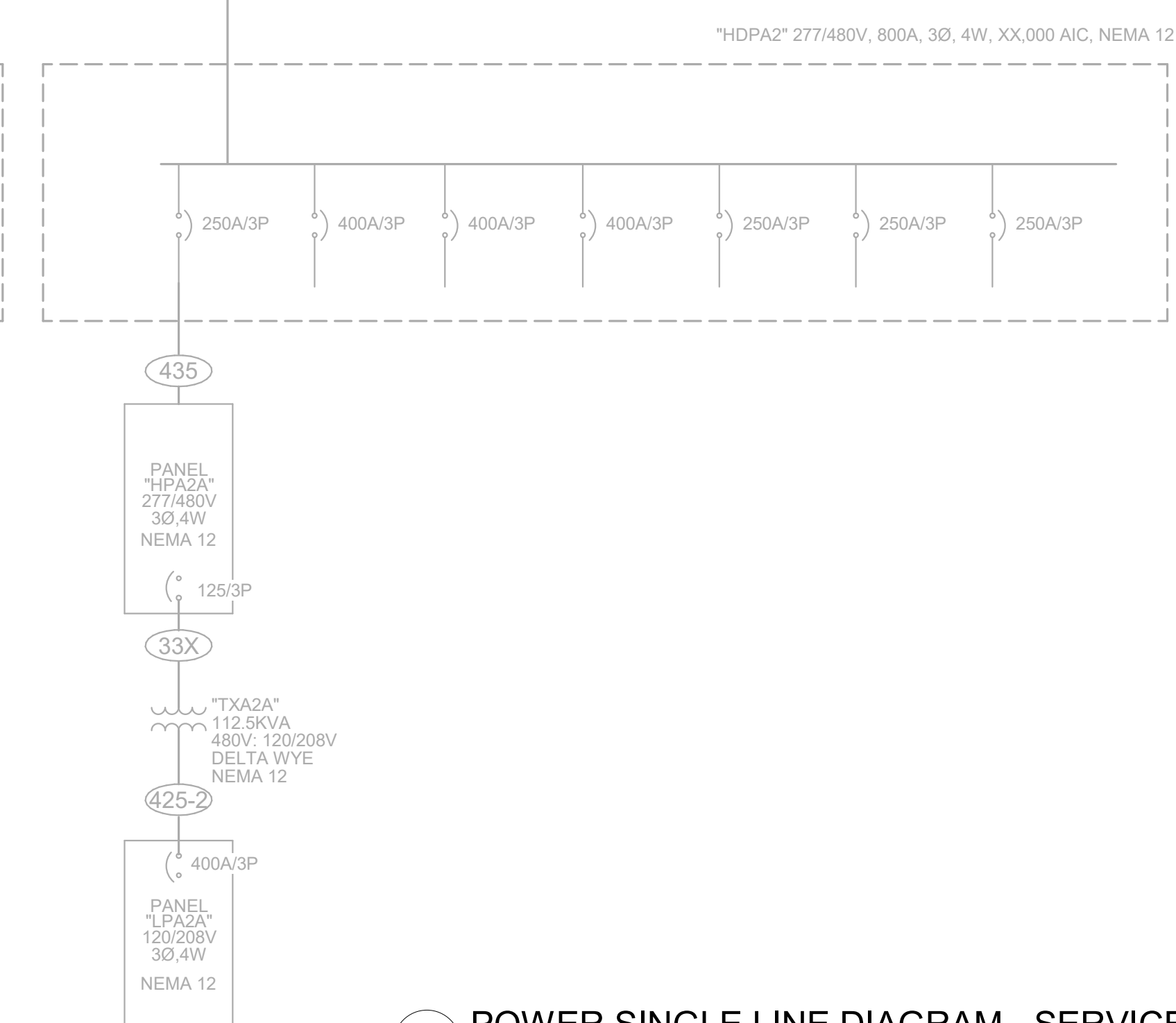
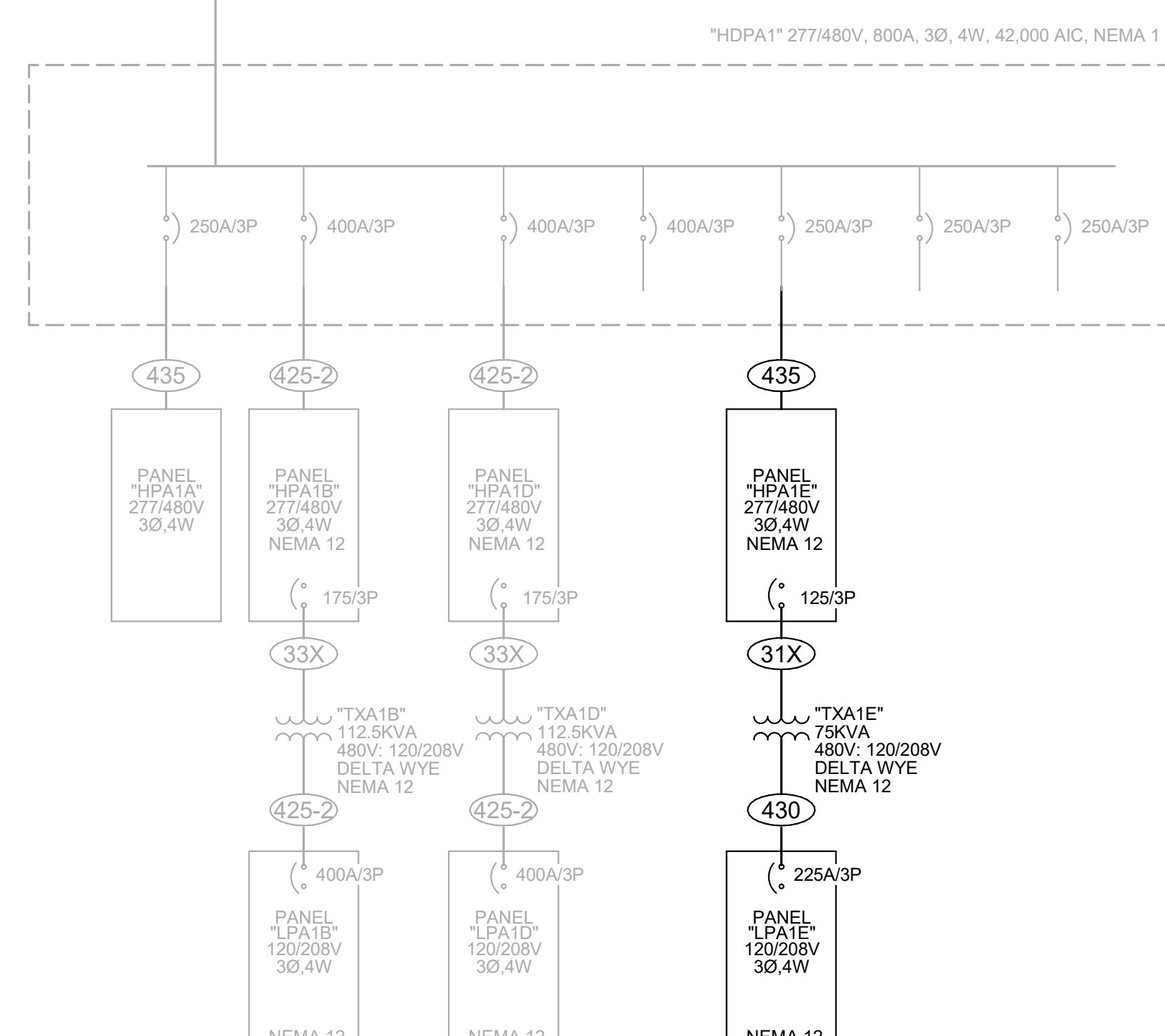
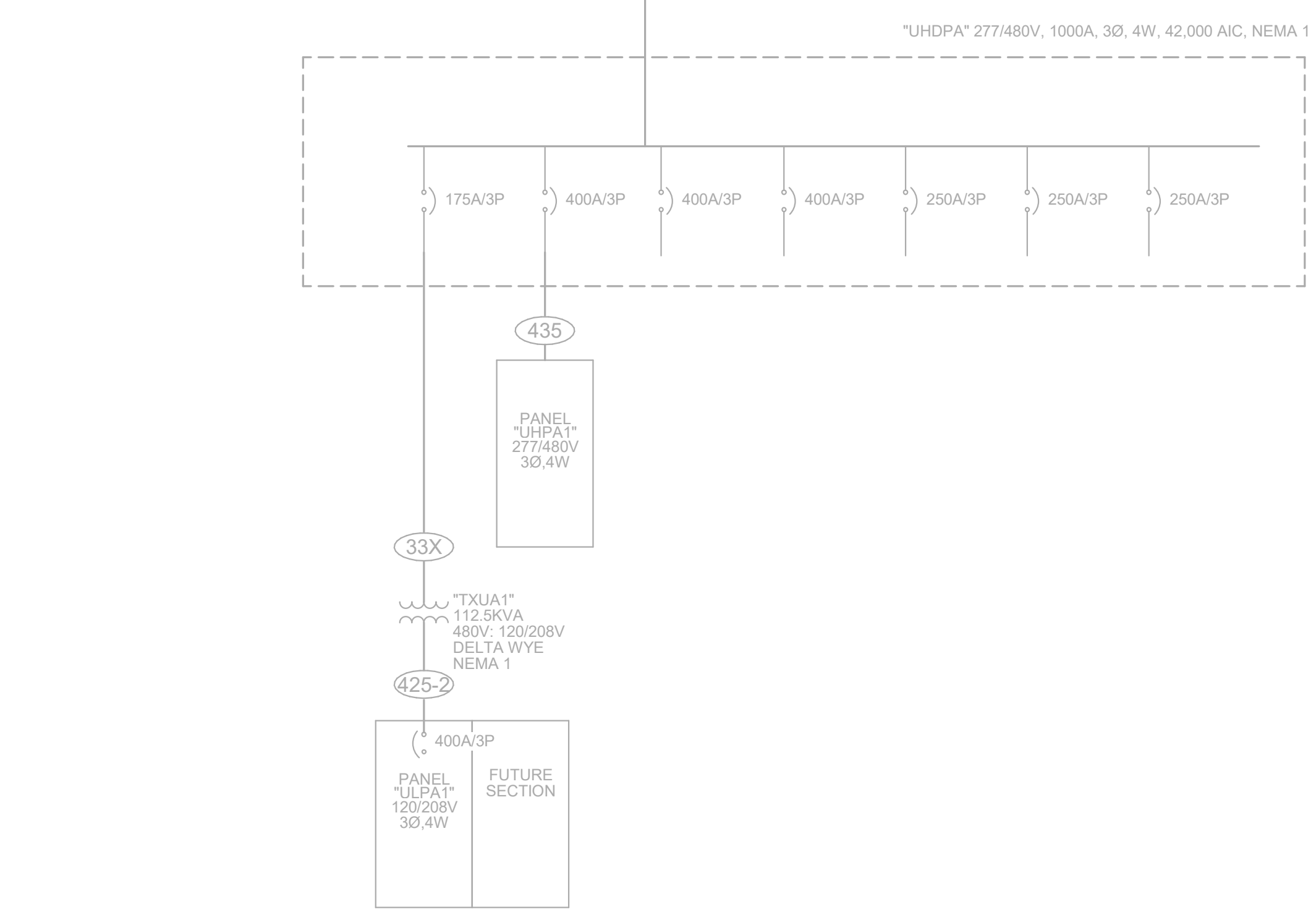
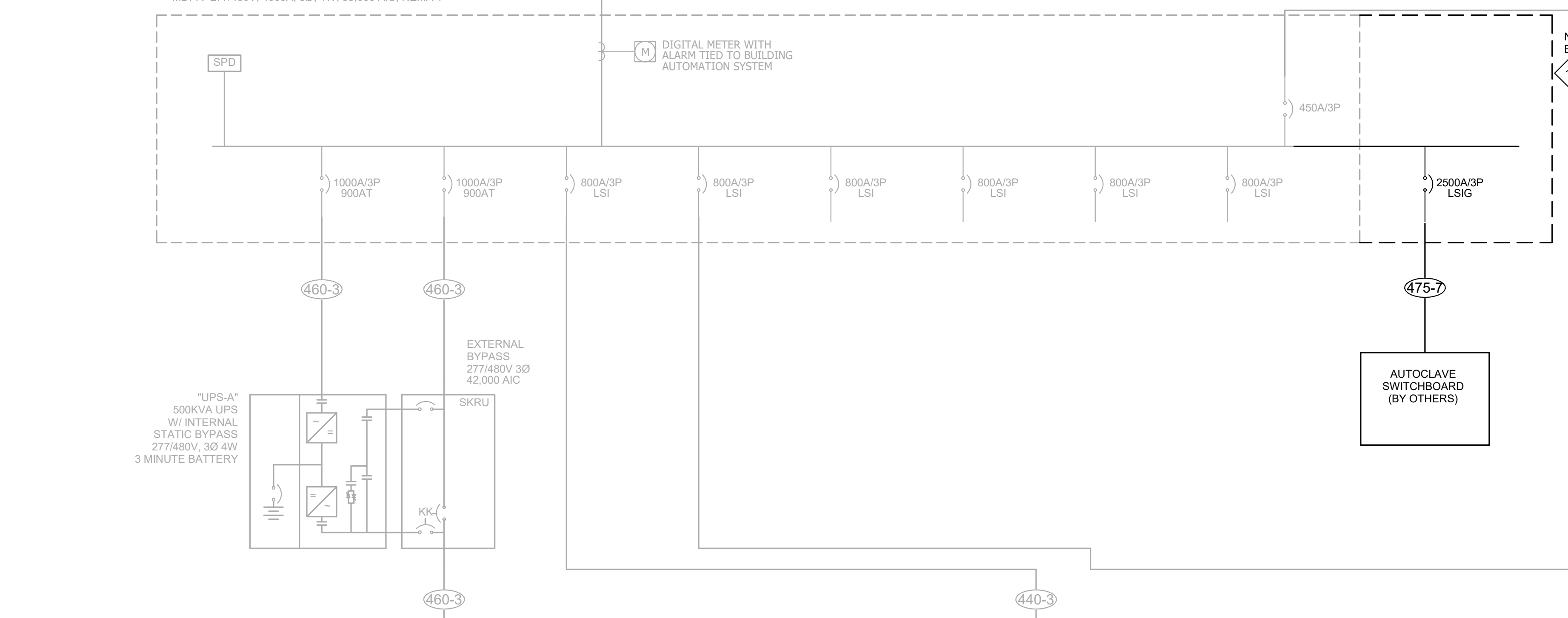
