BRIGHAM YOUNG UNIVERSITY

SAB GOLF CART CANOPY PARKING **ADVANCEMENT VP - ATHLETICS**

STUDENT ATHLETE BUILDING SITE- NORTHWEST CORNER

ABBREVIATIONS

And Angle At Centerline Diameter or Round Square Feet Perpendicular Pound, Number (E) Existing Air Conditioning AC ACC. ACOUS. Access Acoustical ADD. Addendum ADD'L. A.F.F. Additional Above Finished Floor ALUM. ALT. Aluminum Alternate ALT. APPROX. ARCH. AVG. BD. BET. BF. Approximate Architect(ural) Average Board Between Board Feet BF.Board FeetBITUM.BituminousBLDG.BuildingBLKG.BlockingCAB.CabinetCHAM.ChamferCLCenterlineCLG.CeilingCLR.Clear(ance)CMUConcrete MasCOL.ColumnCONC.ConcreteCONSTR.ConstructionCONT.ContinuousCONTR.Contractor Concrete Masonry Unit CONTR. Contractor DBL. DEMO. DIA. DIM. DN. DR. DET. DWG. EA. ELEC. ELEV. EQ. EQUIP. EST. EXIST. Double Demolish, Demolition Diameter Dimension Down Door Detail Drawing Each Electric(al) Elevation Equal Equipment Estimate

Existing

FIN. FLR. Finish Floor FLUOR. Fluorescent FURN. Furnish, Furnished GA. GYP. Guage Gypsum HDWD. Hardwood HDWR. Hardware HORIZ. HT. Horizontal Height Heating Ventilating & A/C Insulation HVAC INSUL. Length Material L MATL. MAX. MECH. Maximum Mechanical MANUF. Manufacturer MANUF MIR. MISC. MTL. N.I.C. NO. N.T.S. O.C. OPN'G Mirror Miscellaneous Metal Not In Contract Number Not To Scale On Center(s) Opening OPP. O.T.S. Opposite Open To Structure P-LAM. PLBG. Plastic Laminate Plumbing Plywood Prefabricate Prefinished PLYWD. PREFAB. PREFIN. P.S.F. P.S.I. QTY. Pounds per Square Foot Pounds per Square Inch Quantity Radius Required Resilient RAD. REQD. RESIL. SPECS. SQ. S.S. STD. STL. TYP. T.O. VAT. VCT. VCT. VCT. VERT. W/O SCHED. SIM. Specifications Square Stainless Steel Standard Steel Typical Top of Vinyl Asbestos Tile Vinyl Composition Tile Vertical Without Schedule Similar

NOTE: SEE OTHER CONSULTANT DRAWINGS FOR ADDITIONAL ABBREVIATION INFORMATION



	GENERAL NOTES	VOU	NGU
	1. GENERAL CONTRACTOR SHALL TAKE NECESSARY ACTIONS TO PROTECT EXIST. CONDITIONS SURROUNDING EACH SITE INCLUDING BUILDINGS, SITE ELEMENTS, AND VEGETATION.	PRIGHTAR	NDED YU 875
	2. GENERAL CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND CONDITIONS PRIOR TO CONSTRUCTION AND FABRICATION OF CANOPIES.	FACILITIES	S PLANNING
	3. CONTRACTOR SHALL BE AWARE THAT OWNER WILL OCCUPY SURROUNDING BUILDINGS DURING CONSTRUCTION AND WILL BE REQUIRED TO COORDINATE CONSTRUCTION ACTIVITIES WITH OWNER.	PHONE: (8 FAX: (8 DATE:	01) 422-5504 01) 422-0566
	4. CONTRACTOR SHALL PROVIDE TEMP. LIGHTING AS NECESSARY DURING CONSTRUCTION.	DESIGNER: DRAWN BY:	J. PALMER JP
	5. TESTING AND SPECIAL INSPECTIONS WILL BE PROVIDED AS DEEMED NECESSARY BY OWNER. GENERAL CONTRACTOR TO COORDINATE WITH OWNER ON SCHEDULING PROVIDE NECESSARY ARRANGEMENT FOR TESTING TO OCCUR.	ADA CHECK: CODE CHECK: STRUCTURAL: ENGINEERING: PLANNING DIR:	
			/AL DATE
		REVISIONS	
	DRAWING INDEX	IJZ	RKING TICS
	GENERAL: G0.0 COVER SHEET G0.1 CANOPY CAMPUS LOCATION PLAN G0.2 BUILDING SHELTER DIMENSIONAL CONTROL PLANS	Ο U] I T Υ	DPY PA ATHLE
	CIVIL: CIVIL: C-2 SAD COLF CART SITE REMOVED FROM SET STRUCTURAL:	(MY) Frs	T CANC UT VP -
	SE-001 GENERAL NOTES & 3D RENDERING SE-111 FRAMING PLANS ARCHITECTURAL: A1.0 CANOPY SHELTER - TYPICAL DRAWINGS	G H A J N I V	F CAR. CEMEN
	ELECTRICAL: E.1.0 ELECTRICAL SITE PLAN	BRI(ADVAN STUDENT A
and the second			S A S
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		MUSTATE.	OF U.A.
		NE 308	2464-0301 1/2025
		COVER SI	HEET
	PROJECT DIRECTORY 1. BYU Planning PM: Joseph Palmer		
	801-422-5558 joseph_palmer@byu.edu 2. BYU Architectural Manager - Campus Projects: Keith Martin 801 422-5571		
	3. Please CC project correspondence to the following: Bart Smith bart_smith@byu.edu Bob Coleman robert_coleman@byu.edu		b/8
		G	J.U
		CONSTRUCTIO	N DOCUMENTS



		FACILITIES 240 BRWB PRO PHONE: (8 EAX: (8) DATE: DESIGNER: DRAWN BY: ADA CHECK: CODE CHECK: STRUCTURAL: ENGINEERING: PLANNING DIR: CLIENT APPROV	NG UH NDED YU 875 YU SPLANNING OVO, UTAH 84602 01) 422-0566 02.02.2024 J.PALMER JP
		BRIGHAM YOUNG UNIVERSITY	SAB GOLF CART CANOPY PARKING ADVANCEMENT VP - ATHLETICS STUDENT ATHLETE BUILDING SITE- NORTHWEST CORNER
	BIKE CANOPY LOCATIONS	CONSTRUCTIC	A SHEET NO.



NOTES:

- 1. CONCRETE SHALL BE MONOLITHIC 4500 PSI CONCRETE.
- 2. PLACE SCORE LINES AS OUTLINED ON SHEET GO.2. 3. EDGE SIDEWALK WITH 1/2" RADIUS EDGING TOOL.
- 4. FINISH CONCRETE WITH MEDIUM BROOM FINISH.
- 5. SCARIFY AND RECOMPACT UPPER 8" OF NATURAL SUBGRADE TO 95% PER ASTM D1557.
- 6. ROADBASE TO MEET SPECIFICATION OF UDOT CLASS A AGGREGATE UNTREATED BASE COURSE. COMPACT TO 95% OF MAXIMUM DRY DENSITY. 7. CONCRETE TO BE SUPPLIED BY GENEVA ROCK OR JACK B. PARSON/STAKER PARSON TO MEET THE FOLLOWING SPECIFICATIONS: 6.5-BAG
- MIX, 4500 PSI, MAX 45% WATER-CEMENT RATION, 6% AIR CONTENT (+/-1.5%), 3-INCH SLUMP (+/-1 INCH). 8. FORM HEIGHT SHOULD EXCEED CONCRETE DIMENSION. NO 2x4 FORMING IS ALLOWED FOR 5" CONCRETE CONSTRUCTION.
- 9. APPLY WATERBORNE MEMBRANE-FORMING CURING COMPOUND TO CONCRETE AFTER PLACING & FINISHING CONCRETE.
- 10. APPLY PENTRA–SHIELD OR PENTRA–SIL (244+) SURFACE HARDENER TO BE APPLIED TO ALL EXTERIOR FLATWORK AT LEAST 7 DAYS AFTER PLACEMENT & AFTER REMOVING CURING COMPOUND AT MANUFACTURER'S RECOMMENDED RATES.
- 11. DO NOT PERMIT VEHICULAR TRAFFIC OVER PAVEMENT FOR 7 DAYS MINIMUM AFTER FINISHING.