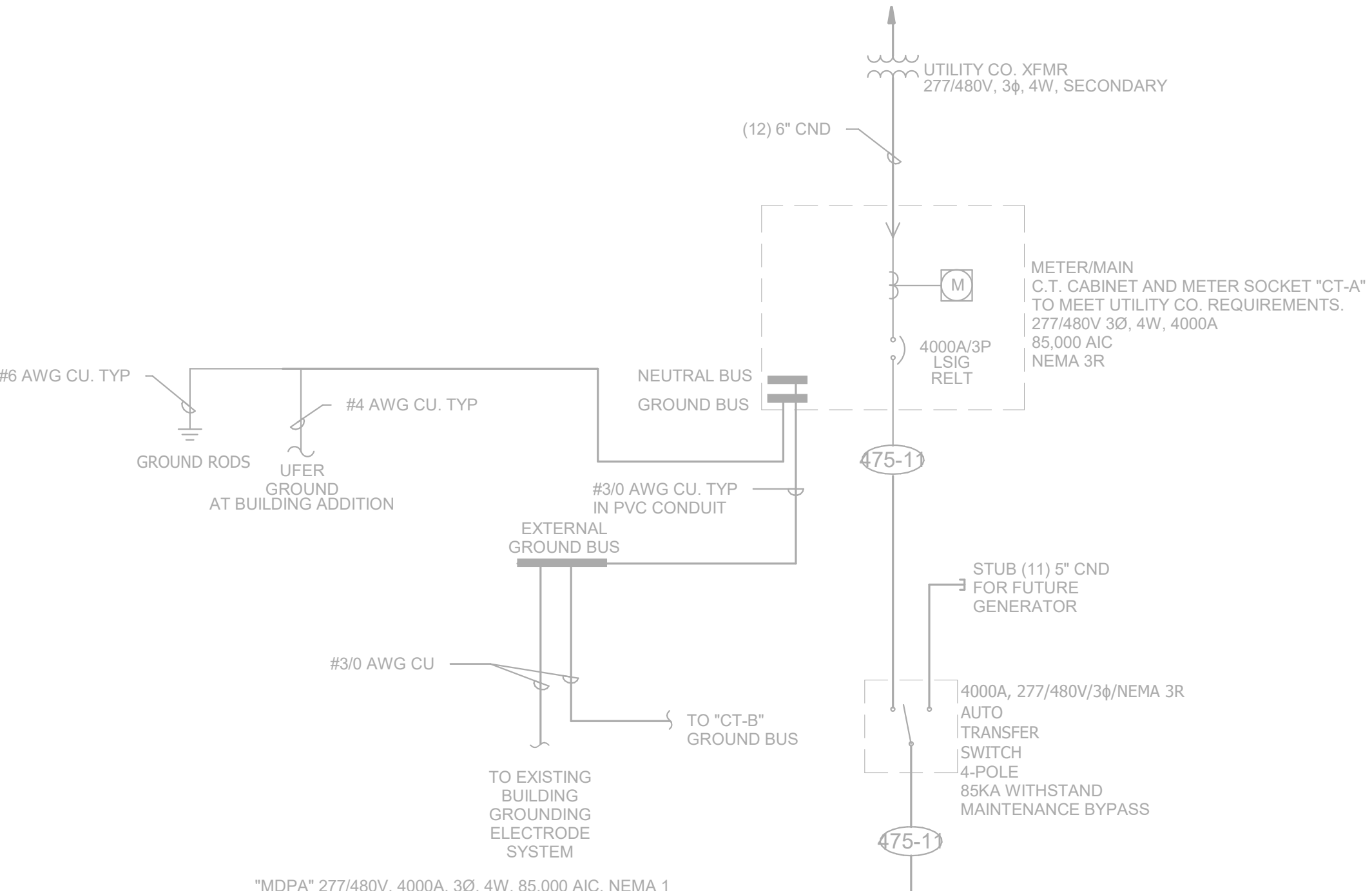
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DRAWN BY: _____ HE
ENGINEER: _____ AB