STRU	CTURAL GENERAL NOTES		4.	FOOTIN	NGS:
DAG				A.	FOOTINGS SHALL BEAR ON PROPERLY PREPARED MATERIAL. SEE THE SITE
BASI	S OF DESIGN			в	PREPARATION NOTES. FOOTINGS SHALL BE CENTERED BELOW THE WALL AND/OR COLUMN ABOVE
1.	GOVERNING BUILDING CODE	021 IBC		5.	TYPICAL UNLESS NOTED OTHERWISE.
3.	LOAD COMBINATIONS:			C.	EXTERIOR FOOTINGS SHALL BEAR BELOW THE EFFECTS OF FROST.
	ASD - GRAVITY AND LATERAL 1	605.3.1		D.	PROVIDE A 2X4 BEVELED KEY WAY IN ALL CONTINUOUS WALL FOOTINGS.
4.	GRAVITY DESIGN:			E.	STAGGER FOUTING CONSTRUCTION JOINTS FROM WALL CONSTRUCTION
	ROOF 1	5 PSF		F	REINFORCING IN CONTINUOUS FOOTINGS SHALL BE CONTINUOUS AT
	ROOF LIVE LOAD	0101		•••	CORNERS AND/OR INTERSECTIONS BY PROVIDING PROPER LAP LENGTHS
	(NOT CONCURRENT WITH ROOF SNOW LOAD)	0 PSF			AND/OR CORNER BARS.
	ROOF SNOW LOAD			G.	NO PENETRATIONS SHALL BE ALLOWED THROUGH ANY CONCRETE
	GROUND SNOW LOAD	3 PSF			FOOTING. WHEN CONFLICTS ARISE BETWEEN UNDERGROUND PLUMBING,
	FLAT ROOF SNOW LOAD	00			UTILITIES, ETC., THE FOOTING SHALL BE STEPPED DOWN BELOW THE
	Ce	.00			EXTENDED TO THE FOOTING AS REQUIRED.
	Ct	.20		Н.	BEARING SURFACES FOR FOOTINGS WHICH ARE, OR BECOME,
	DRIFTING PER ASCE 7				UNDERMINED DURING CONSTRUCTION SHALL BE BACKFILLED WITH A LEAN-
5.	LATERAL DESIGN:				MIX CONCRETE (2000 PSI MIN.).
	WIND DESIGN PER ASCE 7-10	00 100			
	ULTIMATE DESIGN WIND SPEED 1	U3 MPH	REINF	ORCING	STEEL
	INTERNAL PRESSURE COFFEICIENT	, 18	1	CODES	AND STANDARDS
	TOPOGRAPHIC FACTOR	.00	1.	A.	REINFORCING STEEL SHALL COMPLY WITH:
6.	SOIL BEARING PRESSURE: 1500 PS	F (ASSUMED)			I. CONCRETE REINFORCING STEEL INSTITUTE "MANUAL OF STANDARD
7.	MINIMUM FROST COVER: 30 INCH	ES			PRACTICE".
8.	SEISMIC DESIGN				II. AMERICAN CONCRETE INSTITUTE "DETAILING MANUAL", ACI 315 (OR
	SEISMIC DESIGN PROCEDURE	:LF 016 a	2	MATED	SP-00).
	SD3	1.910 g	Ζ.		IALO. REINFORCING STEEL SHALL BE NEW STOCK DEFORMED BARS AND SHALL
	SITE CLASS)			CONFORM TO ASTM A615, GRADE 60, WITH A DESIGN YIELD STRENGTH OF
	SEISMIC DESIGN CATEGORY)			60,000 PSI, EXCEPT AS NOTED BELOW.
	IMPORTANCE FACTOR le 1	.00			I. DOWELS TO BE BENT IN THE FIELD DURING CONSTRUCTION SHALL
	REDUNDANCY FACTOR 1	.00			BE ASTM A615, GRADE 40 OR ASTM A706, GRADE 60, "LOW-ALLOY
	PURTION OF SNOW LOAD INCLUDED			c	STEEL".
	LATERAL FURGE RESISTING SYSTEM			U.	INICUTIANIUAL SPLICE COUPLERS SHALL BE CAPABLE OF DEVELOPING 125%
GENE	RAL		3	CONST	RUCTION:
1.	THE GENERAL CONTRACTOR SHALL:		0.	A.	REINFORCING SHALL BE DETAILED. BOLSTERED. AND SUPPORTED PER ACI
	A. BECOME FAMILIAR WITH ALL PORTIONS OF THE CONTR	RACT DOCUMENTS			315.
	AND INSURE THAT ALL SUBCONTRACTORS ARE FAMIL	IAR WITH THOSE		В.	REINFORCING STEEL SHALL BE FREE OF LOOSE FLAKY RUST, SCALE,
	PORTIONS PERTAINING TO THEIR AREA OF WORK. NO	DEVIATIONS WILL BE			GREASE, OIL, DIRT, AND OTHER MATERIALS WHICH MIGHT AFFECT OR
	ALLOWED UNLESS AGREED UPON BY ALL PARTIES IN V	VRITING PRIOR TO		c	IMPAIR BUND. DEINEODOING SHALL DE CONTINUIQUE IN WALLE, SLADS, EOOTINGS, ETC
	B VERIEVALL DIMENSIONS AND ELEVATIONS COORDINA	TE ALL DOORS		С. D	SPLICES IN CONTINUOUS REINFORCING SHALL BE MADE IN AREAS OF
	WINDOWS, NON-BEARING INTERIOR AND EXTERIOR WA	ALLS, ELEVATIONS.		Б.	COMPRESSION AND/OR AT POINTS OF MINIMUM STRESS. TYPICAL UNLESS
	SLOPES, STAIRS, CURBS, DRAINS, RECESSES, DEPRES	SIONS, RAILINGS,			NOTED OTHERWISE. LAP SPLICES SHALL BE MINIMUM OF 40 BAR
	WATERPROOFING, FINISHES, CHAMFERS, KERFS, ETC.				DIAMETERS LONG IN CONCRETE AND 48 BAR DIAMETERS LONG IN
	C. FIELD VERIFY ALL SITE CONDITIONS AND IMMEDIATELY	NOTIFY THE			MASONRY, TYPICAL UNLESS NOTED OTHERWISE. MINIMUM LAP SHALL BE 24
	STRUCTURAL ENGINEER REGARDING ACTUAL CONDITI	IONS AT THE			INCHES LONG. DOWELS SHALL HAVE A MINIMUM OF 30 BAR DIAMETERS
	SITE WHICH ARE NOT PER THE DRAWINGS.				EMBEDMENT. TENSION SPICES SHALL BE USED IN CONCRETE WHEN
CON	RETE				SPECIFICALLY NOTED, USE A CLASS B SPLICE. SPLICES IN TOP BARS IN SUSPENDED SLABS AND REAMS SHALL BE MADE AT MID SPAN. SPLICES IN
1.	CODES AND STANDARDS:				BOTTOM BARS IN SUSPENDED SLABS AND BEAMS SHALL BE MADE AT
	A. CONCRETE CONSTRUCTION, WORKMANSHIP, AND MAT	ERIALS SHALL			SUPPORTS.
	COMPLY WITH THE AMERICAN CONCRETE INSTITUTE (A	ACI) EDITIONS OF:		E.	BENDS SHALL BE MADE COLD. DO NOT USE HEAT. BENDS SHALL BE DONE
	I. ACI 301, "SPECIFICATIONS FOR STRUCTURAL CO	NCRETE FOR			IN THE FABRICATOR'S SHOP UNLESS SPECIFICALLY NOTED FOR THE FIELD.
	BUILDINGS".			-	DO NOT UN-BEND OR RE-BEND A PREVIOUSLY BENT BAR.