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5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
NOTES, LEGENDS, SCHEDULES
CLIENT REVIEW

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E001



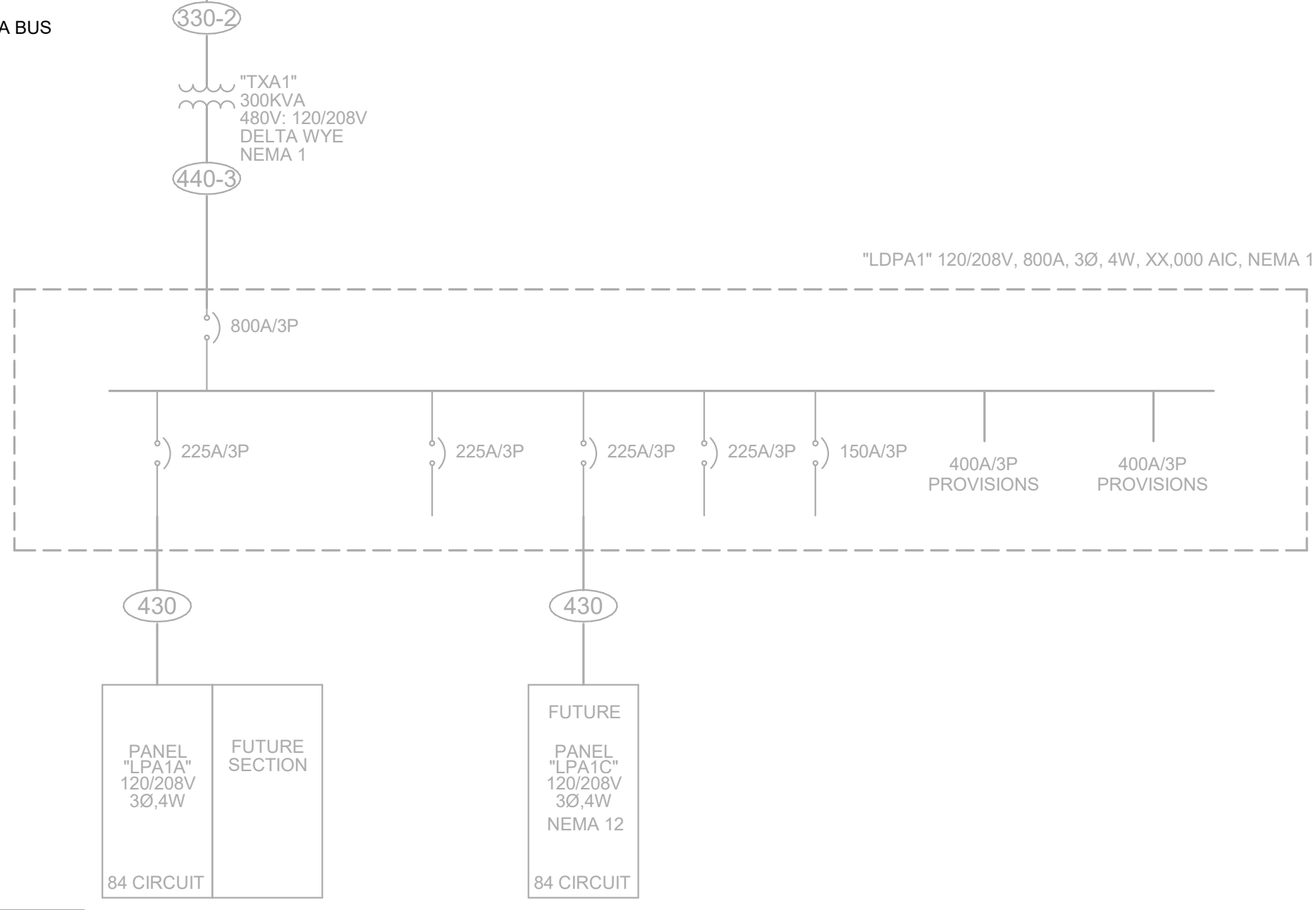
GENERAL NOTES

A.

KEYED NOTES

1 COORDINATE PLANNED POWER OUTAGE WITH OWNER PRIOR TO INSTALLATION OF NEW GEAR SECTION. BUDGET FOR AFTER HOURS WORK.

ALUMINUM FEEDER SCHEDULE										
TYPE	MAX PROT	CONDUCTOR AMPS	SETS	CONDUIT SIZE	CONDUCTOR QTY	CONDUCTOR SIZE	GND	PARALLEL	COPPER	ALUMINUM
31X	-	120	-	1-1/2"	3	1/0	4	N	N	Y
33X	-	155	-	2"	3	3/0	4	N	N	Y
330-2	-	460	2	3"	3	300	1/0	Y	N	Y
425-2	400	410	2	3"	4	250	1	Y	N	Y
430	-	230	-	3"	4	300	2	N	N	Y
435	-	250	-	3"	4	350	2	N	N	Y
440-3	800	810	3	3"	4	400	3/0	Y	N	Y
460-3	1000	1020	3	4"	4	600	4/0	Y	N	Y
475-7	2500	2695	7	4"	4	750	600	Y	N	Y
475-8	3000	3080	8	4"	4	750	600	Y	N	Y
475-11	4000	4235	11	5"	4	750	750	Y	N	Y



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QA/QC Completed Date: --/--

1 POWER SINGLE LINE DIAGRAM - SERVICE A PHASE 2
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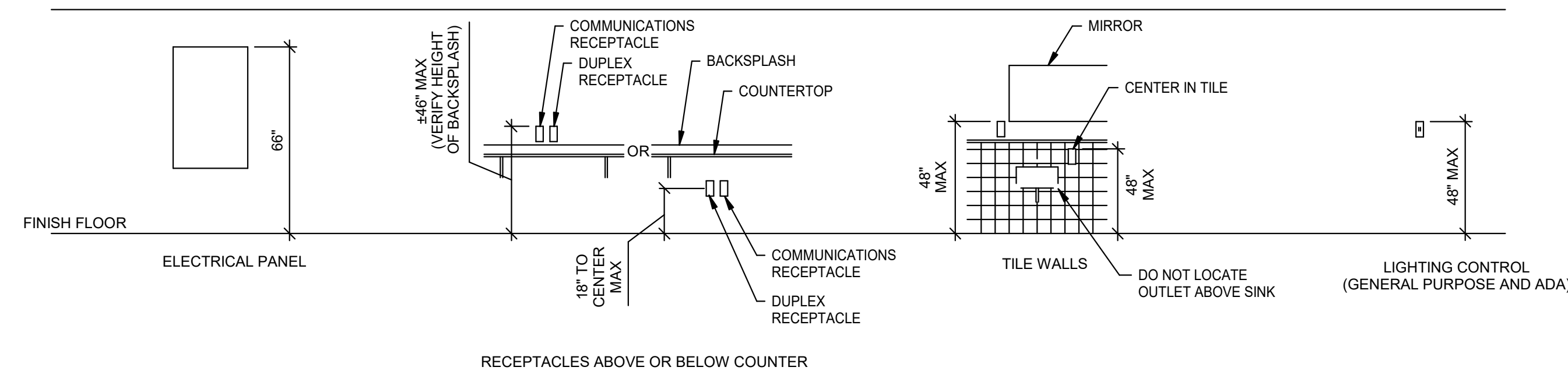
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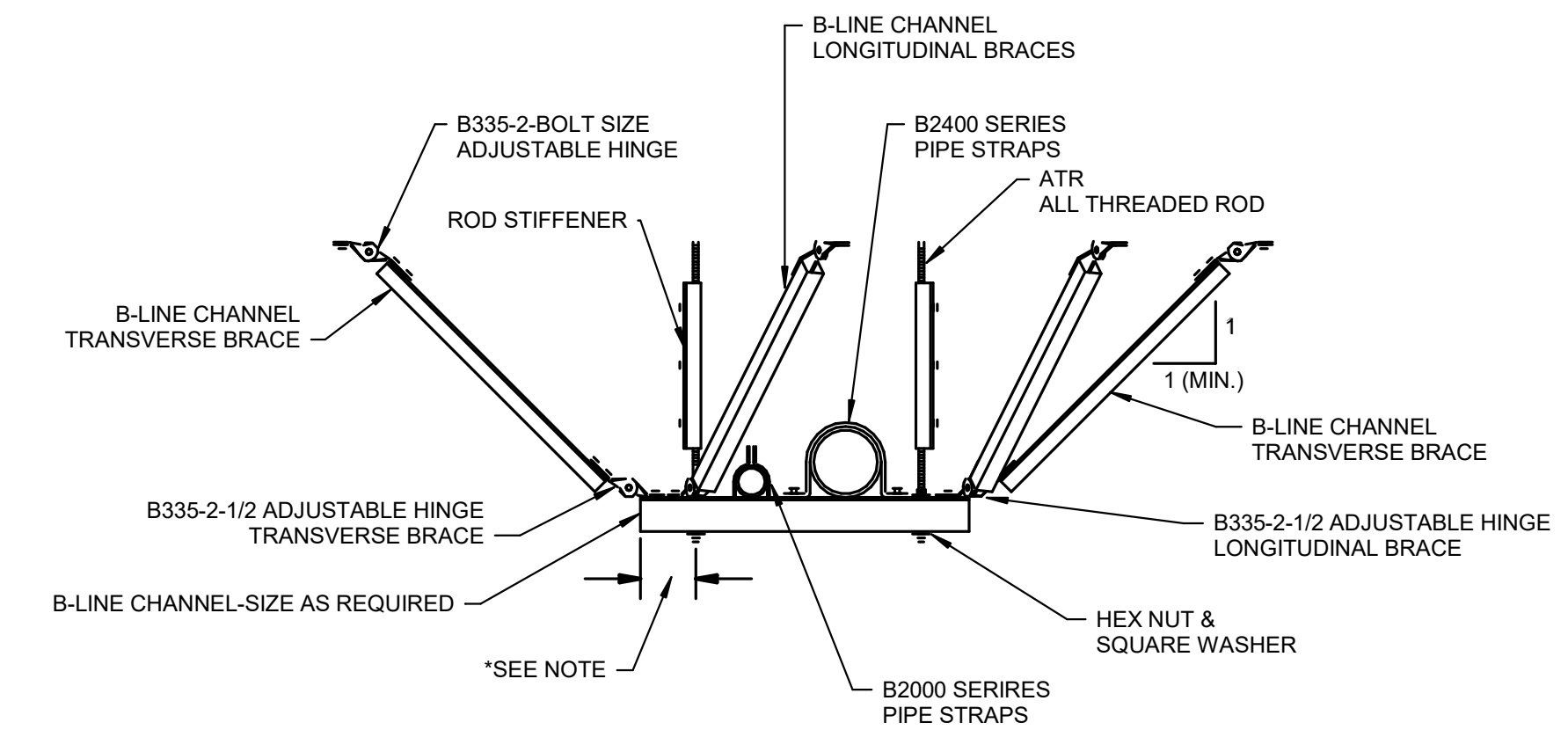
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E002