	II. ACI 318, "BUILDING CODE REQUIREMENTS FOR F	REINFORCED		F	
	ACI 347 "RECOMMENDED PRACTICE FOR CONCI	RETE FORM WORK"			WITH THE FOLLOWING MINIMUM CONCRETE AND SHALL BE FOSTIONED
2.	MATERIALS:				COVER:
	A. CEMENT SHALL CONFORM TO ASTM C150, PORTLAND (CEMENT.			CONCRETE CAST AGAINST AND PERMANENTLY
	I. USE TYPE I OR II FOR EXPOSURE CLASS S0				EXPOSED TO EARTH 3"
	II. USE TYPE I OR V FOR EXPOSURE CLASS S1				CONCRETE EXPOSED TO EARTH OR WEATHER:
	III. USE I YPE V FUK EXPUSURE CLASS S2 AND S3 B HARD ROCK ACCPECATES SHALL CONFORM TO ASTM				#0 AINU LAKGEK
	AGGREGATES SHALL CONFORM TO ASTM C330	COOL LIGHT WEIGHT			CONCRETE NOT EXPOSED TO EARTH OR WEATHER:
	C. WATER SHALL BE POTABLE.				SLABS AND WALLS, #11 AND SMALLER
	D. AIR ENTRAINMENT SHALL CONFORM TO ASTM C260.				SLABS ON GRADECENTER OF
	I. LIMIT AIR CONTENT TO 3% AT TROWEL FINISHED) FLOOR SLABS OR			SLAB
	FINISH PER ACI302.1R-96 "GUIDE FOR CONCRE	TE FLOOR AND SLAB			
	F FLY ASH CLASS F PO77OLAN SHALL CONFORM TO AST	M C618			
	F. CALCIUM CHLORIDE SHALL NOT BE USED.				
	G. DEFORMED BAR ANCHORS (DBA) SHALL CONFORM TO	ASTM A496.			
	H. HEADED ANCHOR STUDS (HAS) SHALL CONFORM TO A	STM A108.			
	I. SILICA FUME (2% TO SUSPENDED PARKING SLABS) SHA	ALL CONFORM TO			
2	ASTM 01240				
J.		HE SITE ΔT ΔΝΥ			
	GIVEN TIME.				
	B. LIMIT SLUMP TO 4" MAXIMUM PRIOR TO THE ADDITION	OF PLASTICIZERS			
	AND WATER REDUCING ADD MIXTURES. CONCRETE SL	IPPLIER SHALL			
	INDICATE FINAL SLUMP OF EACH CONCRETE MIX DESIG				
	UNITELY ASH TO 25% OF THE TOTAL COMPUTITIOUS A	UND ON SHEET SOUT			
	E. PEA GRAVEL AGGREGATE AND/OR PLASTICIZER MAY R	E USED IN			
	CONGESTED AREAS WHEN REQUIRED TO PROPERLY F	ILL ALL VOIDS			
	AND/OR FOR WORKABILITY. (CONTRACTORS OPTION).				
4.	CONSTRUCTION:				
	A. CONCRETE SHALL BE PROPERLY VIBRATED DURING PL	ACEMENT.			
	B. PRIOR TO PLACING CONCRETE, CHECK WITH ALL TRAD	ES IU INSURE			
	CONDUITS BOLTS INSERTS EMBEDS DOWELS	LVES, OURDS,			
	ETC. ANCHOR BOLTS AND DOWELS SHALL BE PLACED	PRIOR TO CASTING			
	CONCRETE.				
	C. CONSTRUCTION JOINTS AND BULKHEADS SHALL BE FO	RMED WITH A KEY			
	WAY. ALL CONTACT SURFACES, NEW OR EXISTING, AT	CONSTRUCTION			
	JUIN IS SHALL BE INTENTIONALLY ROUGHENED TO A 1/	4" AMPLII UDE PRIOR			
	TO GASTING ADJAGENT POUK.				