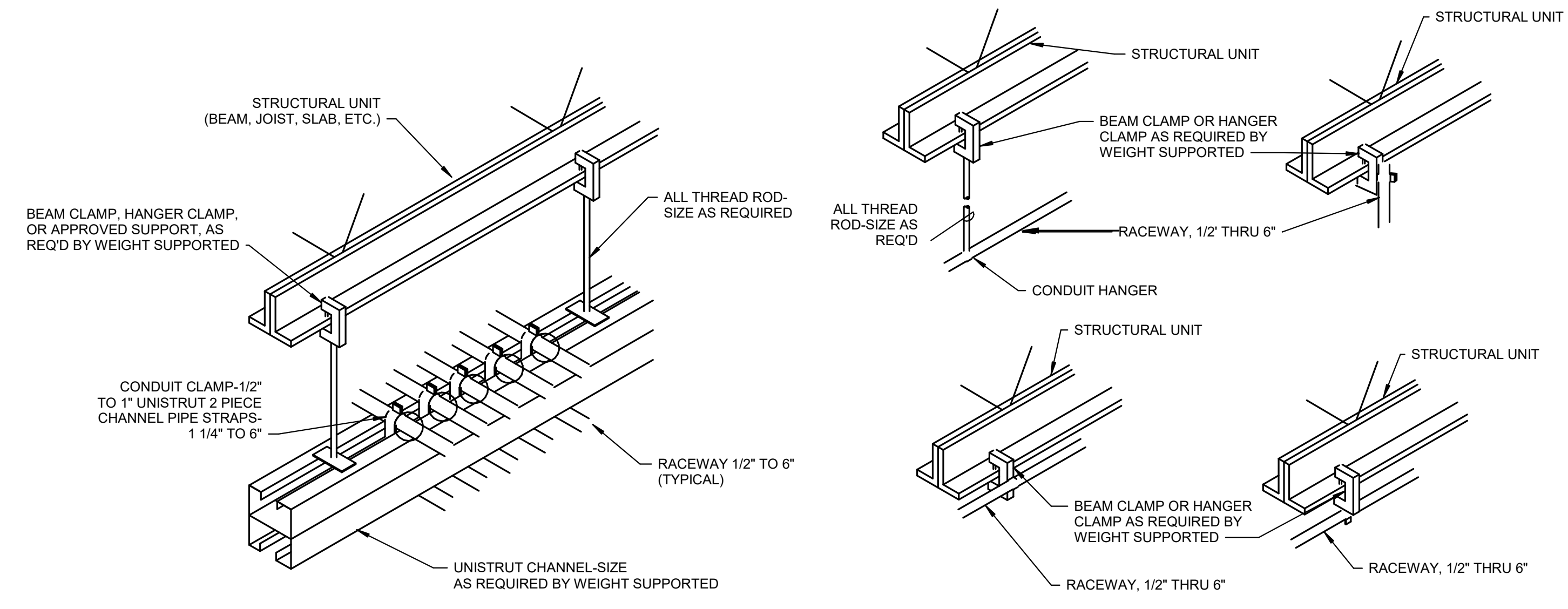


2 TYPICAL MOUNTING HEIGHTS
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- NOTES:
- 1). B335-2 ADJUSTABLE HINGES FOR LONGITUDINAL BRACES MAY BE ATTACHED ON EITHER SIDE ADJACENT TO THE ALL THREAD ROD OR ATTACHED TO THE ALL THREAD ROD ITSELF.
 - 2). B335-2 ADJUSTABLE HINGES FOR TRANSVERSE BRACES MAY BE ATTACHED TO THE ALL THREAD ROD.
 - 3). TWO B335-W ADJUSTABLE HINGES MAY BE ATTACHED TO THE STRUT TRAPEZE USING THE SAME BOLT OR ALL THREAD ROD.
 - 4). IT IS NOT NECESSARY TO INSTALL BOTH TRANSVERSE BRACES AND LONGITUDINAL BRACES ON SAME TRAPEZE SUPPORT. EITHER SET OF BRACES MAY BE REMOVED TO FORM A LONGITUDINAL BRACE ONLY OR A TRANSVERSE BRACE ONLY IF DESIRED.
 - 5). LONGITUDINAL BRACES, WHEN NEEDED, MUST BE INSTALLED AT BOTH ENDS OF TRAPEZE.
 - 6). THE EQUIPMENT SHOWN ON THIS TRAPEZE SUPPORT IS GENERIC IN NATURE. ANY NUMBER OF PIPES, CONDUITS, DUCTWORK OR CABLE TRAY MAY BE SUPPORTED FOLLOWING THE SYSTEM WEIGHT AND SUPPORT SPANS LISTED IN APPENDIX 2 - TABLE 1.
- * DETERMINE LENGTH OF TRAPEZE, MAKING SURE SUFFICIENT LENGTH IS ADDED TO ATTACH THE ALL THREAD ROD AND BRACING ATTACHMENTS.

1 TYPICAL TRAPEZE TRANSVERSE AND LONGITUDINAL BRACING DIAGRAM
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3 TYPICAL CONDUIT RACKING AND SUPPORT DIAGRAMS
SCALE: NTS

QA/QC Completed Date: --/~/--
QA/QC Completed By: HUNT ELECTRIC

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(2410 SO.)
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E003

LIGHTING FIXTURE SCHEDULE - EXPANSION						
TYPE	MANUFACTURER	CATALOG NUMBER	LAMP	TOTAL VA	DESCRIPTION	
EX1W	LITHONIA LIGHTING OR EQUAL	EDG 2 GMR EL M6	LED	1 VA	WALL MOUNTED SINGLE SIDE LED EXIT SIGN WITH BATTERY BACKUP	
HB1	GE CURRENT	ABV3 0 90 57 1D NA TQ 42 A D W	LED	158 VA	LED HIGH BAY LIGHT FIXTURE	
HB1E	GE CURRENT	ABV3 0 90 57 1D NA TQ 42 A D W EL1	LED	158 VA	LED HIGH BAY LIGHT FIXTURE W/ EMERGENCY BATTERY PACK	
WL4E	LITHONIA LIGHTING OR EQUAL	DSXW1 LED 10C 1000 40K T3M MVOLT DDBXD PE E20WC	LED	39 VA	D-SERIES SIZE 1 LED WALL LUMINAIRE - 10 LEDS EM BATTERY PACK PHOTOCCELL	

MECHANICAL EQUIPMENT SCHEDULE EXPANSION															
IDENTIFICATION	EQUIPMENT NAME	QUANTITY	RATING	PHASE	VOLTAGE	FLA/RLA	MCA	MOC/ MFS	NON-FUSED DISC SIZE	FUSED DISC SIZE	RK-1 FUSE SIZE	VFD	WIRE SIZE AND QTY	GROUND WIRE SIZE	NOTES
AUTO_C-1	AUTOCLAVE	1	-	3	480 V	1647	-	2500	-	-	-	-	(7) SETS 4#750 AL	(7) SETS 600 AL	-
OHD-100A	OVERHEAD DOOR	1	-	3	208 V	-	-	20	30	-	-	-	3#12	#12	1, NEMA 12
RTU-X	ROOF TOP UNIT			3	0 V										1, 4X

- NOTES:
- PROVIDE DISCONNECT SWITCH.
 - DISCONNECT SWITCH SHALL HAVE BREAK-BEFORE-MAKE CONTACT. TIE TO VFD EMERGENCY STOP CIRCUIT TO STOP VFD BEFORE SWITCH IS PLACED IN THE OPEN POSITION.
 - PROVIDE THERMAL OVERLOAD SWITCH FOR DISCONNECTING MEANS.
 - PROVIDE COMBO STARTER/DISCONNECT WITH H.O.A. SWITCH AND (2) N.O. AND (2) N.C. CONTACTS.
 - TIE TO 120 VOLT POWER THROUGH FIRE ALARM RELAY. TIE RELAY TO FIRE ALARM CONTROL PANEL FOR CLOSURE OF DAMPER UPON ALARM.
 - CHILLER A SINGLE POINT CONNECTION.
 - PROVIDE 120V/20A/1P TOGGLE SWITCH DISCONNECT ADJACENT TO UNIT.
 - PROVIDE DUCT SMOKE DETECTOR IN RETURN AIR DUCT. TIE AIR HANDLER CONTROLS TO FIRE ALARM SYSTEM FOR SHUT DOWN OF UNIT UPON ALARM.
 - DISCONNECT CONVENIENCE OUTLET PROVIDED WITH UNIT.
 - CONTROLLED VIA WALL SWITCH.
 - INTEGRAL DISCONNECT. DIRECT CONNECT TO UNIT.
 - RUN POWER TO CU-xx FIRST, THEN FROM CU-xx TO FC-xx. VERIFY WITH SUBMITTALS.
 - CORD AND PLUG CONNECTED.
 - PROVIDE DUCT DETECTOR.
 - VFD PROVIDED BY MECHANICAL INSTALL BY ELECTRICAL.

PANEL: LPA1E										
LOCATION: TXA1E			VOLTS: 120/208 WYE			A.I.C. RATING: MCB				
SUPPLY FROM: TXA1E			PHASES: 3			MAINS TYPE: MCB				
MOUNTING: SURFACE			WIRES: 4			MAINS RATING: 225 A				
ENCLOSURE: TYPE 4X						MCB RATING: 225 A				
						SUB-FEED LUGS				
CKT	CIRCUIT DESCRIPTION	TRIP	POLES	A	B	C	POLES	TRIP	CIRCUIT DESCRIPTION	CKT
1	ELECTRICAL ROOM	20 A	1	360 VA						2
3	AUTOCLAVE 100 NW RECEPTACLES	20 A	1		720 VA					4
5	AUTOCLAVE 100 SW RECEPTACLES	20 A	1			540 VA				6
7	AUTOCLAVE 100 NE RECEPTACLES	20 A	1	540 VA						8
9	AUTOCLAVE 100 SE RECEPTACLES	20 A	1		540 VA					10
11	AUTOCLAVE ROOF TOP RECEPTACLE	20 A	1			180 VA				12
13	OVERHEAD DOOR -100A	20 A	3	500 VA						14
15	--	--	--		500 VA					16
17	--	--	--			500 VA				18
19	SPARE	20 A	3	0 VA						20
21	--	--	--		0 VA					22
23	--	--	--			0 VA				24
25	SPARE	20 A	3	0 VA						26
27	--	--	--		0 VA					28
29	--	--	--			0 VA				30
31	SPARE	20 A	1	0 VA						32
33	SPARE	20 A	1		0 VA					34
35	SPARE	20 A	1			0 VA				36
37	SPARE	20 A	1	0 VA						38
39	SPARE	20 A	1		0 VA					40
41	SPARE	20 A	1			0 VA				42
TOTAL LOAD:		1400 VA		1760 VA		1220 VA				
TOTAL AMPS:		12 A		15 A		10 A				
LOAD CLASSIFICATION		CONNECTED LOAD	DEMAND FACTOR	ESTIMATED DEMAND	PANEL TOTALS					
RECEPTACLE		2880 VA	100.00%	2880 VA	TOTAL CONN. LOAD: 4380 VA TOTAL EST. DEMAND: 4755 VA TOTAL CONN.: 12 A TOTAL EST. DEMAND: 13 A					
LIGHTING		0 VA	0.00%	0 VA						
NOTES:										