	CONCRETE MIX DESIGN TABL	DNCRETE MIX DESIGN TABLE							
	TYPE OF MEMBER	MIN. STRENGTH @ 28 DAYS (PSI)	MAX. W/C (RATIO)	DRY WEIGHT (PCF)	MAX AGGREGTE SIZE (INCHES)	AIR ENTRAIN- MENT (%)	MIN. CEMENT PER YARD (LBS)	EXPOSURE CLASS	
EXTERIOR MEMBERS									
	FOOTINGS	4000	0.45	145	1	6±1.5	564	F0,S0,P0,C0	
	WALLS, PIERS, BEAMS	4500	0.45	145	3/4	6±1.5	611	F2,S0,P0,C1	
	OTHER SITE CONCRETE	4500	0.45	145	3/4	6±1.5	611	F2,S0,P0,C1	
	SLAB ON GRADE	4500	0.45	145	3/4	6±1.5	611	F2,S0,P0,C1	

STRUCTURAL STEEL

- CODES AND STANDARDS: A. STRUCTURAL STEL DESIGN, FABRICATION, AND ERECTION SHALL COMPLY WITH: I. THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360-16)", WITH "COMMENTARY". II. AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (AISC 303-10)" EXCLUDING SECTIONS 1.5.1,4.4,7.5.4, AND 7.13.3.
- AMERICAN WELDING SOCIETY (AWS-D1.1) "STRUCTURAL WELDING CODE STEEL", EXCLUDING ITEMS CONFLICTING WITH AISC REQUIREMENTS. MATERIALS
- A. STRUCTURAL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. OTHER STRUCTURAL STEEL SHAPES, PLATES, ANGLES, ETC. SHALL CONFORM TO ASTM A36.
- RUSSIAN STEEL IS NOT PERMITTED. HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500, GRADE B, WITH A MINIMUM В. YIELD STRENGTH Fy = 46 KSI. STRUCTURAL PIPE SHALL CONFORM TO ASTM A53, WITH A MINIMUM YIELD STRENGTH
- С Fy = 35 KSI. D. HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM A325 (A490). ANCHOR RODS SHALL HAVE HEADED STUDS AND SHALL CONFORM TO ASTM F1554. ALL OTHER BOLTS SHALL CONFORM TO
- ASTM A307 OR BETTER.
- E. HEADED ANCHOR STUDS AND DEFORMED BAR ANCHORS SHALL CONFORM TO THE MANUFACTURER'S SPECIFICATIONS.
 F. ALL MEMBERS AND WELDS WHICH HAVE COMPLETE PENETRATION GROOVE WELDS SHALL MEET THE CHARPY V-NOTCH TOUGHNESS IMPACT TEST OF 20 FOOT POUNDS AT 0 DEGREES FAHRENHEIT AND 40 FOOT POUNDS AT 70 DEGREES FAHRENHEIT.
- CONSTRUCTION: FABRICATION SHALL BE DONE IN AN APPROVED FABRICATOR'S SHOP. CAMBER IN BEAMS SHALL BE AS INDICATED ON PLANS.
- PROVIDE A SHOP COAT OF PAINT ON ALL STEEL ITEMS, EXCEPT AT AREAS OF WELDING
- AND/OR BOLTING. USE HIGH STRENGTH (8000 PSI MINIMUM AT 28 DAY), NON-SHRINK, LIQUID EPOXY GROUT BENEATH ALL STEEL BASE PLATES AND BEARING PLATES. MIX GROUT WITH SAND OR PEA
- GRAVEL AS RECOMMENDED BY THE MANUFACTURER. PLACE GROUT AS SOON AS STEEL MEMBER HAS BEEN PROPERLY POSITIONED AND ALIGNED. WHERE STRUCTURAL STEEL WIDE FLANGE, PIPE, OR HOLLOW STRUCTURAL SECTIONS ARE EMBEDDED IN CONCRETE OR MASONRY AND REINFORCING BARS BUTT TO IT. DEFORMED BAR
- ANCHORS OR REINFORCING BARS WITH THE SAME SIZE AND SPACING AS THE ADJACENT REINFORCING BARS, 48 BAR DIAMETERS LONG, SHALL BE WELDED TO THE STRUCTURAL STEEL. THE MANUFACTURER?S WELDING PROCEDURES SHALL BE ADHERED TO.
- BOLTED CONNECTIONS: A. STEEL TO STEEL BOLTED CONNECTIONS SHALL BE MADE WITH ASTM A325 (A490) HIGH
 STRENGTH BOLTS AND NUTS, UNLESS NOTED OTHERWISE. BOLTS SHALL CARRY THE IDENTIFYING MARK OF THREE (3) RADIAL LINES
- ALL OTHER BOLTED CONNECTIONS SHALL BE MADE WITH BOLTS AND NUTS CONFORMING TO ASTM A307 UNLESS NOTED OTHERWISE. BOLTS SHALL BE 3/4"(<u>7/8")</u> DIAMETER, TYPICAL, UNLESS NOTED OTHERWISE. STANDARD SPACING SHALL BE 3" O.C. AND STANDARD EDGE DISTANCE SHALL BE 1.1/2", TYPICAL, UNLESS NOTED OTHERWISE BOLT SHALL BE BEARING TYPE CONNECTIONS WITH THREADS EXCLUDED UNLESS NOTED
- DOTHERWISE.
 BOLTED CONNECTIONS SHALL BE TIGHTENED PER SLIP CRITICAL REQUIREMENTS AND SHALL HAVE WASHERS AS REQUIRED BY AISC UNLESS NOTED OTHERWISE.
 F. ENLARGING OF HOLES SHALL BE ACCOMPLISHED BY MEANS OF REAMING. DO NOT USE A TORCH ON ANY BOLT HOLES.
- WELDED CONNECTIONS: A. WELDING AND GAS CUTTING SHALL BE DONE PER AWS.
- WELDERS SHALL BE CURRENTLY CERTIFIED ACCORDING TO AWS. ALL WELDING PROCEDURES SHALL BE PRE-QUALIFIED. WELDERS SHALL FOLLOW WELDING PROCEDURES. WELDED CONNECTIONS SHALL BE MADE USING LOW HYDROGEN MATCHING FILLER MATERIAL ELECTRODES, UNLESS NOTED OTHERWISE. WELDS SHALL HAVE THE SLAG REMOVED.
- FULL PENETRATION WELDS, SHOP OR FIELD, SHALL HAVE BACKER BARS REMOVED, BE BACK GOUGED, AND REWELDED PER AWS TO HAVE FULL PENETRATION WELD.

STEEL DECK PRODUCT:

E

D

- A. STEEL DECK SHALL BE SIZE AND GAUGE AS SPECIFIED ON PLANS.
 B. STEEL DECK AND ACCESSORIES SHALL BE MANUFACTURED FROM COLD ROLLED STEEL CONFORMING TO ASTM A-653, (GALVANIZED G-60), OR ASTM A-611, GRADE C (PAINTED), AND SHALL CONFORM TO THE STEEL DECK INSTITUTE (SDI) AND AISC STANDARDS
- ATTACHMENT AND HANDLING:
 A. WELDING PATTERN SHALL BE AS SPECIFIED ON PLAN. WELD METAL DECK TO SUPPORTING FRAMING MEMBERS WITH E60XX OR E70XX ELECTRODES.
- PUDDLE WELDS SHALL HAVE A FUSION AREA OF NOT LESS THAN 34" DIAMETER AT THE UPPER SURFACE. WELD METAL SHALL PENETRATE ALL LAYERS OF THE DECK MATERIAL AND SHALL HAVE PROPER FUSION TO THE SUPPORTING MEMBERS. D. CRIMP SIDE SEAMS BEFORE WELDING SIDE LAPS. TOP SEAM WELDS SHALL ENGAGE ALL
- D. CRIME SIDE SEAMS BEFORE WELDING SIDE LAPS. TOP SEAM WELDS SHALL ENGAGE ALL LAYERS OF THE DECK MATERIAL.
 E. END LAPS SHALL OVERLAP AT LEAST 2" AND SHALL OCCUR OVER A SINGLE STEEL SUPPORT. PUDDLE WELDS SHALL OCCUR AT LEAST 1" AWAY FROM EITHER END OF DECK.
 F. PUDDLE WELDS 348" X 1.1/2" LONG MAY REPLACE 3/4" DIAMETER PUDDLE WELDS WHEN
- ACCESS IS LIMITED. G. INSTALL DECK WITH A MINIMUM OF 3 SPANS WHEREVER POSSIBLE. PROVIDE ANGLE AND/OR OTHER SUPPORT AROUND THE PERIMETER OF ALL OPENINGS LARGER THAN 10" IN EITHER DIRECTION THROUGH METAL DECK.
- PROVIDE DECK SUPPORT AS REQUIRED AT COLUMNS. DO NOT BEND OR MAR DECK. STORE DECKING OFF THE GROUND WITH ONE END ELEVATED. COVER DECK WITH
- WATERPROOF MATERIAL AND VENTILATE TO AVOID CONDENSATION.





3D VIEWS ARE FOR REFERENCE ONLY. CONSULT PLANS AND DETAIL SHEETS FOR FRAMING SPECIFICATIONS AND **CONNECTION CONFIGURATIONS.**







Section 1 SCALE: 1/2" = 1'-0" 3