PANEL: HPA1E										
LOCATION: HDP1A			VOLTS: 480/277 WYE			A.I.C. RATING: MLO				
SUPPLY FROM: HDP1A			PHASES: 3			MAINS TYPE: MLO				
MOUNTING: SURFACE			WIRES: 4			MAINS RATING: 250 A				
ENCLOSURE: TYPE 4X						SUB-FEED LUGS				
CKT	CIRCUIT DESCRIPTION	TRIP	POLES	A	B	C	POLES	TRIP	CIRCUIT DESCRIPTION	CKT
1	AUTOCLAVE LIGHTING	20 A	1	1741...	--			1	SPACE	2
3	AUTOCLAVE LIGHTING	20 A	1		1897...	--		1	SPACE	4
5	EXTERIOR BUILDING LIGHTING	20 A	1			117 VA	--	1	SPACE	6
7	SPARE	20 A	1	0 VA	--			1	SPACE	8
9	SPARE	20 A	1		0 VA	--		1	SPACE	10
11	SPARE	20 A	1			0 VA	--	1	SPACE	12
13	SPARE	20 A	1	0 VA	--		0 VA	--	1	SPACE
15	SPARE	20 A	1		0 VA	--		1	SPACE	16
17	SPARE	20 A	1			0 VA	--	1	SPACE	18
19	SPARE	20 A	1	0 VA	--			1	SPACE	20
21	SPARE	20 A	1		0 VA	--		1	SPACE	22
23	SPARE	20 A	1			0 VA	--	1	SPACE	24
25	SPARE	20 A	1	0 VA	--		0 VA	--	1	SPACE
27	SPARE	20 A	1		0 VA	--		1	SPACE	28
29	SPARE	20 A	1			0 VA	--	1	SPACE	30
31	SPARE	20 A	1	0 VA	--			1	SPACE	32
33	SPARE	20 A	1		0 VA	--		1	SPACE	34
35	SPARE	20 A	1			0 VA	--	1	SPACE	36
37	SPARE	20 A	1	0 VA	1400...		0 VA	--	3	125 A TXA1E
39	SPARE	20 A	1		0 VA	1760...		0 VA	1220...	--
41	SPARE	20 A	1			0 VA	--	--	--	--
TOTAL LOAD:		3141 VA		3657 VA		1337 VA				
TOTAL AMPS:		12 A		14 A		5 A				
LOAD CLASSIFICATION		CONNECTED LOAD	DEMAND FACTOR	ESTIMATED DEMAND	PANEL TOTALS					
RECEPTACLE		2880 VA	100.00%	2880 VA	TOTAL CONN. LOAD: 8135 VA TOTAL EST. DEMAND: 9448 VA TOTAL CONN.: 10 A TOTAL EST. DEMAND: 11 A					
LIGHTING		3751 VA	125.00%	4689 VA						
NOTES:										

QA/QC Completed By: HUNT ELECTRIC

QA/QC Completed Date: --/--

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 PHONE: 801-975-8844
 FAX: 801-975-0509
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DESIGN-BUILD SERVICES

DATE DESCRIPTION NO.

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DRAWN BY: HE
ENGINEER: HE

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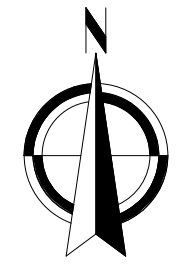
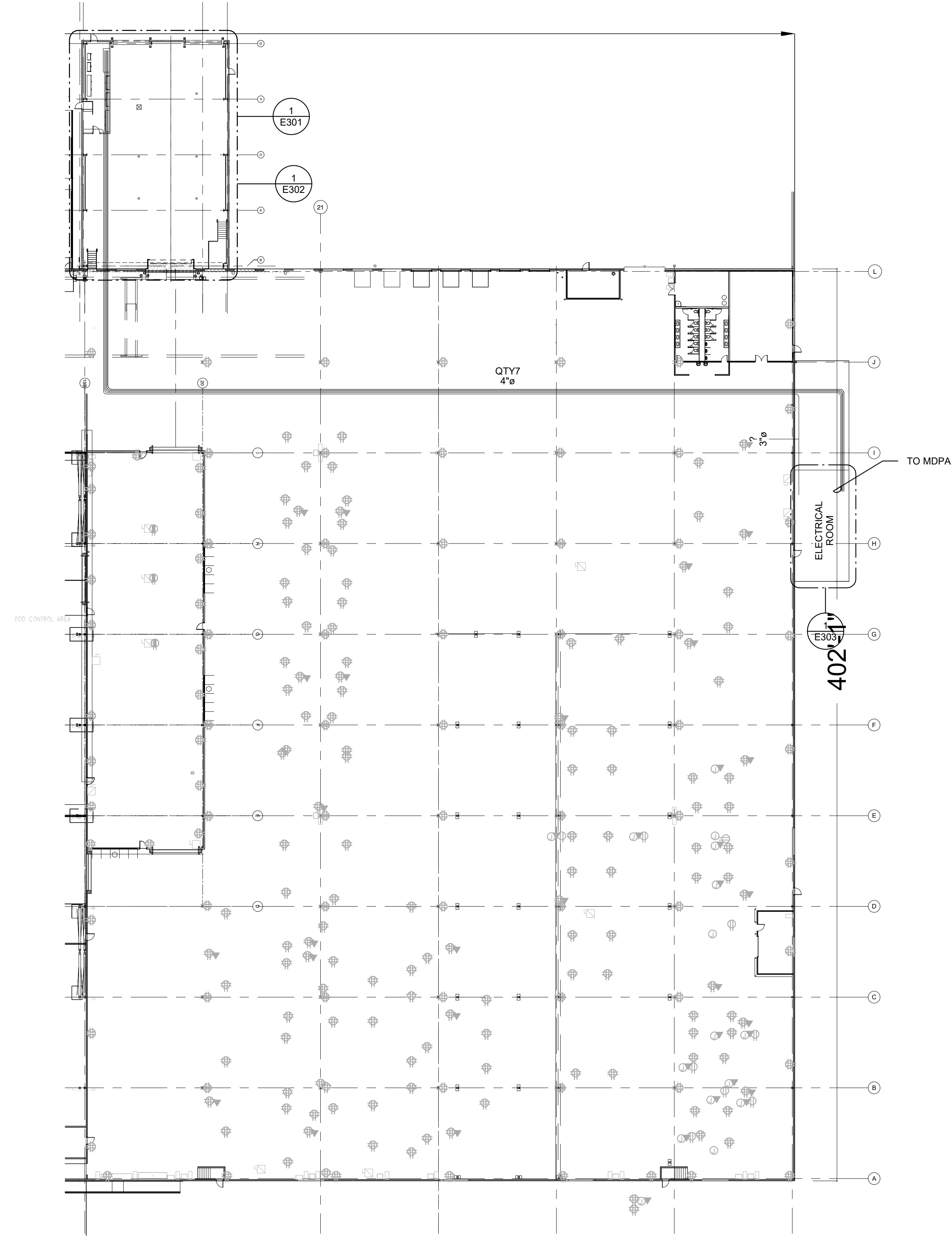
E004

GENERAL NOTES

- A. ALL EXTERIOR EQUIPMENT TO HAVE NEMA 3R LOCKABLE ENCLOSURES.



KEYED NOTES



1 LEVEL 1 EXPANSION ELECTRICAL POWER PLAN

SCALE: 1/32" = 1'-0"

1863 W ALEXANDER ST.
 (2410 SO.)
 SALT LAKE CITY, UTAH 84119
 PHONE: 801-975-8844
 FAX: 801-975-0509



DESIGN-BUILD SERVICES

DATE

DESCRIPTION

NO.

PROJ. MGR.:

DRAWN BY:

ENGINEER:

ALBANY ENGINEERED COMPOSITES
 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
 LEVEL 1 OVERALL ELECTRICAL POWER PLAN
 CLIENT REVIEW

PRINTED DATE:
2/27/2024 7:34:05 AM

SCALE:
1/32" = 1'-0"

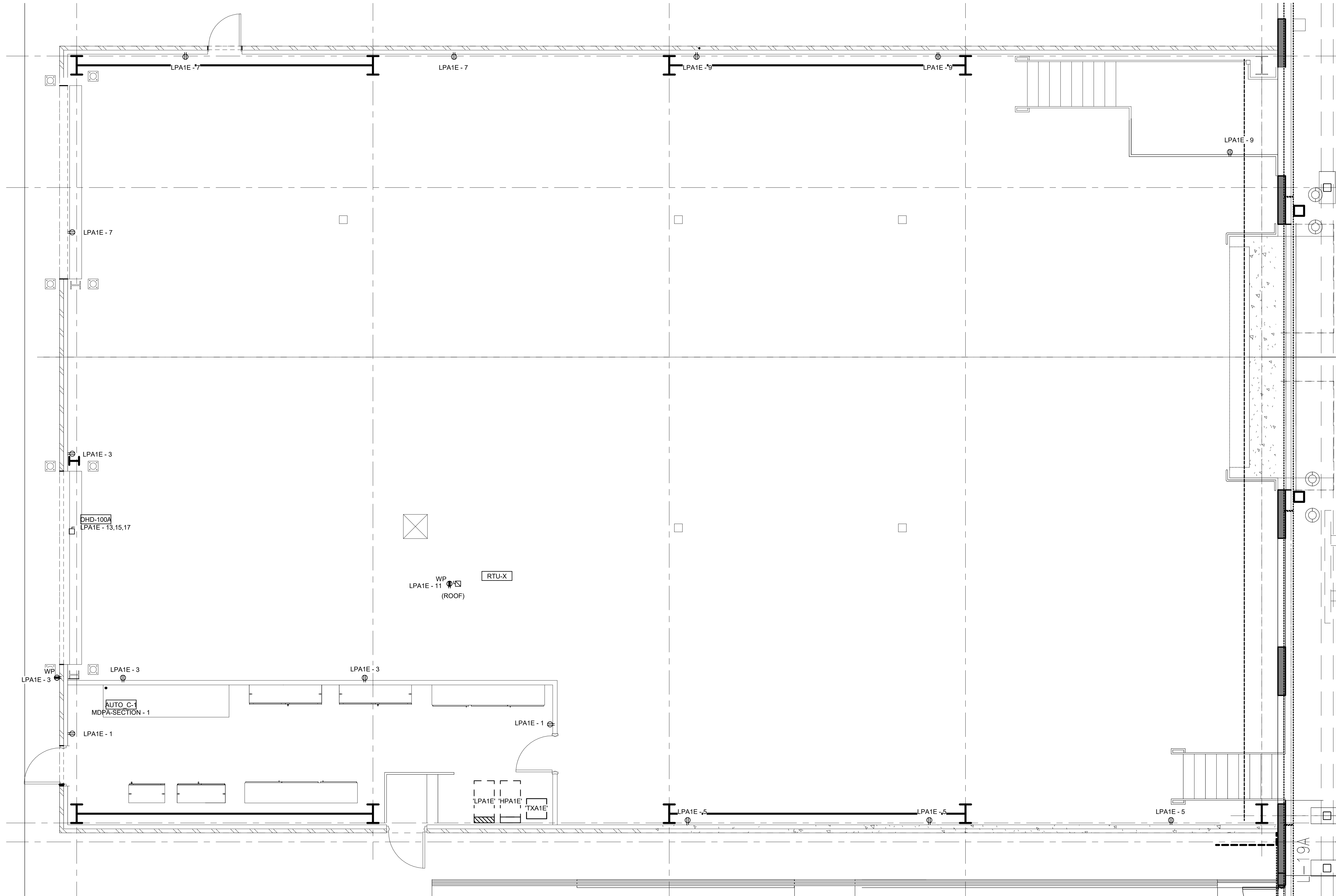
E101

GENERAL NOTES

A. ALL EXTERIOR EQUIPMENT TO HAVE NEMA 3R LOCKABLE ENCLOSURES.

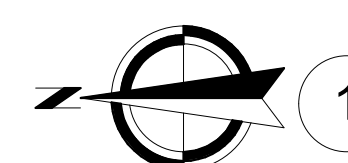


KEYED NOTES



QA/QC Completed By: HUNT ELECTRIC

QA/QC Completed Date: --/--



1

ENLARGED AUTOCLAVE ELECTRICAL POWER PLAN

SCALE: 1/4" = 1'-0"

1863 W ALEXANDER ST. (2410 SO.) SALT LAKE CITY, UTAH 84119 PHONE: 801-975-8844 FAX: 801-975-0509 HUNT ELECTRIC, INC. QUALITY, INTEGRITY, PERFORMANCE & VERSATILITY

DESIGN-BUILD SERVICES

DATE

DESCRIPTION

NO.

PROJ. MGR.:

DRAWN BY:

ENGINEER:

ALBANY ENGINEERED COMPOSITES 5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116 ENLARGED ELECTRICAL PLANS CLIENT REVIEW

PRINTED DATE: 2/27/2024 7:39:06 AM

SCALE: 1/4" = 1'-0"

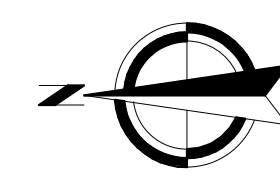
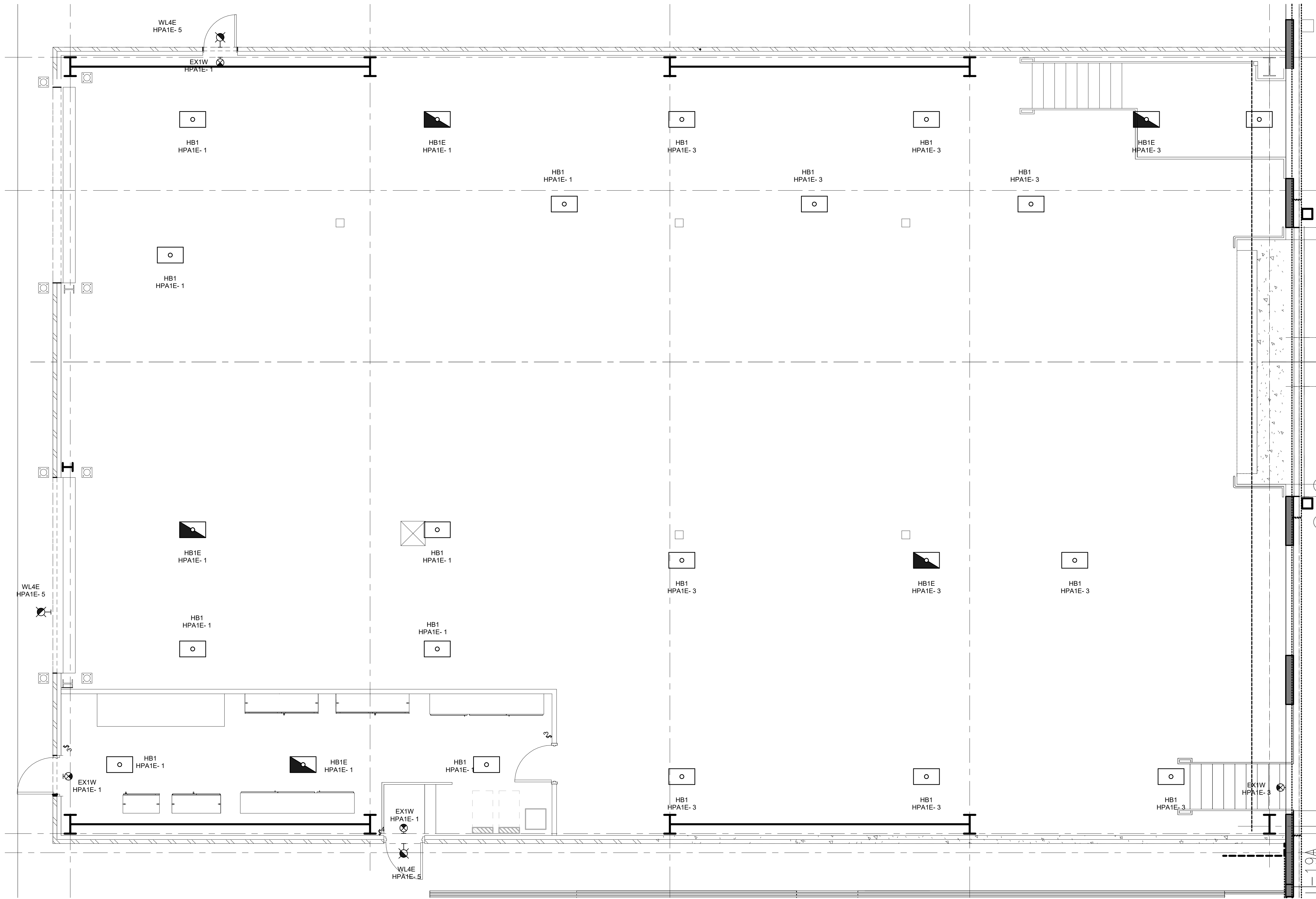
E301

GENERAL NOTES

A. RUN UNSWITCHED HOT TO ALL EMERGENCY BATTERY PACKS AND EXIT SIGNS.



KEYED NOTES



1 ENLARGED AUTOCLAVE ROOM LIGHTING PLAN
SCALE: 1/4" = 1'-0"

QA/QC Completed By: HUNT ELECTRIC

QA/QC Completed Date: --/--/----

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(2410 SO.)
SALT LAKE CITY, UTAH 84119
PHONE: 801-975-8844
FAX: 801-975-0509



DESIGN-BUILD SERVICES
QUALITY, INTEGRITY, PERFORMANCE & VERSATILITY

DATE

DESCRIPTION

NO.

PROJ. MGR.:

DRAWN BY:

ENGINEER:

ALBANY ENGINEERED COMPOSITES
5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
ENLARGED ELECTRICAL PLANS
CLIENT REVIEW

PRINTED DATE:

2/2/2024 7:39:06 AM

SCALE:

1/4" = 1'-0"

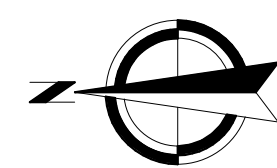
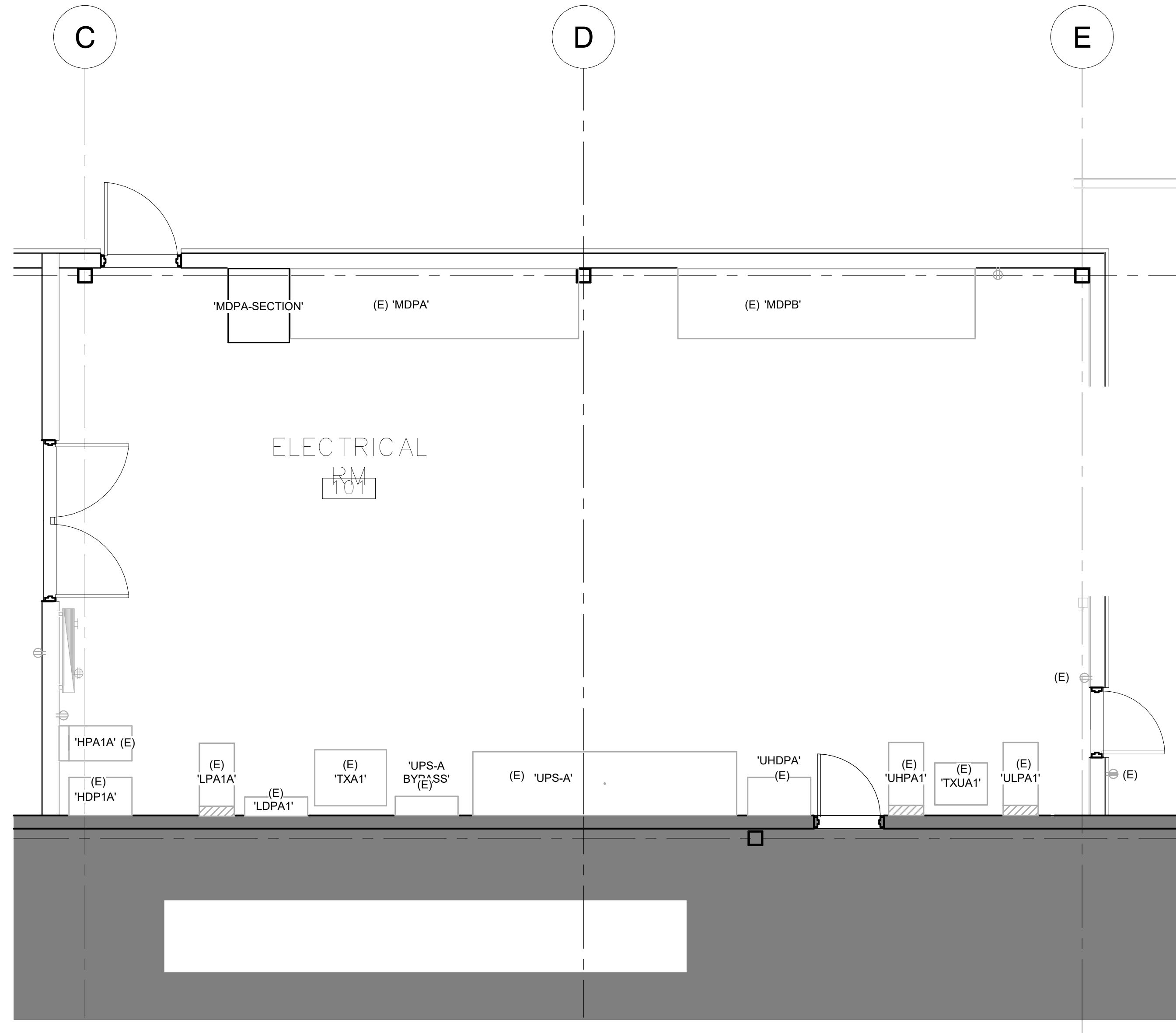
E302

GENERAL NOTES

- A. ALL EXTERIOR EQUIPMENT TO HAVE NEMA 3R LOCKABLE ENCLOSURES.



KEYED NOTES



1 ENLARGED ELECTRICAL ROOM POWER PLAN
SCALE: 1/4" = 1'-0"

QA/QC Completed By: HUNT ELECTRIC

QA/QC Completed Date: --/--/----

1863 W ALEXANDER ST.
(2410 SO.)
SALT LAKE CITY, UTAH 84119
PHONE: 801-975-8844
FAX: 801-975-0509



DESIGN-BUILD SERVICES
QUALITY, INTEGRITY, PERFORMANCE, & VERSATILITY

DATE

DESCRIPTION

NO.

PROJ. MGR.:

DRAWN BY:

ENGINEER:

ALBANY ENGINEERED COMPOSITES
5995 W. AMELIA EARHART DRIVE, SALT LAKE CITY UTAH, 84116
ENLARGED ELECTRICAL PLANS
CLIENT REVIEW

PRINTED DATE:
2/27/2024 7:34:07 AM

SCALE:
1/4" = 1'-0"

E303